

STATE OF KANSAS
DEPARTMENT OF TRANSPORTATION
PLAN AND PROFILE OF PROPOSED
43 C-5078-01

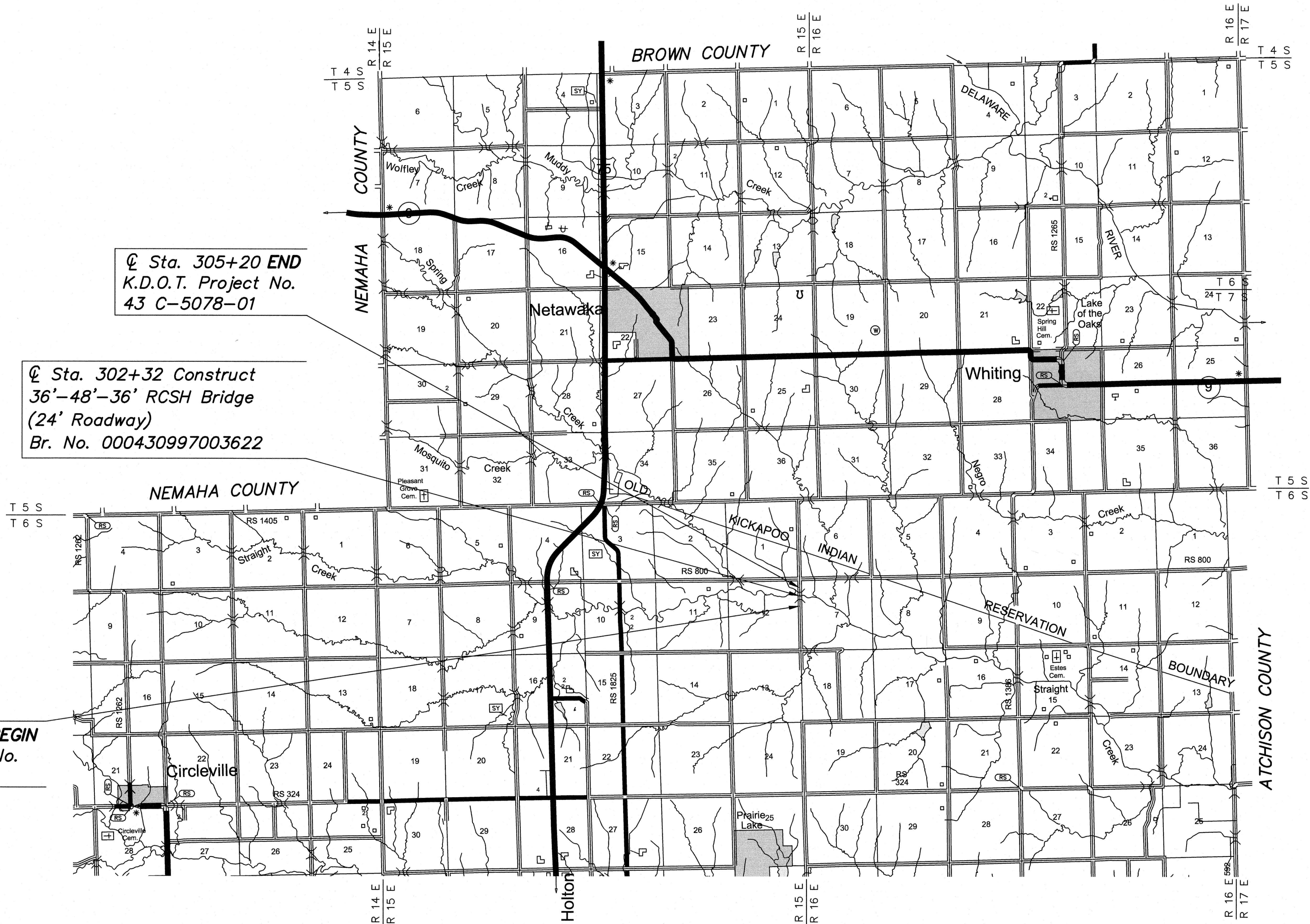
FEDERAL AID PROJECT
JACKSON COUNTY

20-1458M	STATE	Project No.	YEAR	SHEET NO.	TOTAL SHEETS
	KANSAS	43 C-5078-01	2022	1	44
	F.A. NO.	STP-C494(322)			

INDEX OF SHEETS

1. Title Sheet
2. Typical Grading Section
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31. Seeding
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- 38-44. Cross Sections

GRADING
BRIDGE
SEEDING



DESIGN DESIGNATION

AADT = 30 vpd
Design Speed = 30 mph
No Clear Zone

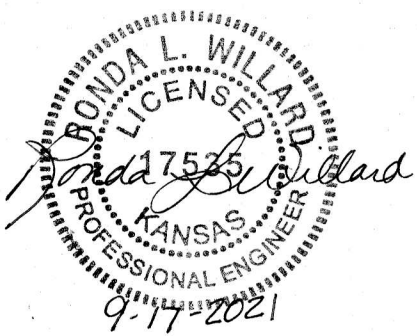
Sta. 299+80 BEGIN
K.D.O.T. Project No.
43 C-5078-01

CONVENTIONAL SIGNS

COUNTY LINE	CENTER LINE OF PROJECT
CITY LIMITS	TERRACE
STATE OR NATIONAL LINE	CULVERTS
TOWNSHIP, SECTION or GRANT LINE	DROP INLET & STORM SEWER
PROPERTY LINE	ACCESS CONTROL
HIGHWAY FENCE	POWER POLE
EXISTING FENCE	TELEPHONE POLE
GUARD FENCE	MARSH
CONSTRUCTION LIMITS	HEDGE
RIGHT OF WAY LINE	TREES
TRAVELED WAY	PROFILE ELEVATION
RAILROADS	STREAM OR CREEK

GROSS LENGTH OF PROJECT 540.00 FT.
EXCEPTIONS 0.00 FT.

NET LENGTH OF PROJECT 540.00 FT. 0.102 MILES
NET LENGTH OF BRIDGES 122.50 FT. 0.023 MILES
NET LENGTH OF ROAD 417.50 FT. 0.079 MILES



PLANS PREPARED AND SUBMITTED BY:	
BG CONSULTANTS ENGINEERS - ARCHITECTS - SURVEYORS 4806 Vue du Lac Place Manhattan KS 66503 T: 1.785.537.7448 Web: www.bgcons.com Lawrence Emporia	
RECOM. FOR APPROVAL-DATE	9-14-2021
COUNTY OFFICIAL	Scott R. [Signature] Public Works Director
	Carlynn Reuser, Admin Assistant

Approved Dec 03, 2021

[Signature]
State Transportation Engineer
By: [Signature]
Chief, Bureau of Local Projects

KANSAS DEPARTMENT OF TRANSPORTATION

Note: Roadway shall be closed to traffic during construction of this project. 1" = 1 Mile

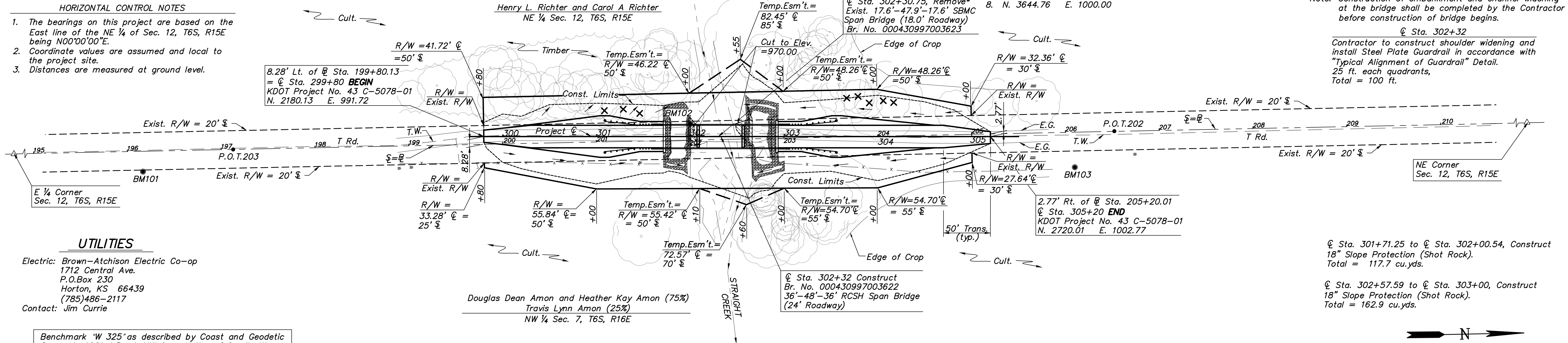
STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
Kansas	43 C-5078-01	2022	3	44

- * Note: All materials obtained from removal of existing structure shall become the property of Contractor and removed from site.*

Q Sta. 302+32

Contractor to construct shoulder widening and install Steel Plate Guardrail in accordance with "Typical Alignment of Guardrail" Detail.
25 ft. each quadrants,
Total = 100 ft.

1. The bearings on this project are based on the East line of the NE ¼ of Sec. 12, T6S, R15E being N00°00'00"E.
2. Coordinate values are assumed and local to the project site.
3. Distances are measured at ground level.



Electric: Brown-Aitchison Electric Co-op
1712 Central Ave.
P.O.Box 230
Horton, KS 66439
(785)486-2117
Contact: Jim Currie

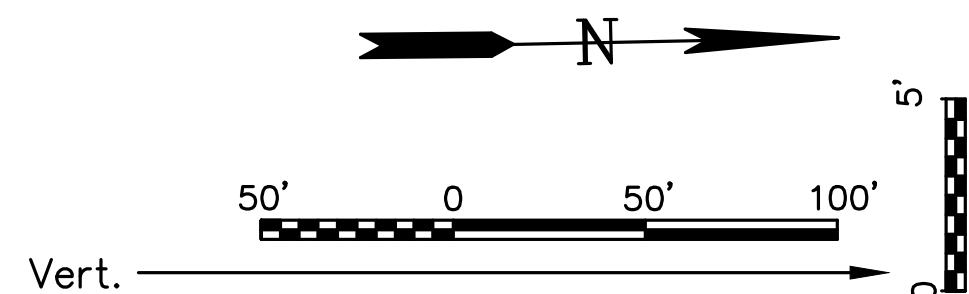
Benchmark "W 325" as described by Coast and Geodetic Survey in 1961; 4.5 mi. NW from Holton. 0.6 mile W. along 5th Street from the Court House at Holton, thence 4.2 miles N. along U.S. Highway 75, 50 feet N. of the center of the road, 4.0 feet SW of the witness post, 1 foot W. of fence corner, set in the top of a concrete post that projects 4 inches.

NAVD88 Elev. = 1088.83

BM101 Railroad Spike in W. Face of Power Pole
21.75' Rt. of \odot Sta. 196+16.23 =
37.48' Rt. of \odot Sta. 296+16.79
Elev. = 990.45 N. 1816.23 E. 1021.75

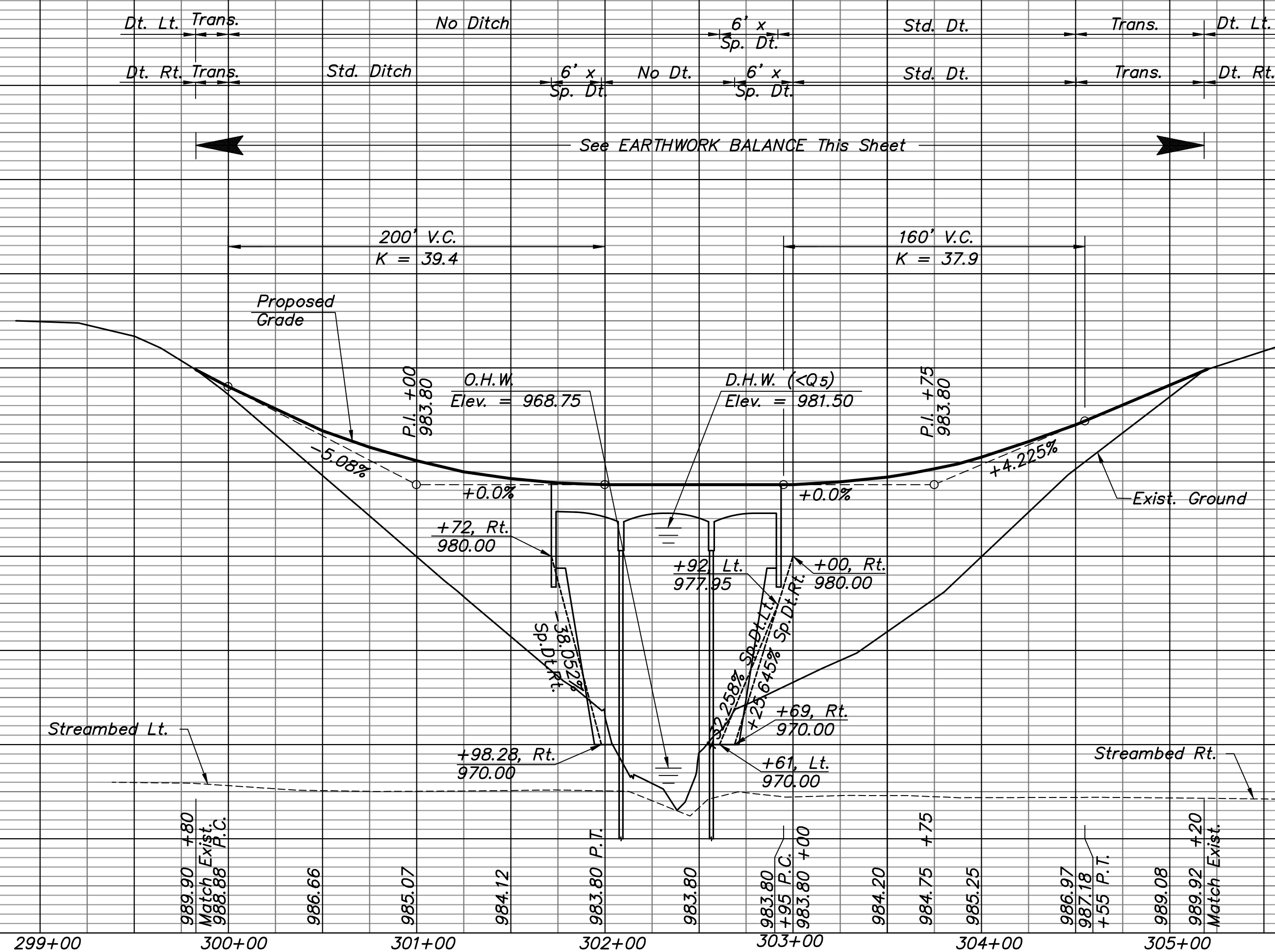
BM102 Set Chiseled Sq. Cut in NW Cor. of SW Conc. Wing.
 17.22' Lt. of \textcircled{B} Sta. 202+00.78 =
 13.45' Lt. of \textcircled{C} Sta. 302+00.45
 Elev. = 971.83 N. 2400.78 E. 982.78

BM103 Set Metal "T" Post 4" Deep.
37.41' Rt. of @ Sta. 206+09.38 =
32.81' Rt. of @ 306+10.06
Elev.=989.65 N. 2809.38 E. 1037.41



Borrow Note:	Borrow areas provided by the Contractor shall be approved by the Engineer as to suitability of material and location. Special care shall be taken in this approval to minimize the increase of siltation and turbidity of streams, lakes, and reservoirs and to avoid interference with the movement of migratory fish. Areas which, in the opinion of the Engineer, may leave an unsightly appearance to the project will not be approved.
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It shall be the responsibility of the Contractor to restore, seed, and/or complete other operations noted in the agreement with the landowner and approved by the Engineer on all disturbed areas used to provide borrow areas for Common Excavation (Contractor Furnished).



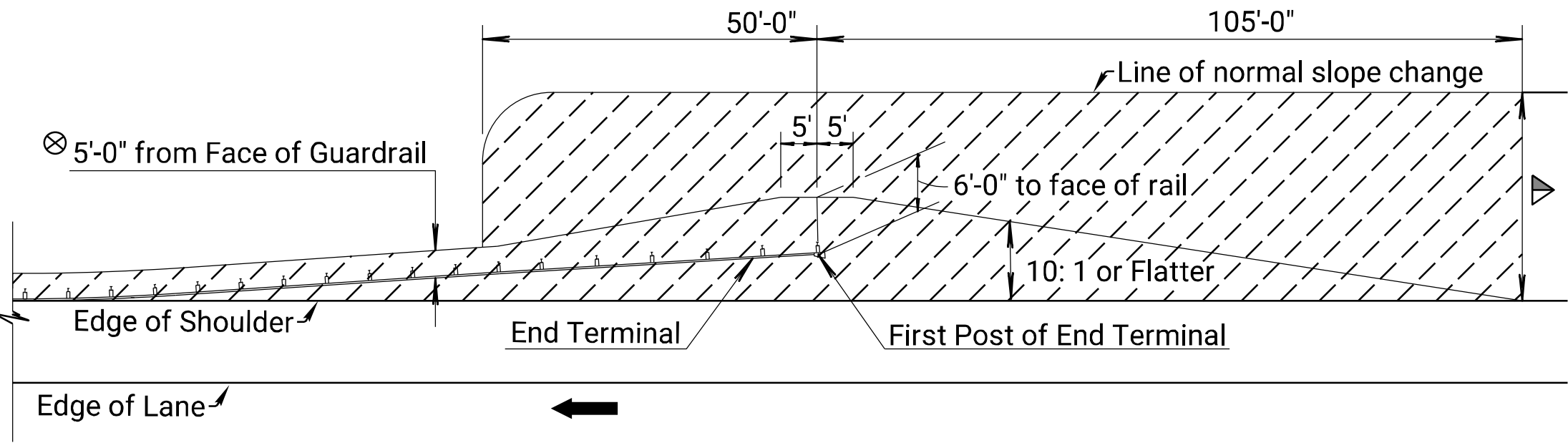
	EARTHWORK BALANCE				
1.	Common Excavation (VMF = 0.70)				
2.	Embankment				
3.	25 cu.yds. Contractor Furnished Excavation				

Note to Designer - Design guardrail installations using guidance shown on KDOT's 'Guardrail Typical Alignments' Standard Drawings. 'Flared' guardrail installations are preferred over 'Parallel' or 'Zero Flare' installations. Where 'Flared' or 'Parallel' installations are used, the flare rate of the guardrail end terminal typically matches the flare rate of the remaining guardrail installation. For 'Zero Flare' installations, 'Parallel' guardrail end terminals should be designed using typical flare rates of 50:1 or flatter for the length of the end terminal. However, while 50:1 or flatter flare rates are typical for 'Parallel' guardrail end terminals, these end terminals may be flared as steep as 26:1 or flatter in order to offset the end terminal head as far from the edge of the through traveled lane as practicable.

Plotted 8/30/2021
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Drawn By : untitled

GUARDRAIL CLEAR AREA

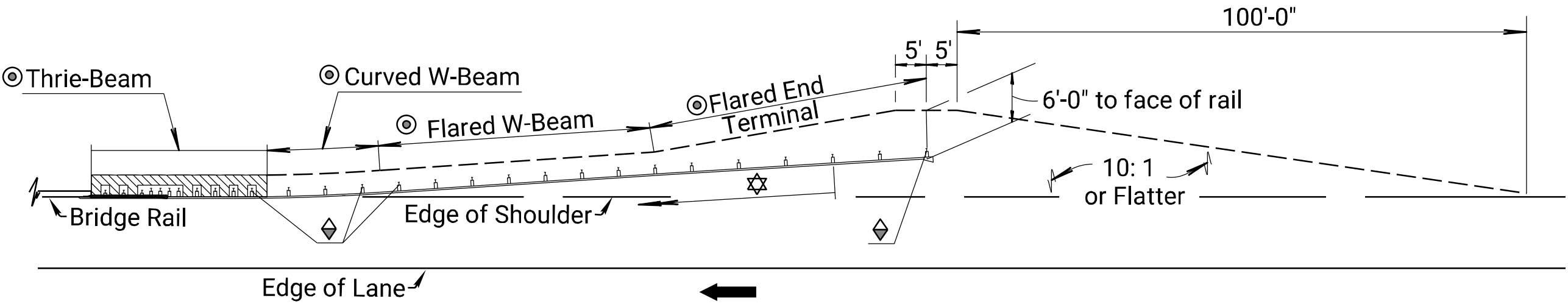
Applies to all guardrail installations unless otherwise shown in the plans.



- Keep Area Free of Stockpiled Material, Equipment, or Other Obstacles, Such as Temporary Signs, Regardless of Crash Worthiness. This Clear Area Extends 105 Feet in Advance of and 50 Feet behind the First Post of the Guardrail End Terminal and Then, in Order to Maintain Full Post Spacing, Continues 5 Feet behind the Face of the Guardrail through the W-Beam Portion of the Installation as Shown in the 'Guardrail Clear Area' Detail on this Sheet.
- Normal Project Side Slope.
- Deflection Distance for Normal Post Spacing

FLARED GUARDRAIL DETAIL

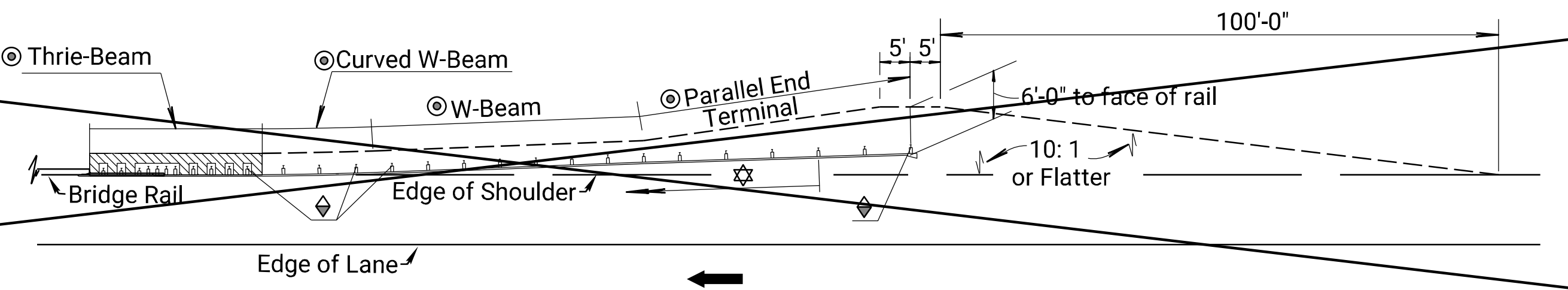
Applies to CGS AND MGS (MGS Shown)



- See Guardrail Layout Sheets for Details
- On Guardrail Layout Sheets, Show Station AND Offset from the Roadway Alignment to the Face of Post at these Locations.
- Length of Need (Begins at Post 3)

PARALLEL GUARDRAIL DETAIL

Applies to CGS AND MGS (MGS Shown)



GENERAL NOTES

Install the guardrail end terminals according to the Manufacturer's Installation Manual. The Contractor will furnish a copy of the Manufacturer's Installation Manual to the Engineer prior to the start of the installation.

Use approved steel (preferred) or wood posts provided by the Manufacturer. The guardrail end terminal post type may be independent of the post type used in the remainder of the installation. However, no mixing of post types is permitted in the remaining w-beam and thrie-beam installation.

Use approved polymer (preferred) or wood blockouts provided by the Manufacturer. The guardrail end terminal blackout size and type may be independent of the blackout size and type used in the remainder of the installation. For blackout size and types for the remaining w-beam and thrie-beam portion of the installation see the details shown on KDOT's 'Guardrail Post Details' and 'Guardrail Thrie-Beam Transition Details' Standard Drawings.

Apply retroreflective sheeting to the end terminal impact head before installation.

Tighten all cable anchor assemblies as per the Manufacturer's Installation Manual.

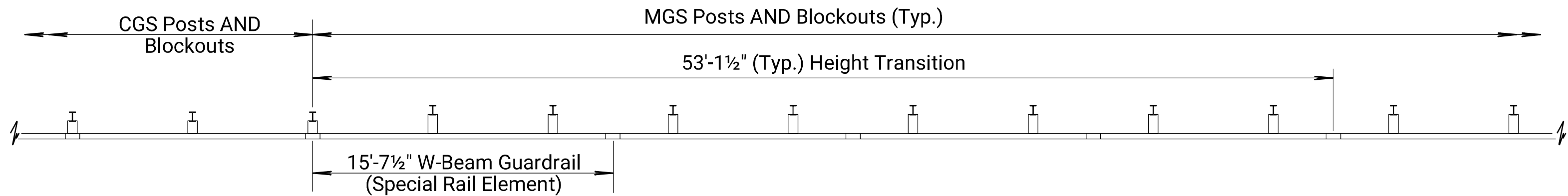
Lap w-beam and thrie-beam guardrail splices, in the direction of permanent traffic, even where temporary traffic may be carried in the opposite direction of the final traffic configuration. Lap end terminal splices per the Manufacturer's Installation Manual in the direction of permanent traffic, even where temporary traffic may be carried in the opposite direction of the final configuration.

The minimum length of w-beam guardrail required between the thrie-beam transition and the guardrail end terminal is 12'-6" for all installations; unless otherwise stated in the Manufacturer's Installation Manual.

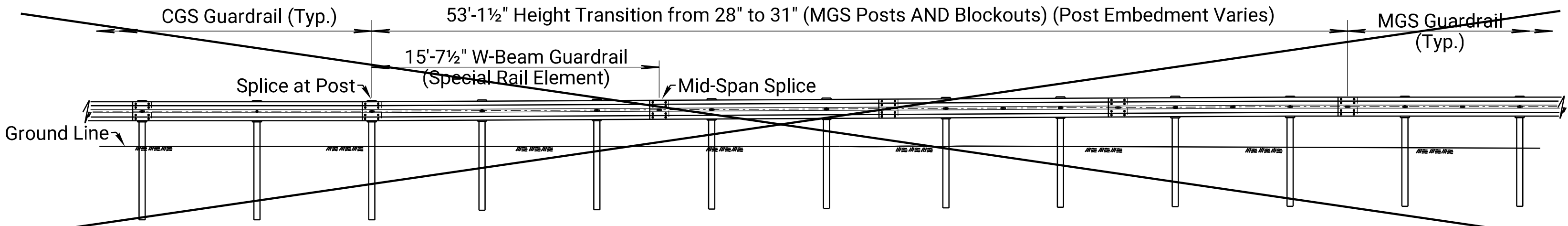
Where pavement with a thickness less than or equal to 8" is encountered during installation, use the details shown on KDOT's 'Guardrail Post Details' Standard Drawings to provide openings in the pavement for the guardrail posts. Where pavement with a thickness greater than 8" or geologic rock is encountered during installation, follow the Manufacturer's Installation Manual for guidance. Where the Manufacturer's Installation Manual does not address pavement with a thickness greater than 8" or geologic rock, contact the manufacturer for instructions or install the guardrail posts as directed by the Engineer.

All work and materials required for w-beam and thrie-beam guardrail installations are paid for under the appropriate bid items for either CGS or MGS guardrail depending on the type of installation.

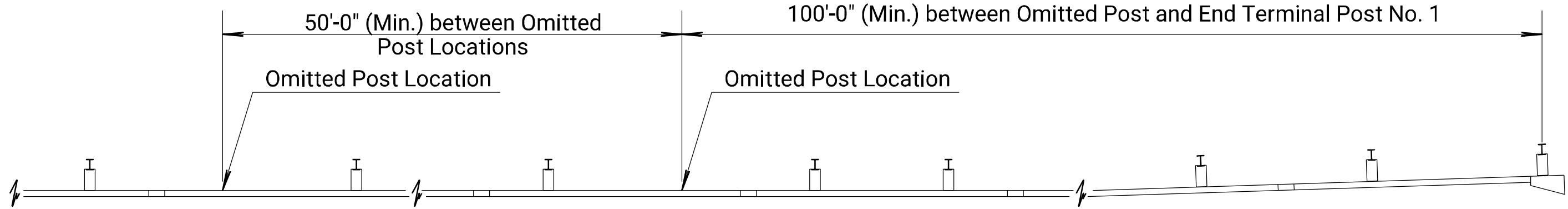
All work and materials required for guardrail end terminal installations are paid for under the bid item for the selected guardrail end terminal. See the table on this sheet for the appropriate end terminal bid item information.



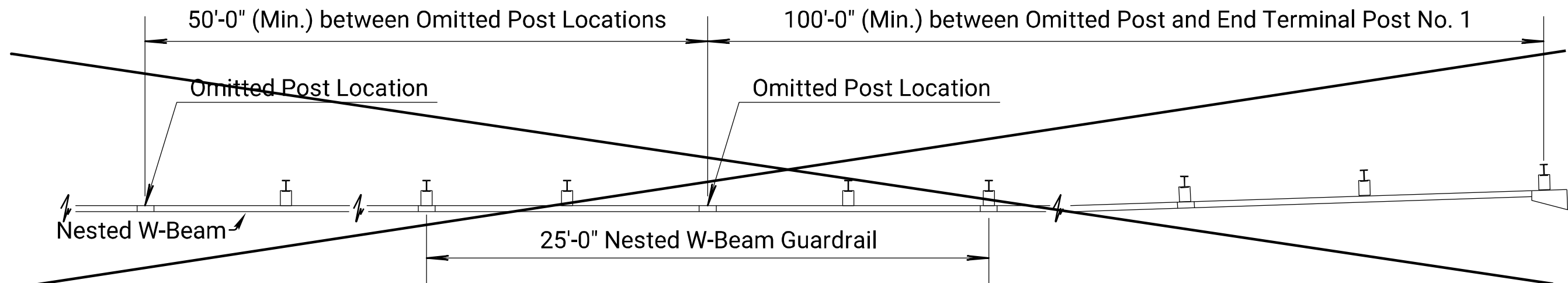
CGS TO MGS TRANSITION DETAILS (PLAN)



CGS TO MGS TRANSITION DETAILS (ELEVATION)



MGS OMITTED POST DETAIL



CGS OMITTED POST DETAIL

MIDWEST GUARDRAIL SYSTEM (MGS) END TERMINALS

END TERMINAL BID ITEM	FLARED OR PARALLEL	MOUNTING HEIGHT	CRASH TESTING CRITERIA	STEEL POST DESIGN AVAILABLE	WOOD POST DESIGN AVAILABLE	ENERGY ABSORBING	MANUFACTURER	DESIGN LENGTH	MANUFACTURER SYSTEM LENGTH
Guardrail End Terminal (MGS-FLEAT)	Flared	31"	NCHRP 350	Yes	Yes	Yes	Road Systems	40'-7½"	37'-6"
Guardrail End Terminal (MGS-SRT)	Flared	31"	NCHRP 350	Yes	Yes	No	Trinity Industries	40'-7½"	37'-6"
Guardrail End Terminal (MGS-MSKT)	Parallel	31"	MASH	Yes	No	Yes	Road Systems	46'-10½"	46'-10½"
Guardrail End Terminal (MGS-SOFTSTOP)	Parallel	31"	MASH	Yes	No	Yes	Trinity Industries	46'-10½"	50'-9½"

CONVENTIONAL GUARDRAIL SYSTEM (CGS) END TERMINALS

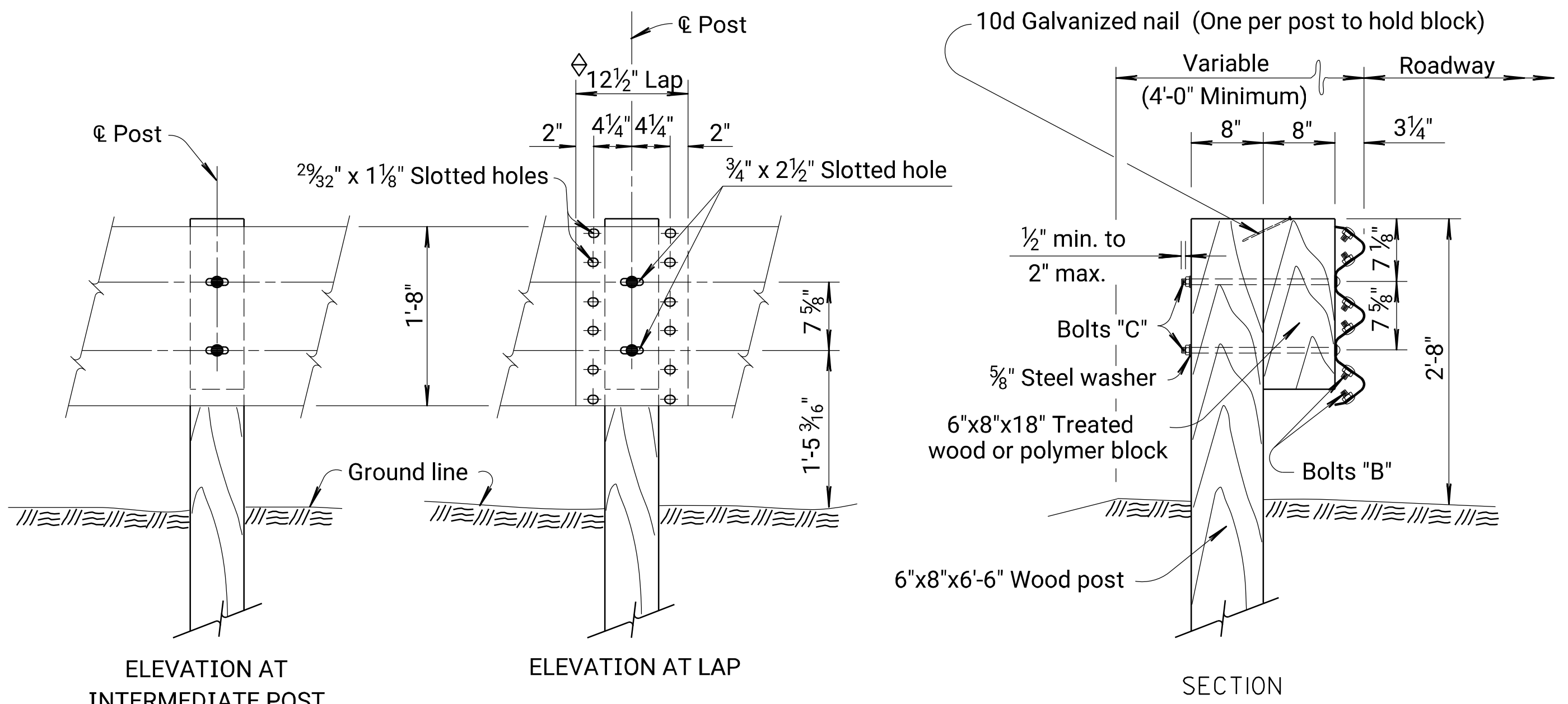
END TERMINAL BID ITEM	FLARED OR PARALLEL	MOUNTING HEIGHT	CRASH TESTING CRITERIA	STEEL POST DESIGN AVAILABLE	WOOD POST DESIGN AVAILABLE	ENERGY ABSORBING	MANUFACTURER	DESIGN LENGTH	MANUFACTURER SYSTEM LENGTH
Guardrail End Terminal (FLEAT)	Flared	28"	NCHRP 350	Yes	Yes	Yes	Road Systems	37'-6"	37'-6"
Guardrail End Terminal (SRT)	Flared	28"	NCHRP 350	Yes	Yes	No	Trinity Industries	37'-6"	37'-6"
Guardrail End Terminal (SKT)	Parallel	28"	NCHRP 350	Yes	Yes	Yes	Road Systems	50'-0"	50'-0"

WOOD POSTS REQUIRED,
NO OPTION

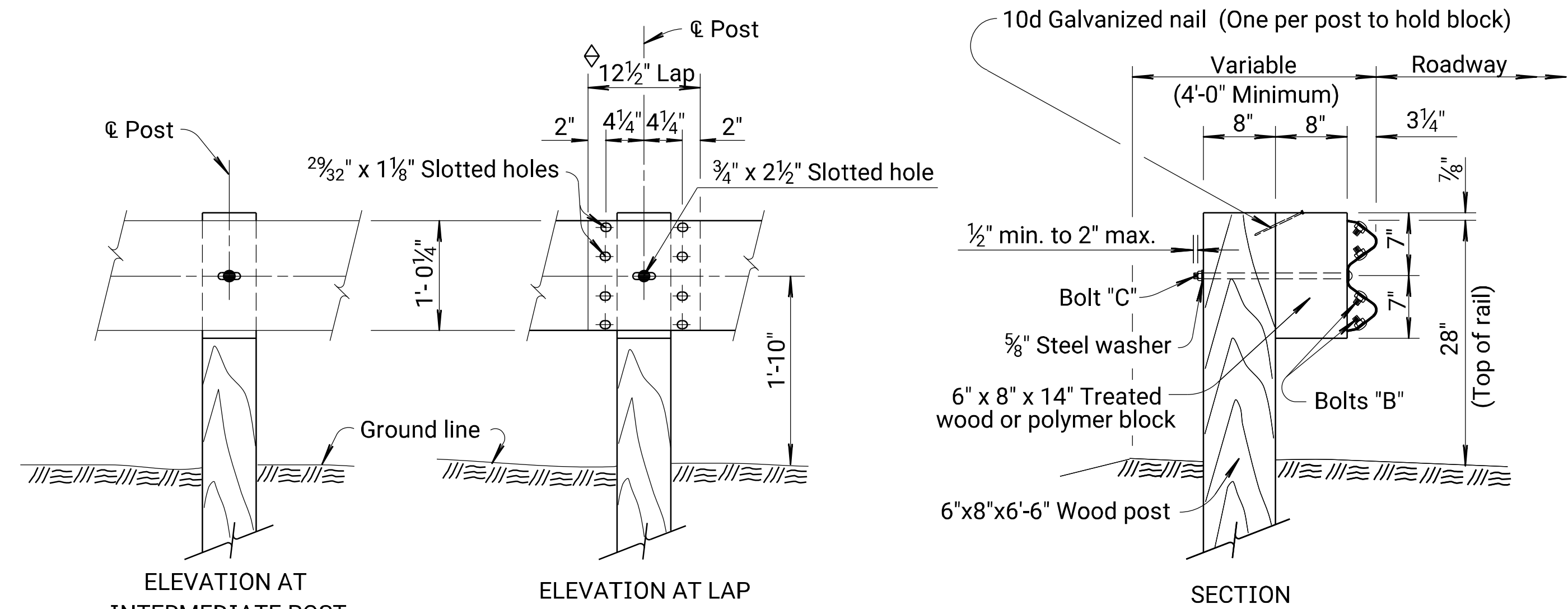
2	9-5-18	ADD. OMITTED POST AND TRANS. DETAILS	A.L.R.	T.T.R.	
1	6-5-18	INITIAL RELEASE	A.L.R.	T.T.R.	
NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
GUARDRAIL AUXILIARY DETAILS					
RD606					
FHWA APPROVAL	9-25-18	APP'D.	SCOTT W. KING		
DESIGNED	DETAILED	QUANTITIES	TRACED		
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.		

Notes to Designer: For posts installed in pavement thicker than 8" or posts installed in rock formations refer to AASHTO's Roadside Design Guide for details then revise this drawing and all supporting drawings appropriately.

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Plotted 8/30/2021



THRIE BEAM POST DETAILS



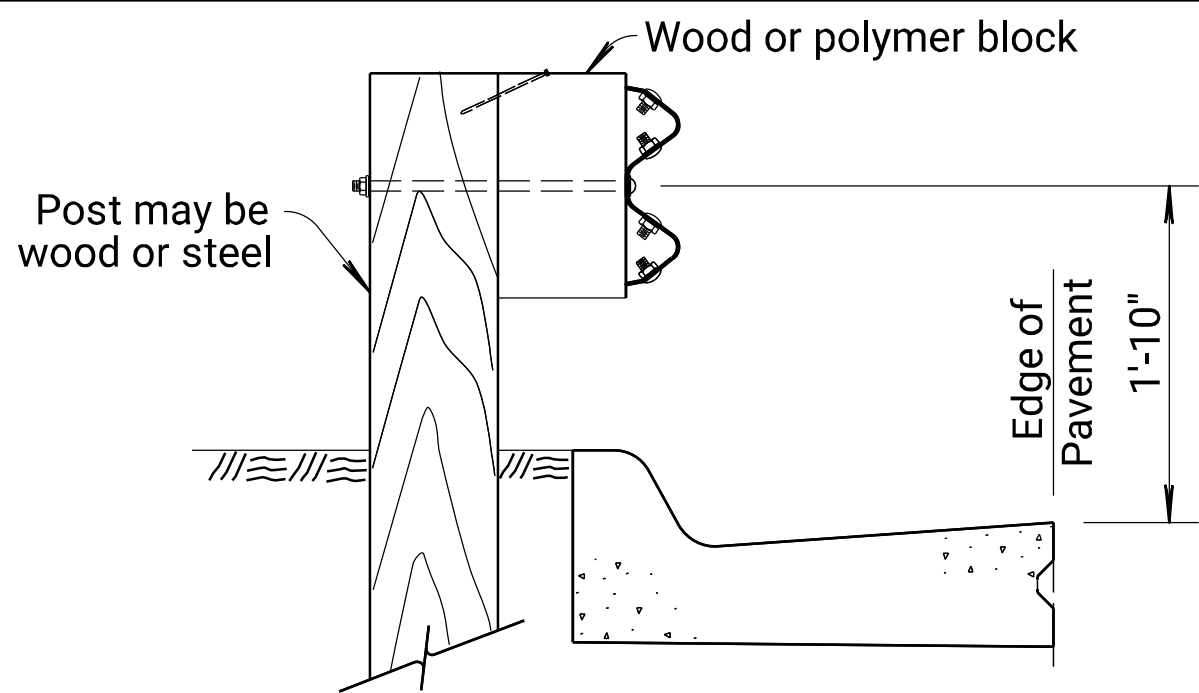
W-BEAM POST DETAILS

◊ Lap guardrail splices, including terminal connector, in the direction of traffic. Where traffic is temporarily carried in the opposite direction of final configuration, lap rail splices in the direction of permanent traffic.

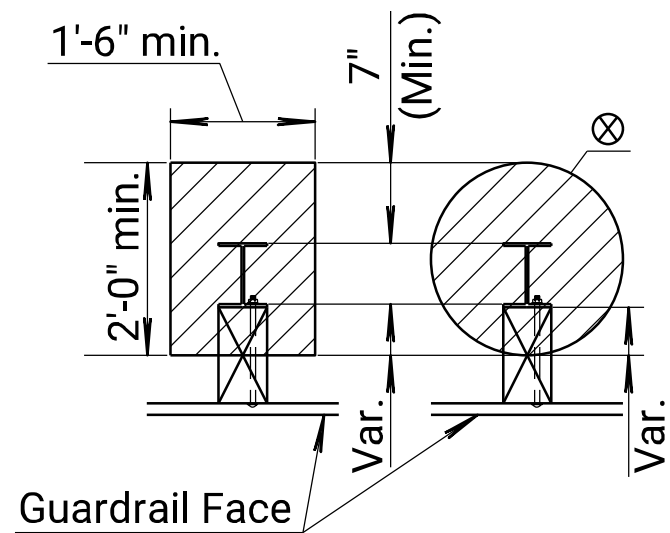
WOOD POSTS

WOOD POSTS REQUIRED,
NO OPTION

GENERAL NOTES (Wood Posts)
Give all wood posts and wood blocks a preservative treatment, see standard specifications. Thoroughly saturate all cuts, injuries and bolt holes on wood posts and blocks with preservative. Use only one type of preservative treatment on a project. Use S4S rectangular posts and wood blocks, see standard specifications. Use only one post/blockout type within guardrail run, this excludes the the guardrail end terminals. Set guardrail posts by digging or by driving. Use post caps to protect the post from crushing during driving operations. Contractor must notify Engineer at the earliest time when a non-removable manmade object (footing, pipe, etc.) is encountered and prevents installation of a full length post. Contractor must obtain Engineer approval prior to cutting post shorter than 6'-6". Approved polymer blockouts may be substituted for wood blockouts. Only one type of blockout is permitted on each guardrail installation. This excludes the guardrail end terminals unless certified by the manufacturer. All dimensions are nominal and are subject to manufacturing tolerances. Excavation including rock, shale, and other materials for erection of Guardrail is subsidiary to various bid items for which payment is made. Where guardrail posts are installed in pavement, form openings in the pavement for the guardrail posts.



DETAIL OF PLACEMENT AT CURB

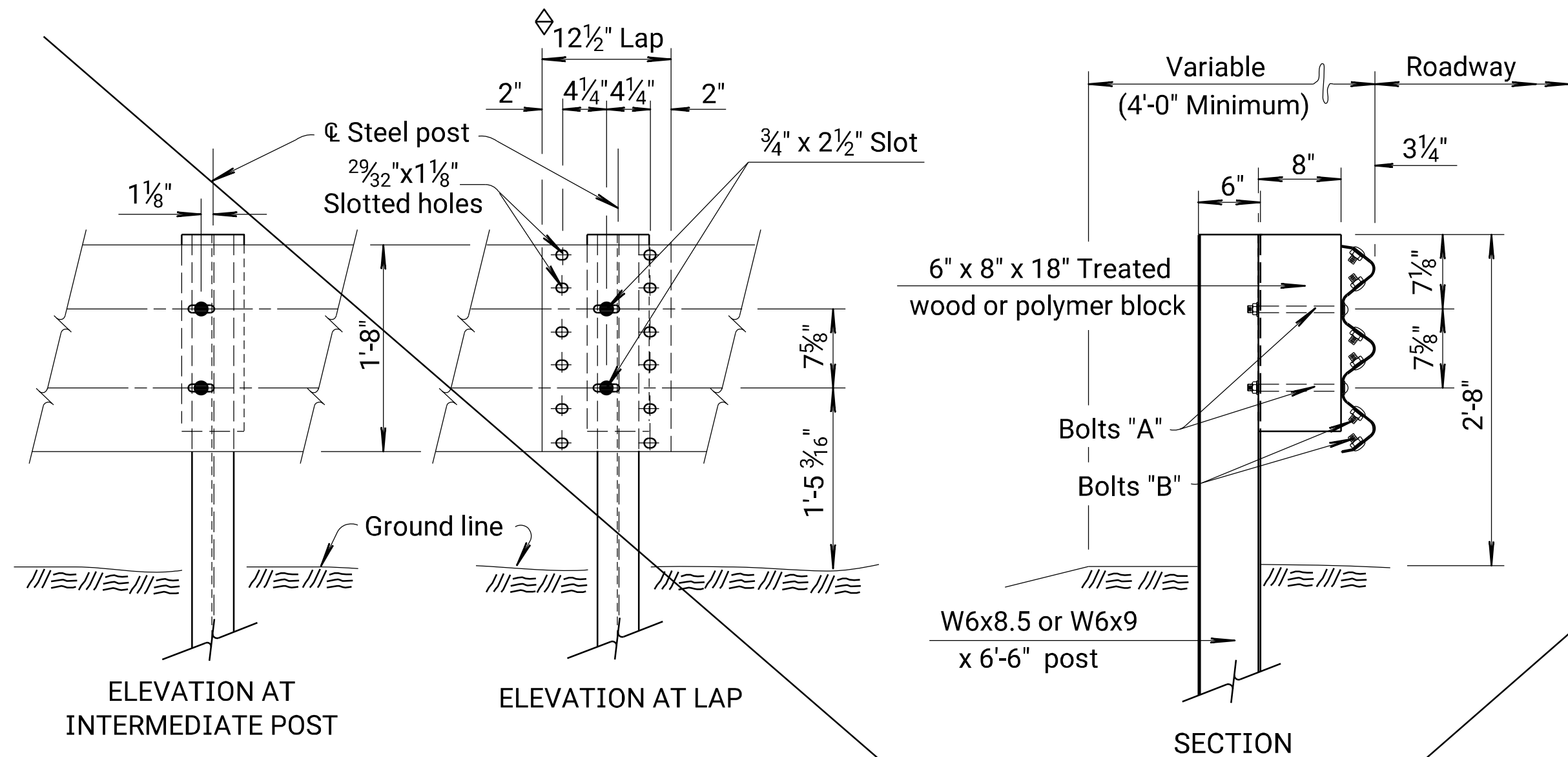


POSTS IN PAVEMENT PLAN
(ALTERNATE GEOMETRIES)
Applies to All Wood and All Steel Posts
(Steel Posts Shown)

- ▣ Slurry Grout (Low Strength). See KDOT's Standard Specifications
- ⊗ Diameter may vary from 1'-6" (min.) to 2'-0".

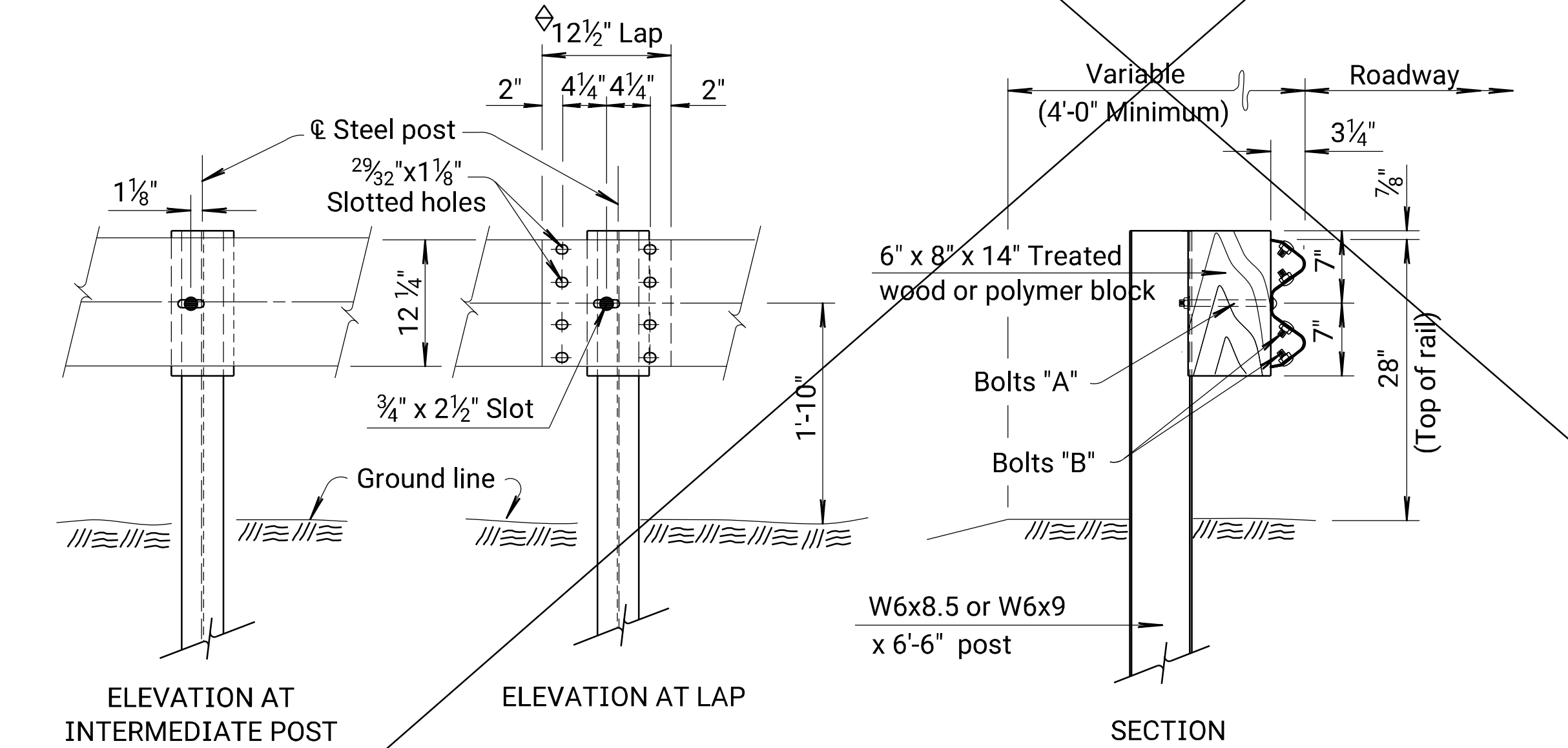
Note: Low Strength Grout must have a 28-day compressive strength of 120 psi or less. All work and materials related to posts in pavement are subsidiary to other guardrail bid items. Rectangular geometry shown in Posts in Pavement detail. Circular geometry, as shown on this sheet, may be used at the Contractor's option.

BOLT SIZE SCHEDULE	
Bolt	L
A	8 1/2"
B	1 1/4"
C	18"



THRIE BEAM POST DETAILS

◊ Lap guardrail splices, including terminal connector, in the direction of traffic. Where traffic is temporarily carried in the opposite direction of final configuration, lap rail splices in the direction of permanent traffic.

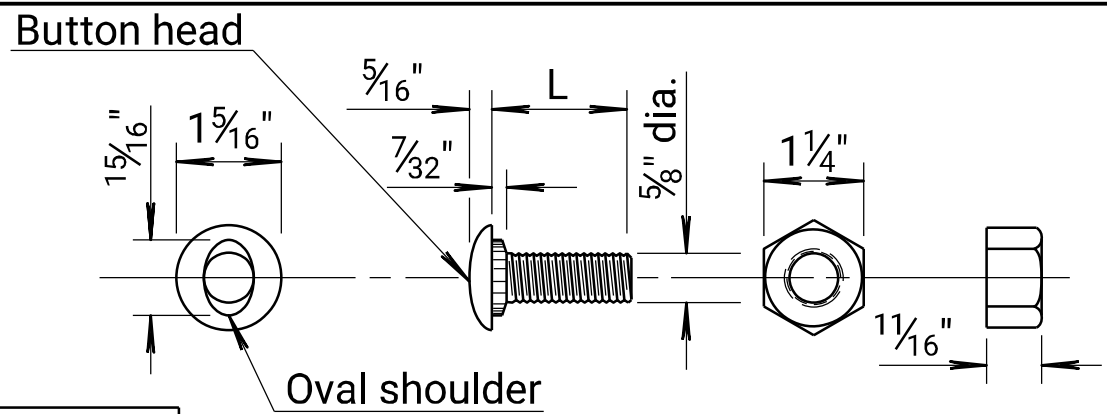


W-BEAM POST DETAILS

STEEL POSTS

GENERAL NOTES (Steel Posts)

Use grade of steel for steel posts that meets the requirements of the standard specifications. Hot dip galvanize the posts after fabrication, see standard specifications. Use only one post/blockout type within guardrail run, this excludes the guardrail end terminals. For wood/polymer blockout requiements see standard specifications. Approved polymer blockouts may be substituted for wood blockouts. Only one type of blockout is permitted on each guardrail installation. This excludes the guardrail end terminals. Set guardrail posts by digging or by driving. Use post caps to protect the post from crushing during driving operations. Contractor must notify Engineer at the earliest time when a non-removable manmade object (footing, pipe, etc.) is encountered and prevents installation of a full length post. Contractor must obtain Engineer approval prior to cutting post shorter than 6'-6" except as allowed on Standard Drawing RD617. All dimensions are nominal and are subject to manufacturing tolerances. Excavation including rock, shale, and other materials for erection of Guardrail is subsidiary to various bid items for which payment is made. Where guardrail posts are installed in pavement, form openings in the pavement for the guardrail posts.



BOLT & NUT DETAILS

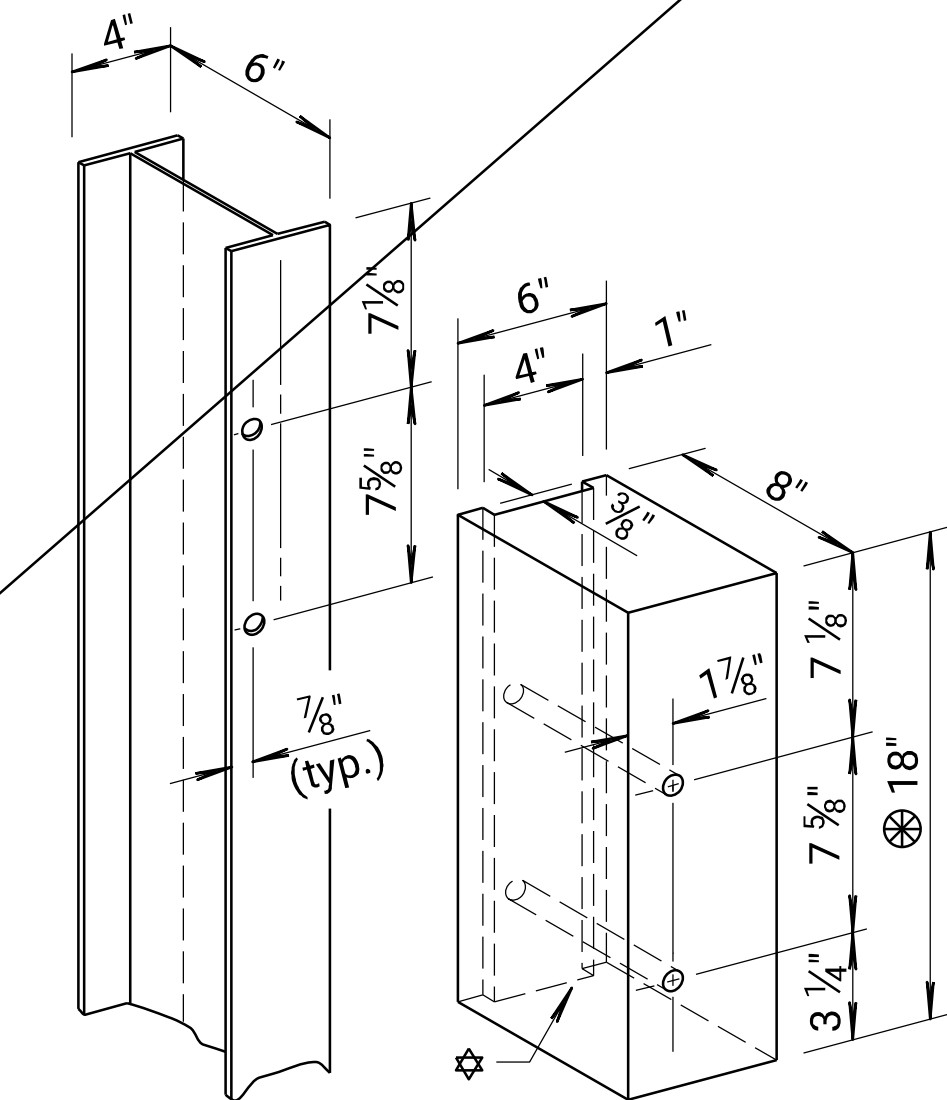
Galvanize all bolts, nuts, and washers in accordance with the KDOT's Standard Specifications.

13	9-5-18	Added Det., Posts In Pavement	A.L.R.	T.T.R.
12	12-14-10	Revised notes, 28" w-be	S.W.K.	J.O.B.
11	6-30-04	Remove steel blockout and notes	S.W.K.	J.O.B.
10	7-15-02	Add polymer block-out alternate	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APP'D

GUARDRAIL POST
DETAILS

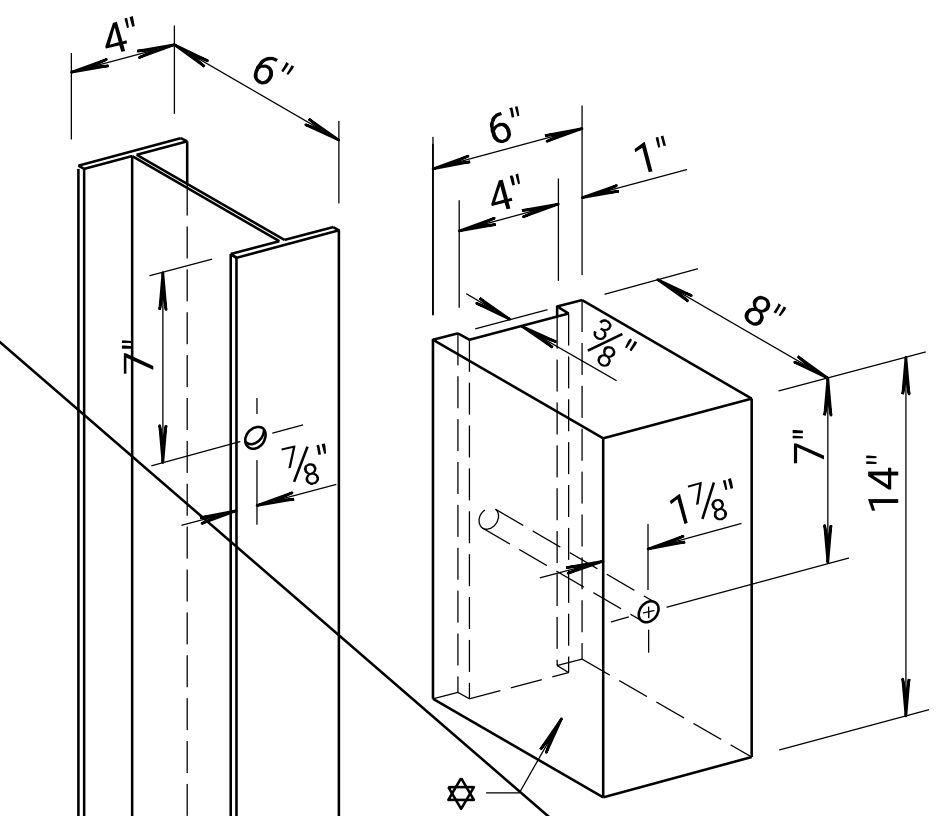
RD611			
FHWA APPROVAL	9-25-18	APP'D.	SCOTT W. KING
DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

⊗ See Standard Drawing RD613 for Thrie-Beam Transition Section Details.



Note: All holes 1 3/16" dia.

THRIE BEAM
HOLE PUNCHING DETAILS



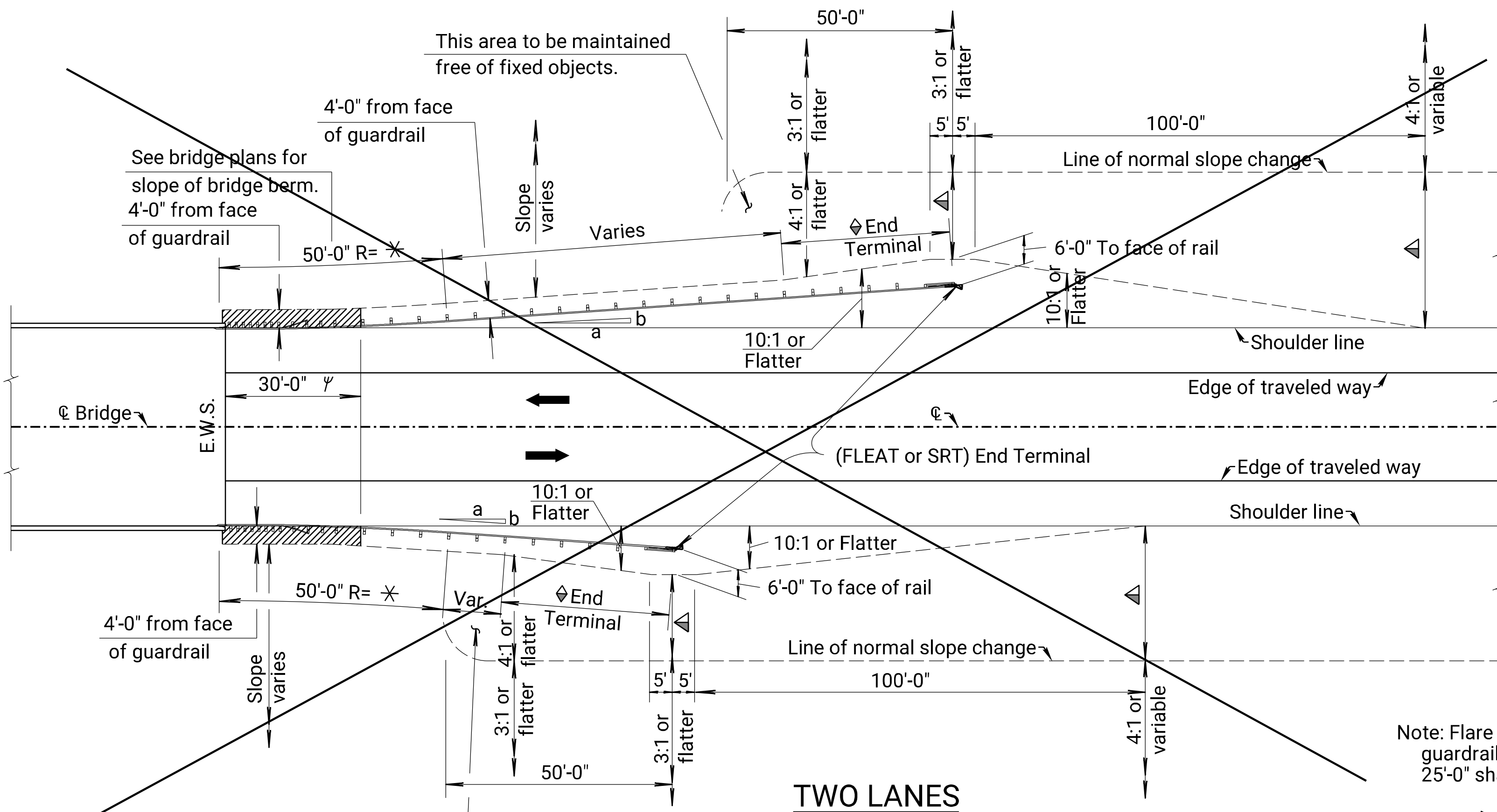
Note: All holes 1 3/16" dia.

"W" BEAM
HOLE PUNCHING DETAILS

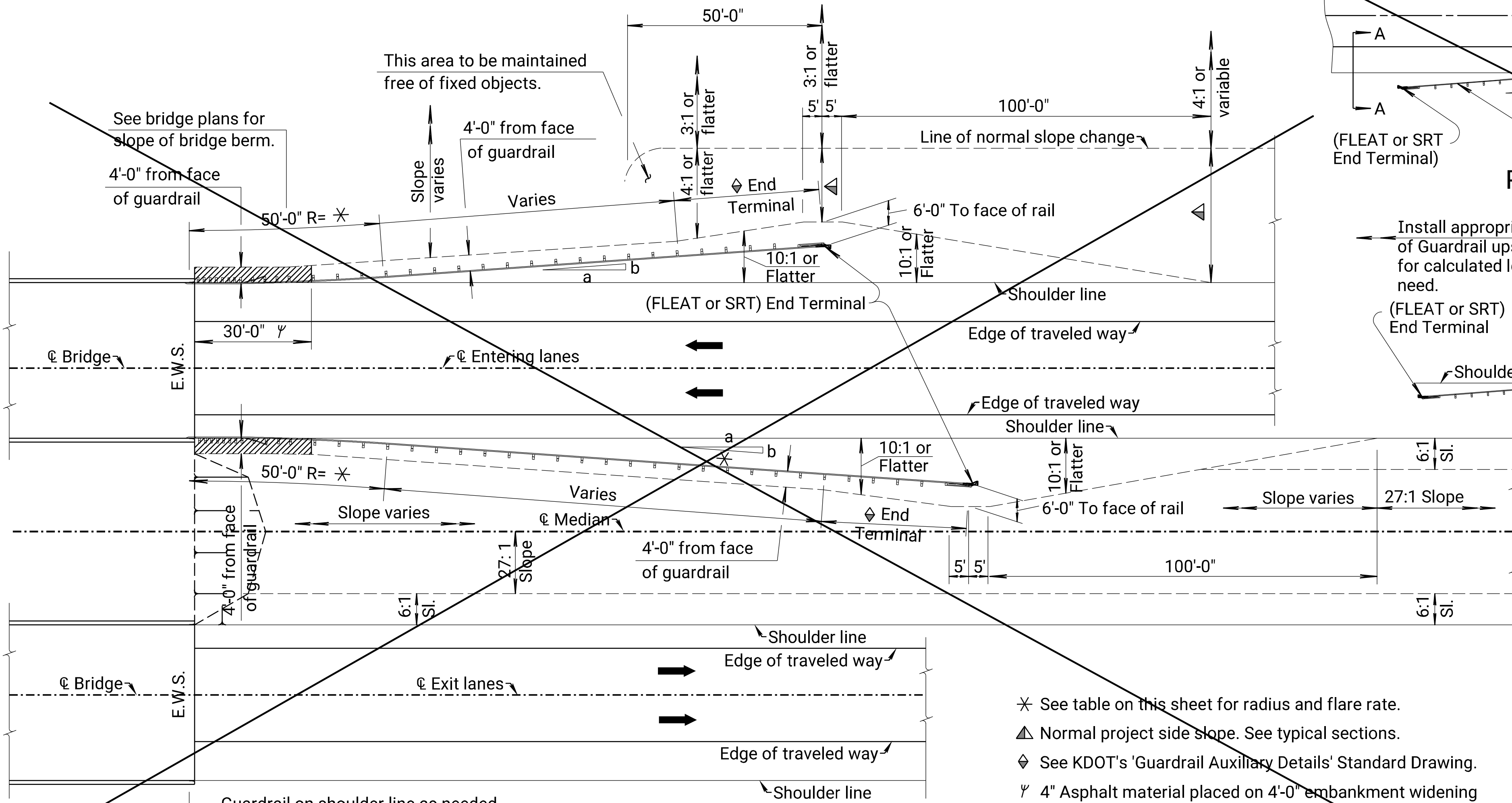
☆ Non-Metallic (Polymer) or
Treated Wood Block

Notes to Designer: Guardrail length of need shall be determined in accordance with the AASHTO Roadside Design Guide using $L_1 = 25'$ for flare rate of a:b and $L_1 = 12.5'$ for flare rate of 2a:b for a typical installation as shown on this sheet. This sheet shall be used when the flared guardrail design for typical layout shown (FLEAT or SRT) is selected. Material for asphalt widening shall be included in the plan quantities.

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Plotted 8/30/2021



TWO LANES

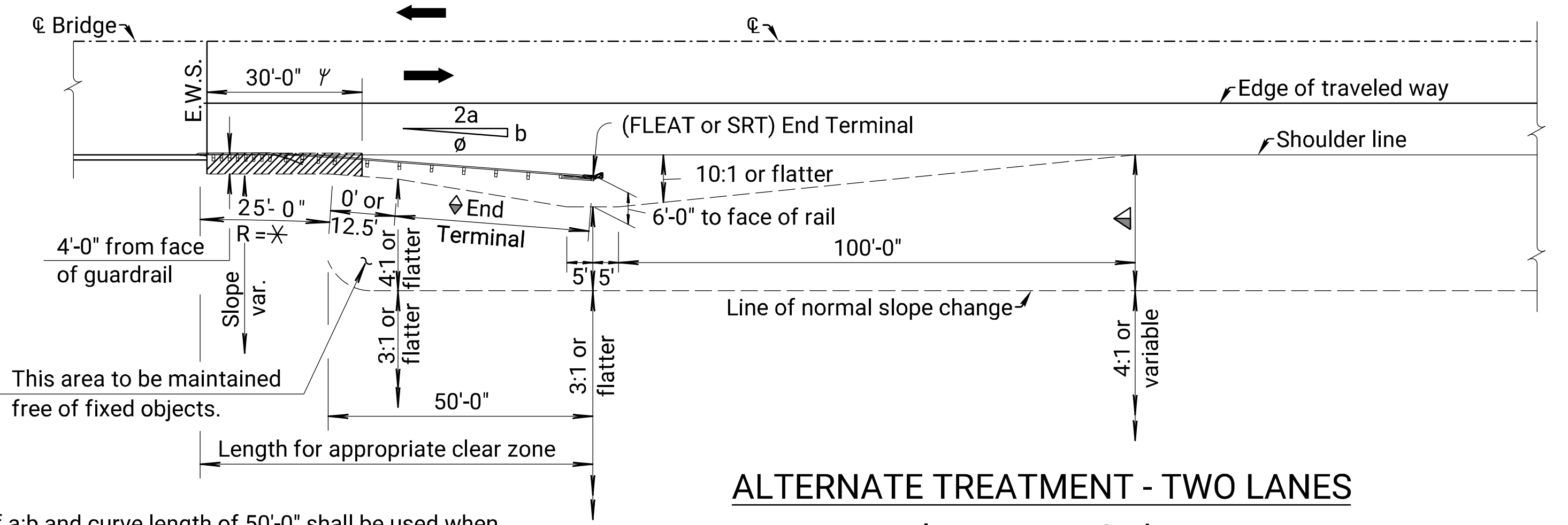


FOUR LANES - DIVIDED

- * See table on this sheet for radius and flare rate.
- ▲ Normal project side slope. See typical sections.
- ◆ See KDOT's 'Guardrail Auxiliary Details' Standard Drawing.
- ∩ 4" Asphalt material placed on 4'-0" embankment widening unless flume inlet and slope drain is constructed.

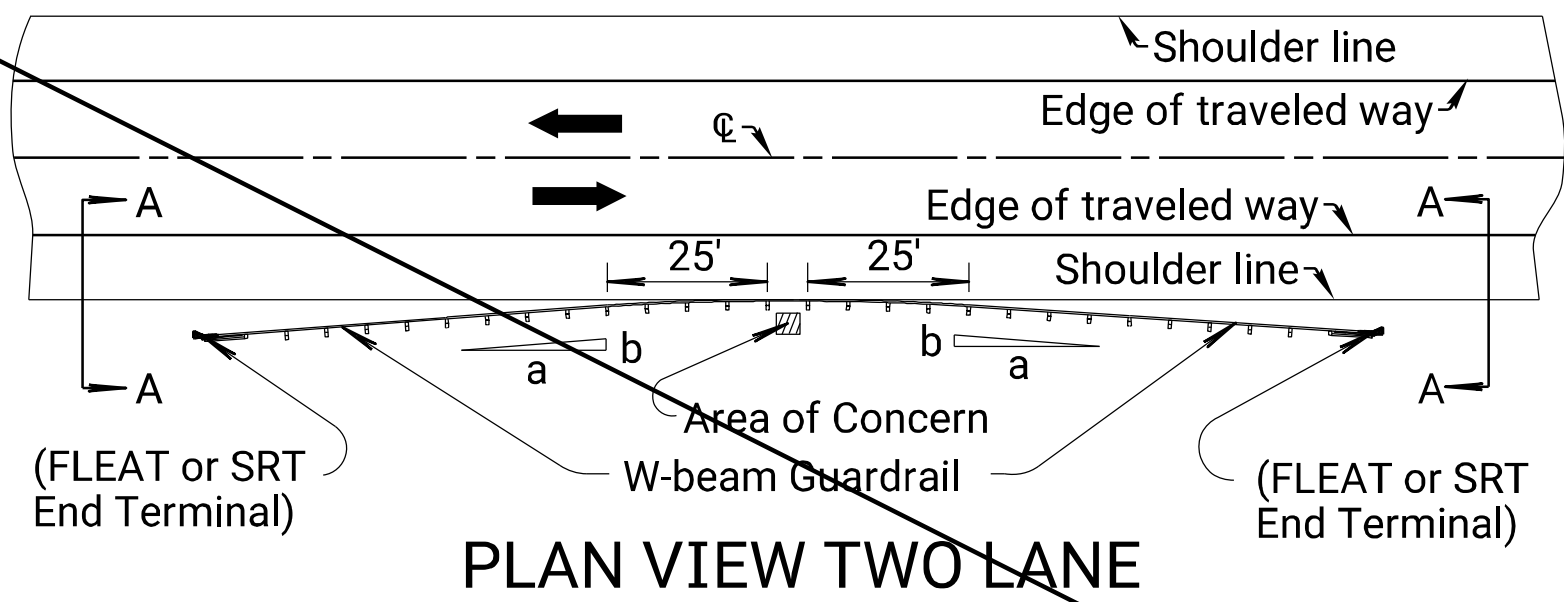
DESIGN PARAMETERS				
Design Speed (mph)	Flare Rate (a:b)	Radius (R)	Flare Rate (2a:b)	Radius (R)
70	15:1	375.55'	30:1	375.14'
60	14:1	350.59'	26:1	325.16'
55	12:1	300.69'	24:1	300.17'
50	11:1	275.76'	21:1	262.70'
45	10:1	250.83'	18:1	225.23'
40	8:1	201.04'	16:1	200.26'

GENERAL NOTE
For guardrail and rubrail sections, details, and general notes see KDOT's 'W-Beam with Rubrail Bridge Approach Transition Details' Standard Drawings. For post details see KDOT's 'Guardrail Post Details' Standard Drawings.
The ratio of a:b may be specified as zero for long runs of guardrail in high fill areas.
Widening, slopes & transition for Four Lane will be similar to that shown on two lane detail.

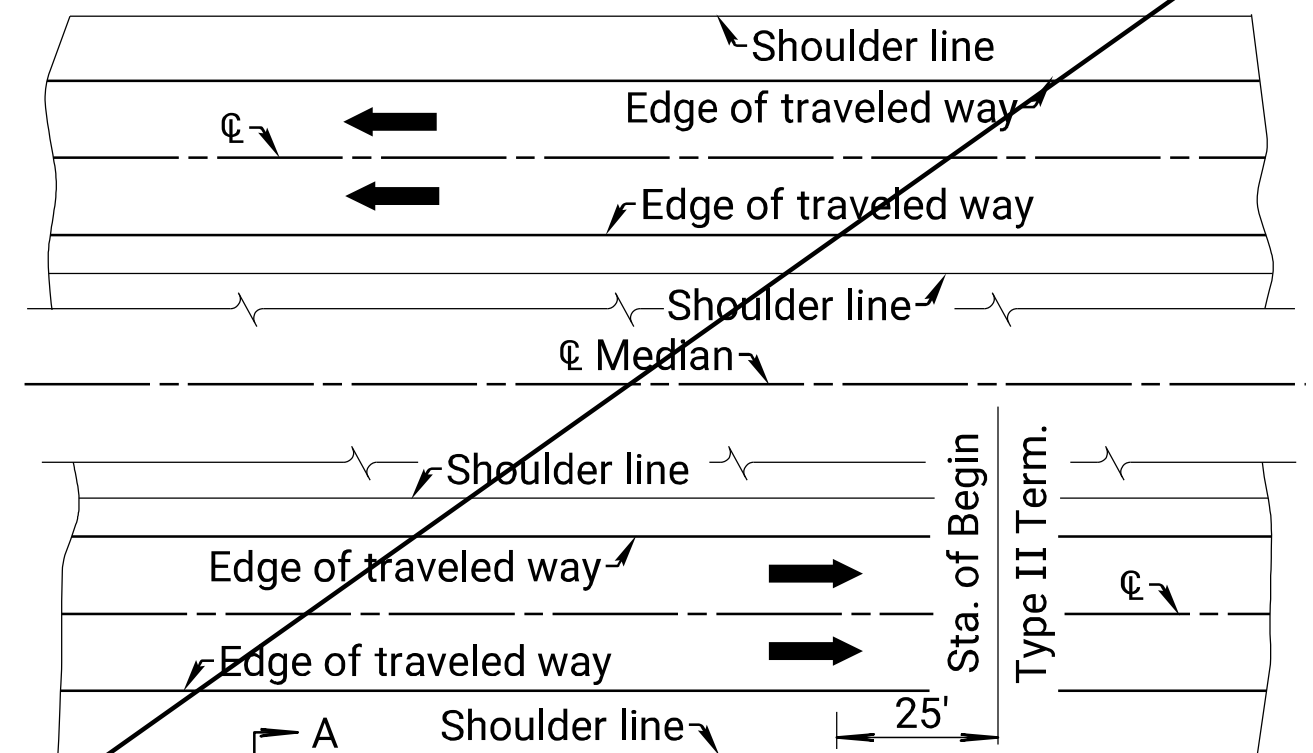


ALTERNATE TREATMENT - TWO LANES

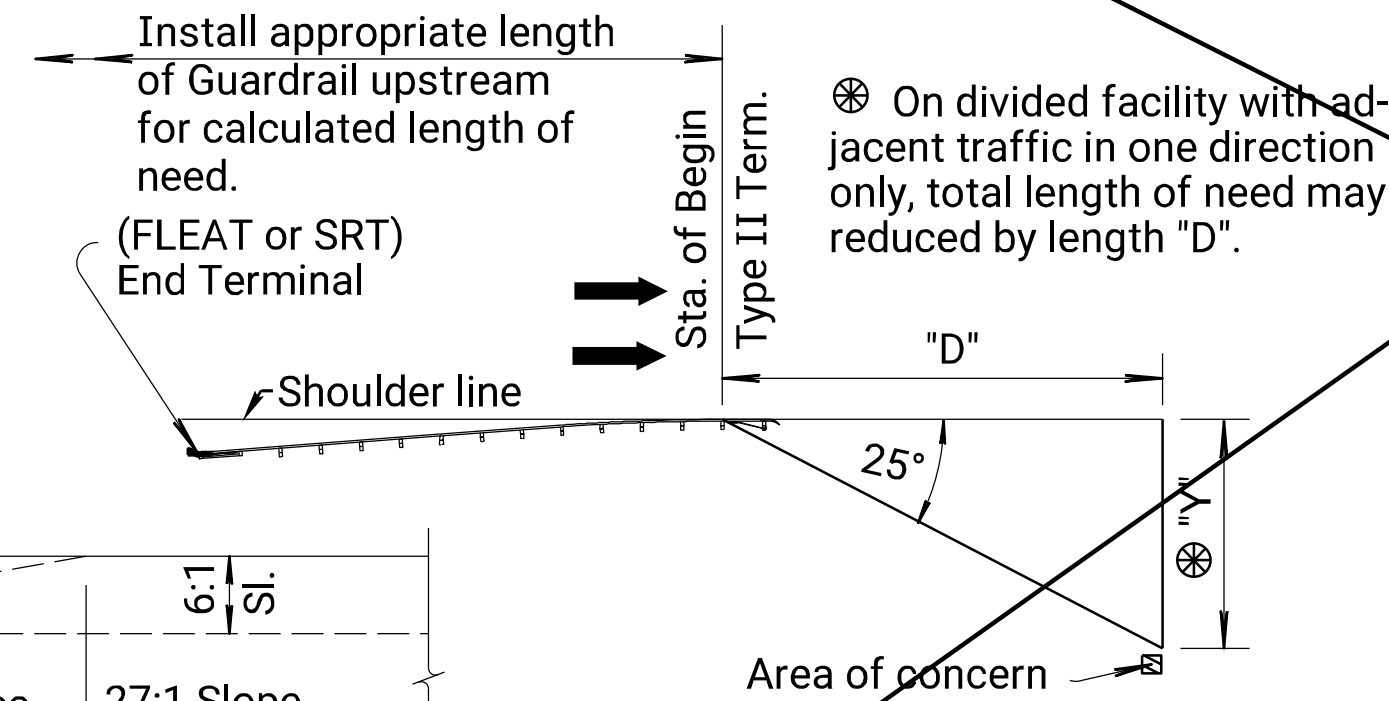
Flare Rate = 2a:b
(GUARDRAIL LENGTHS OF 62.5' AND 75')



PLAN VIEW TWO LANE



PLAN VIEW FOUR LANE



DETAILS OF GUARDRAIL PROTECTION AT ROADSIDE OBSTACLE

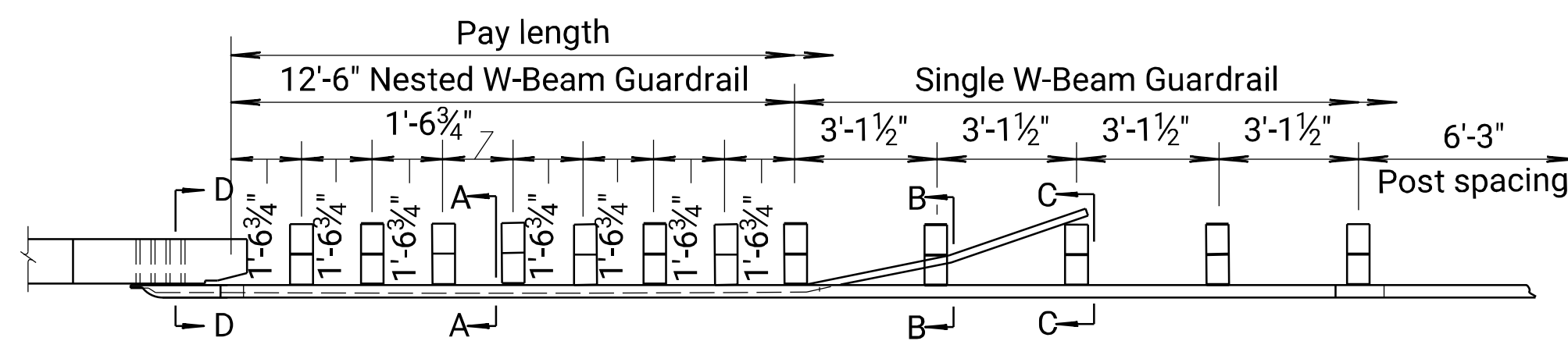
WOOD POSTS REQUIRED,
NO OPTION

ENLARGEMENT - AREA OF CONCERN

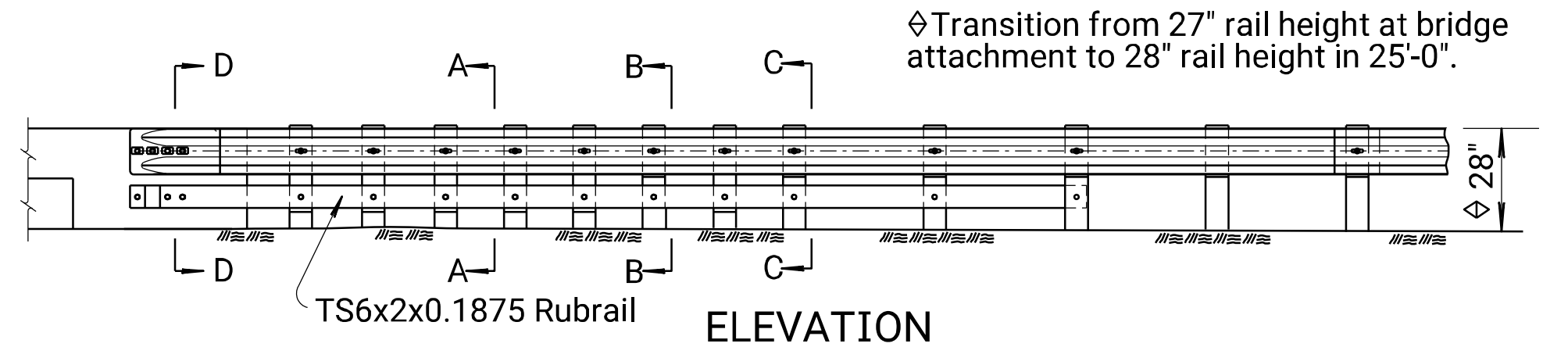
8	6-5-18	Removed Flare-beyond-the-Flare	A.L.R.	T.T.R.
7	5-15-17	Removed X-LITE	A.L.R.	S.W.K.
6	7-2-09	Added roadside obstacle details	S.W.K.	J.O.B.
5	1-10-07	Changed bituminous to asphalt	S.W.K.	J.O.B.
NO.	DATE	REVISIONS	BY	APPD

KANSAS DEPARTMENT OF TRANSPORTATION			
W-BEAM WITH RUBRAIL BRIDGE APPROACH TRANSITION TYPICAL ALIGNMENTS (FLARED)			
RD615A			
FHWA APPROVAL	6-19-18	APPD. SCOTT W. KING	
DESIGNED	DETAILED	QUANTITIES	TRACED
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.

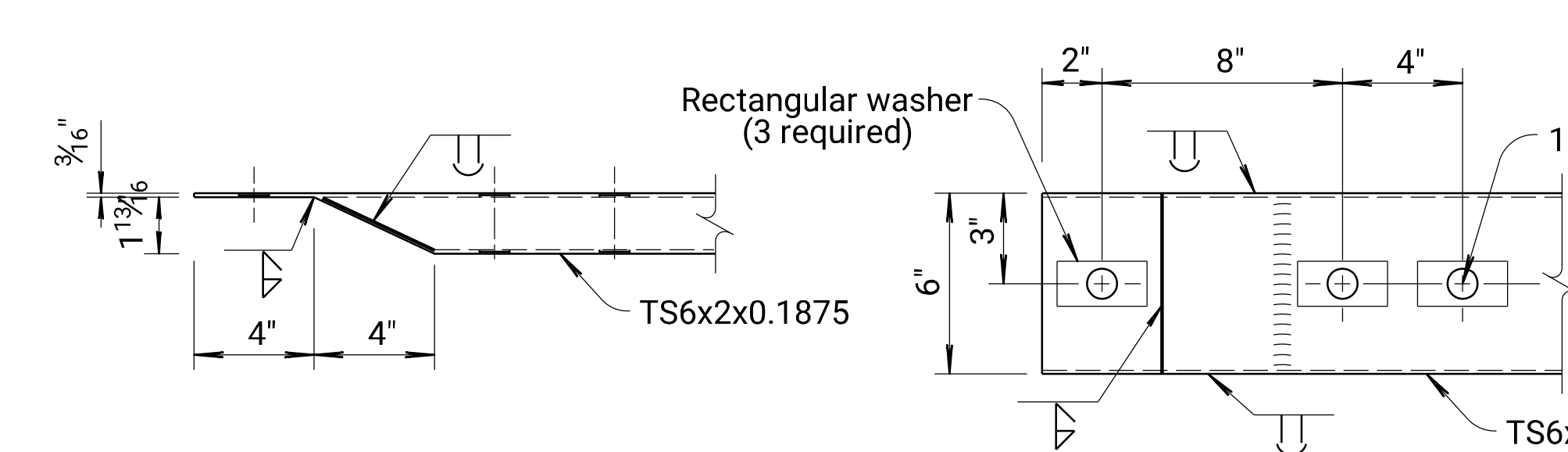
Drawn By : untitled
File : M:\20-20-1458M\CAD\Drawing Set\08-rd616.dgn
Plotted 8/30/2021



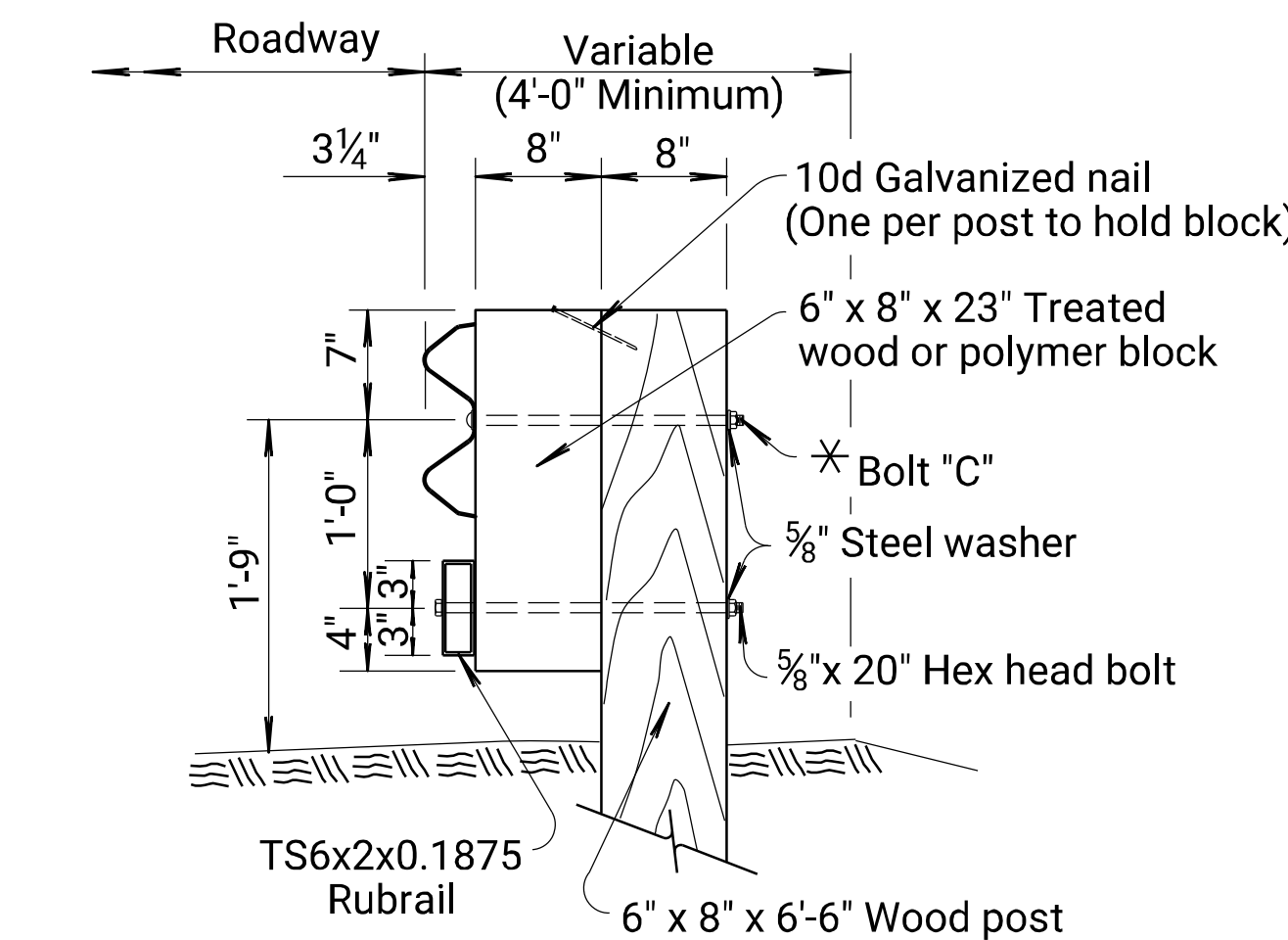
PLAN



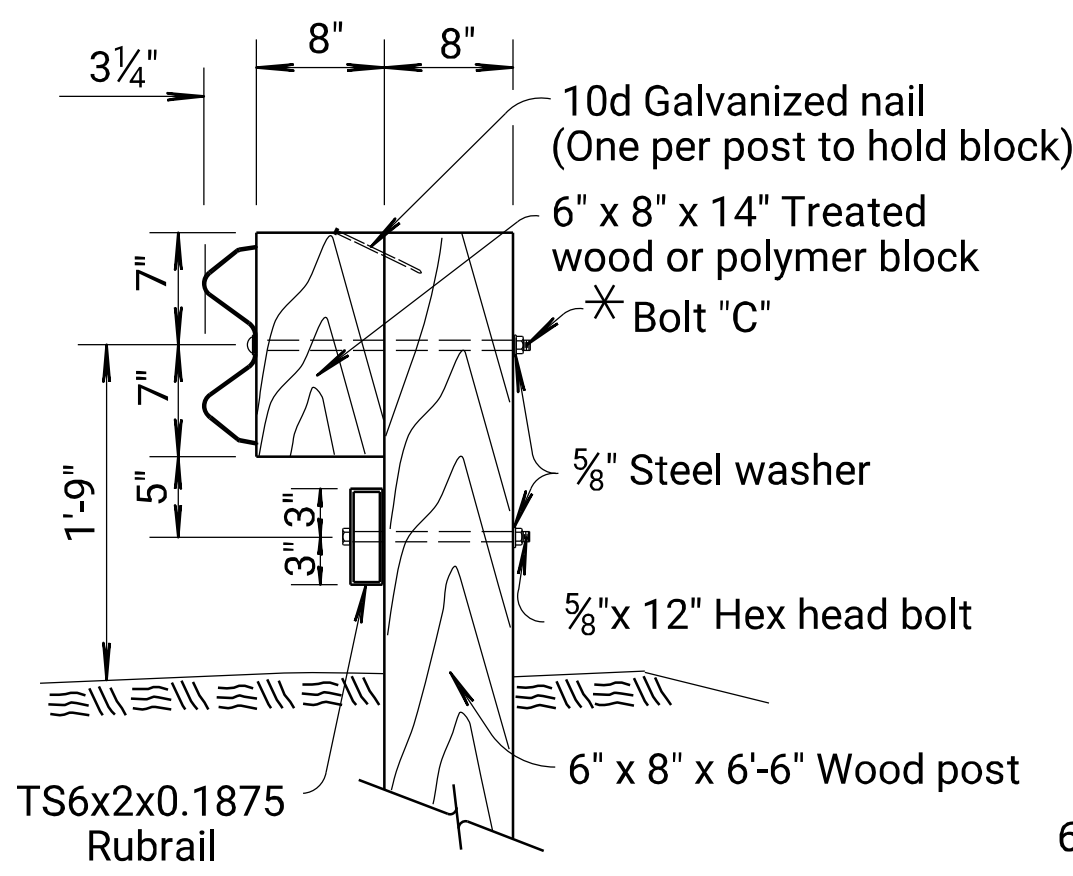
ELEVATION



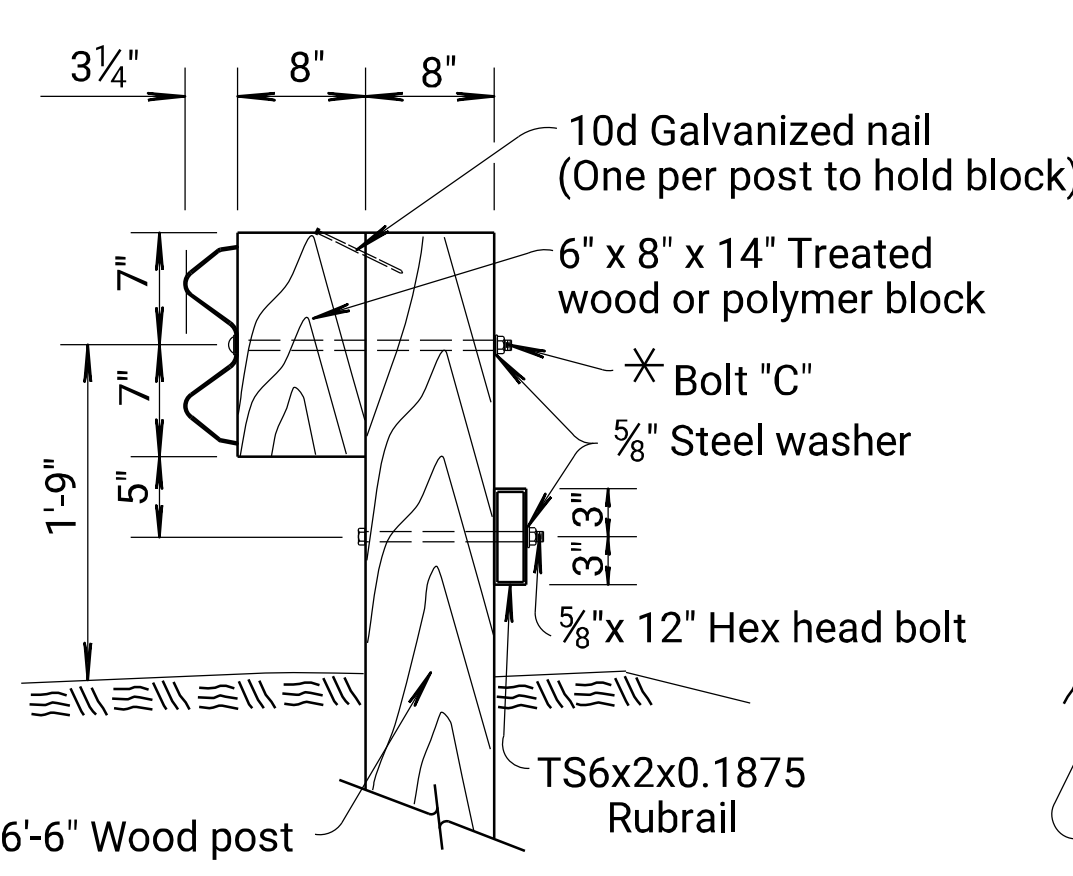
TYPICAL END RUB RAIL DETAILS



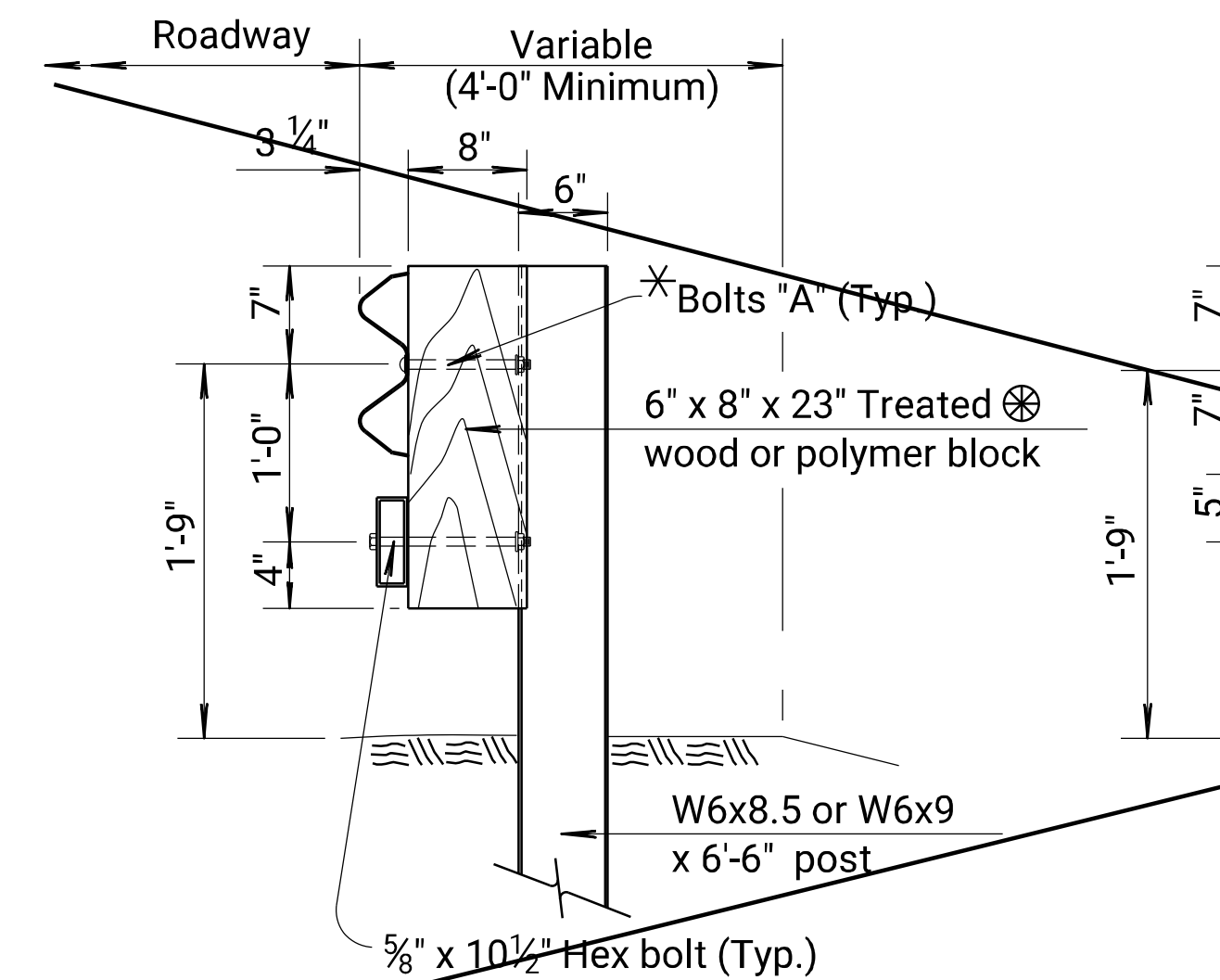
SECTION A-A



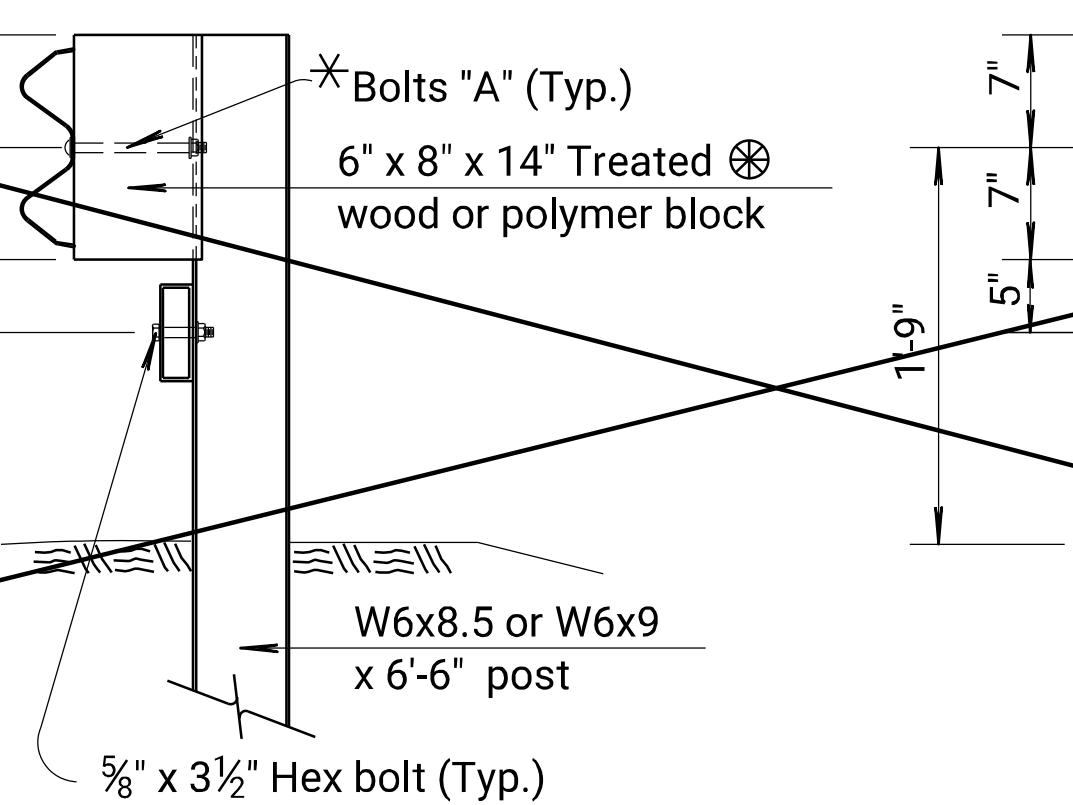
SECTION B-B



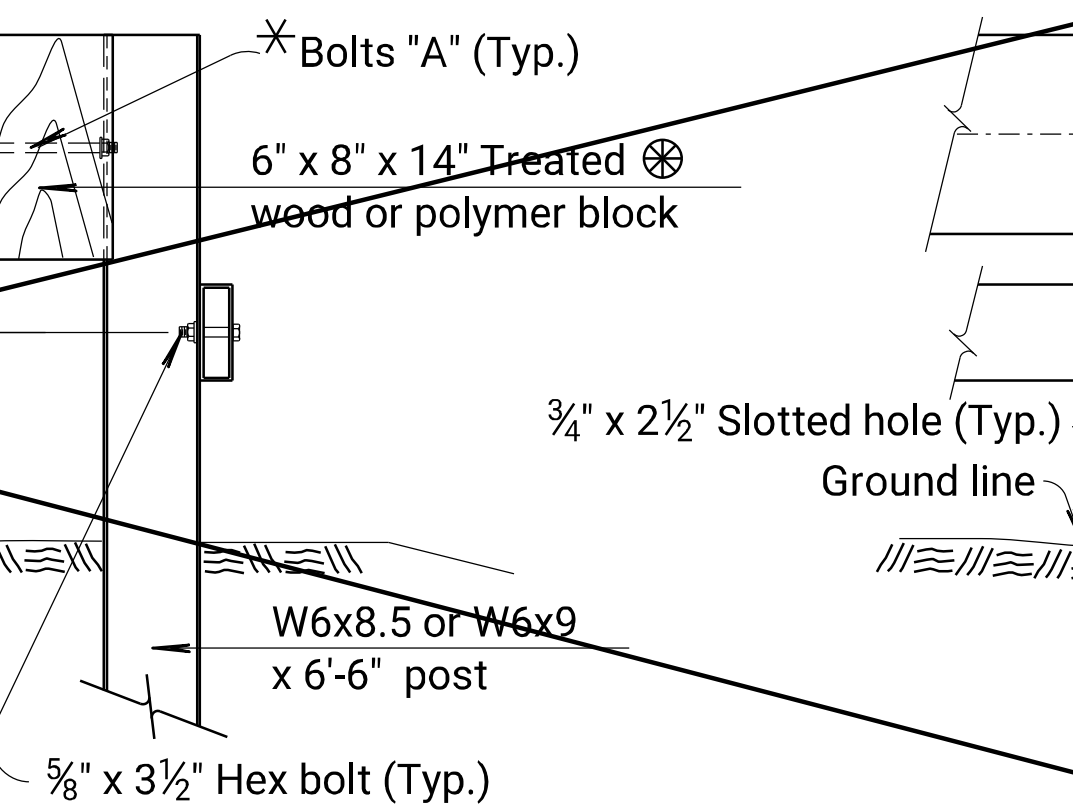
SECTION C-C



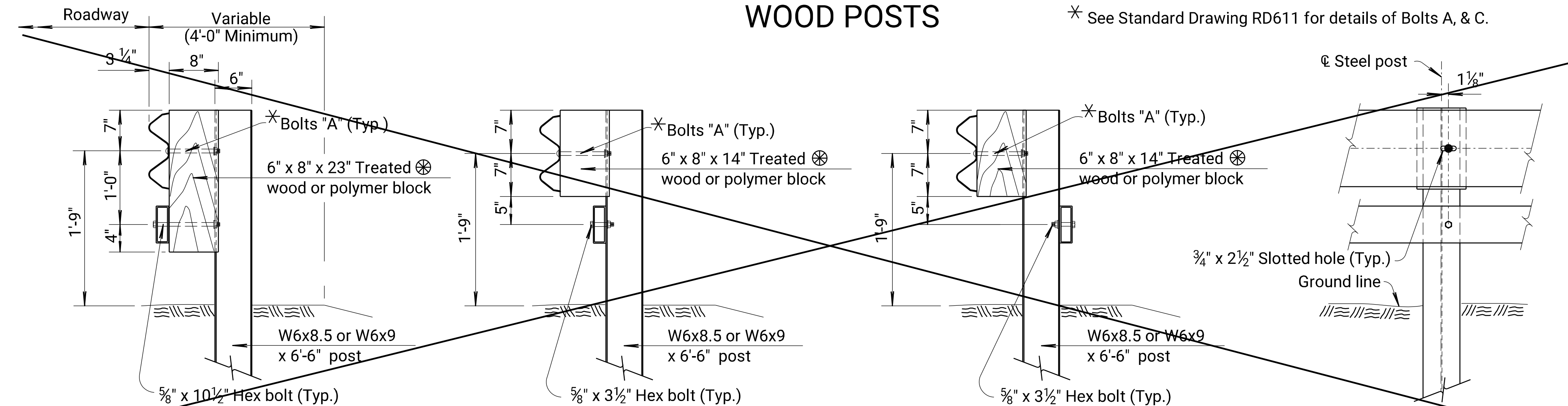
SECTION A-A



SECTION B-B



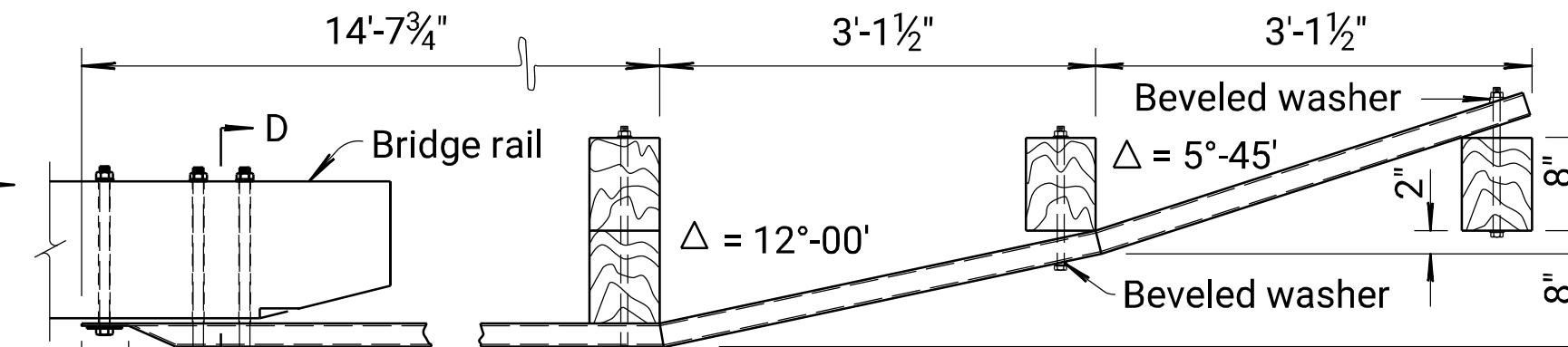
SECTION C-C



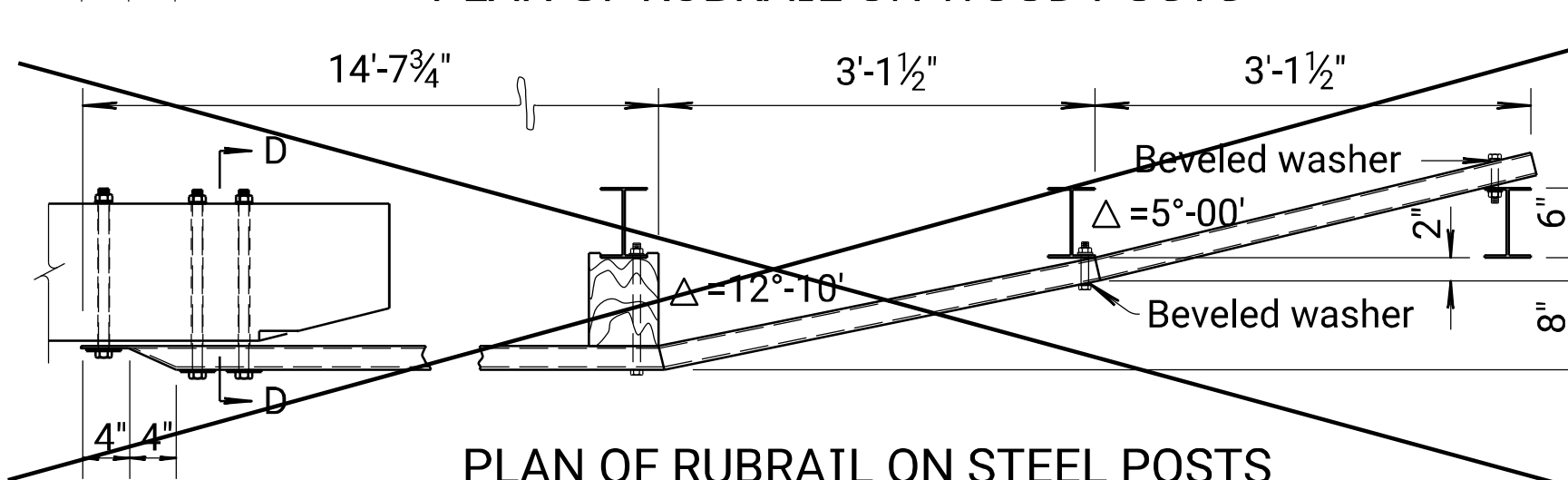
ELEVATION
WITH RUBRAIL

STEEL POSTS

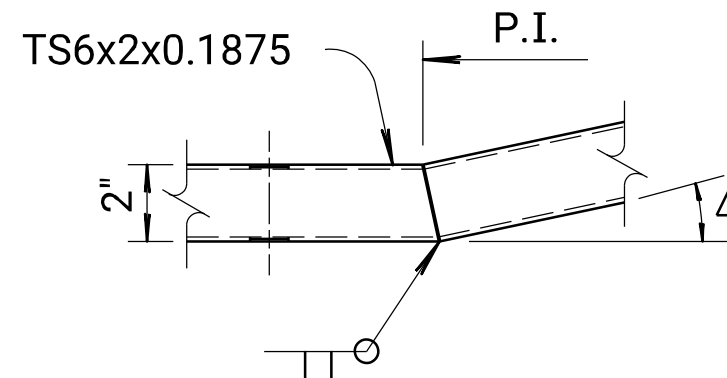
⊗ Blocks used with steel posts shall be grooved to fit over the flange of the post and may be Wood or Polymer.



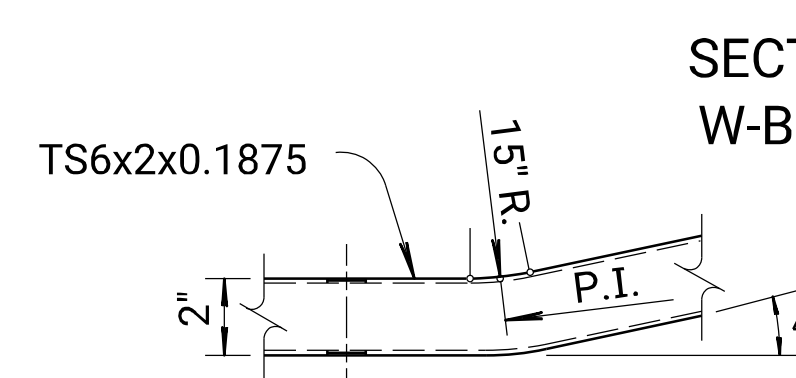
PLAN OF RUBRAIL ON WOOD POSTS



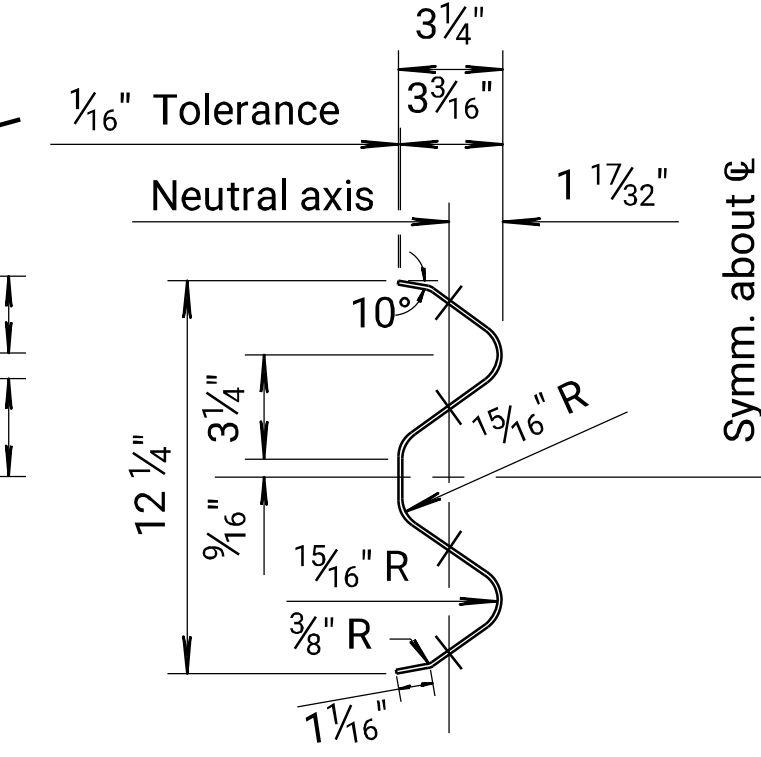
PLAN OF RUBRAIL ON STEEL POSTS



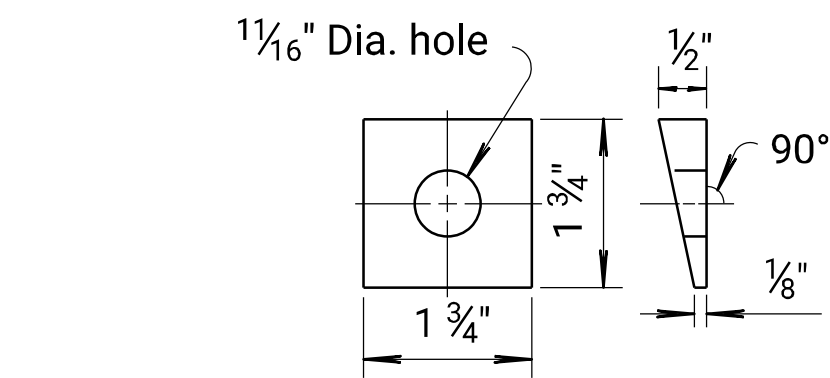
SHOP WELDED OPTION



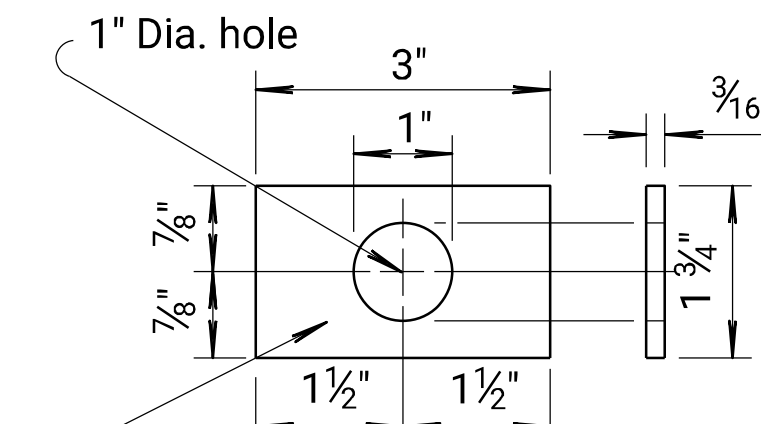
SHOP BENT OPTION



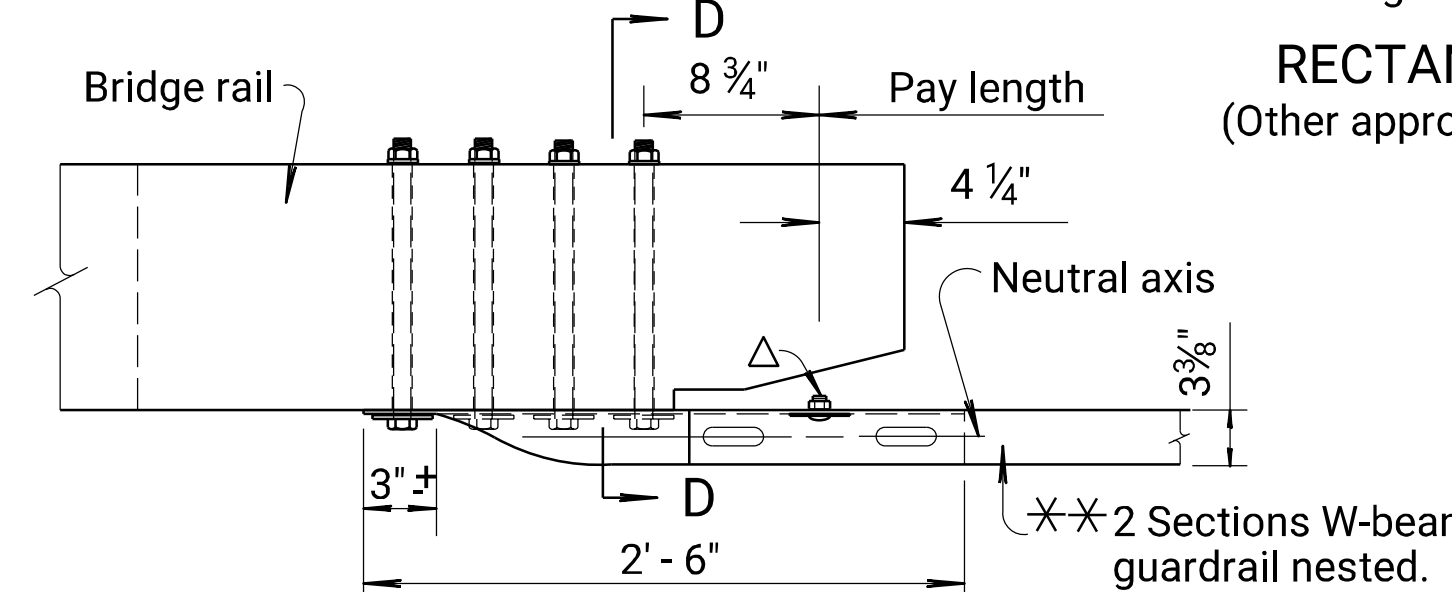
SECTION THRU TYPICAL
W-BEAM RAIL ELEMENT



BEVELED WASHER

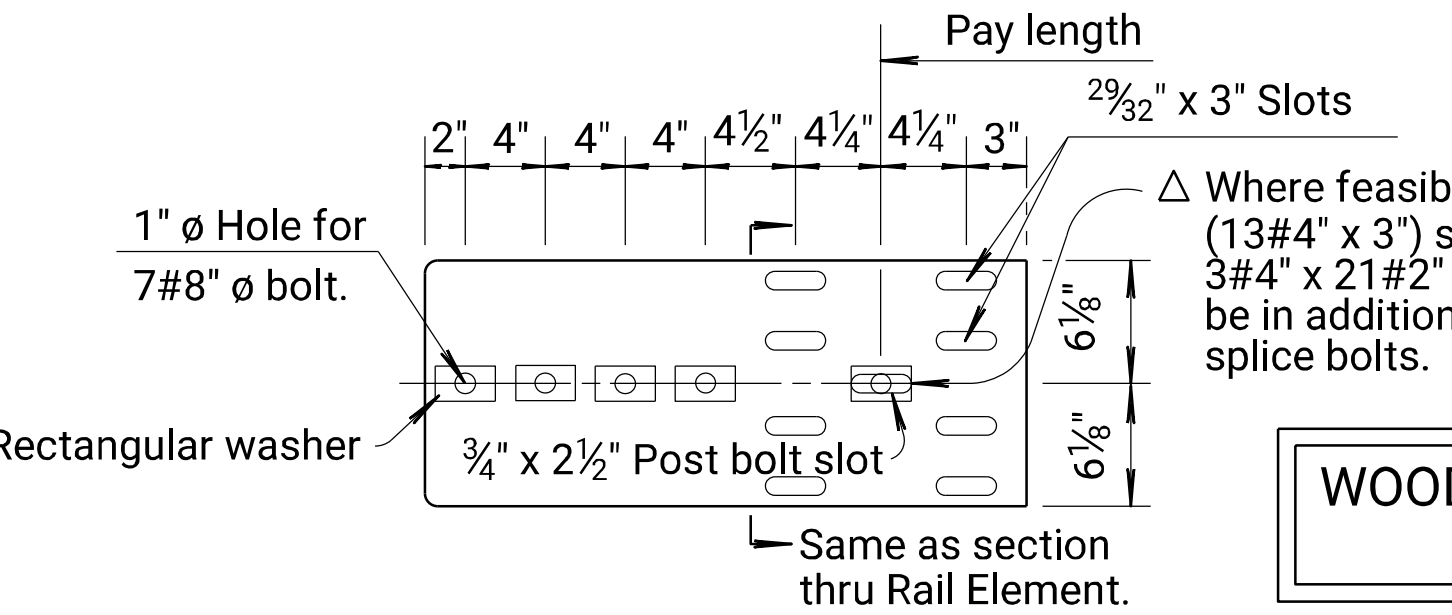


RECTANGULAR WASHER
(Other approved washer may be used.)

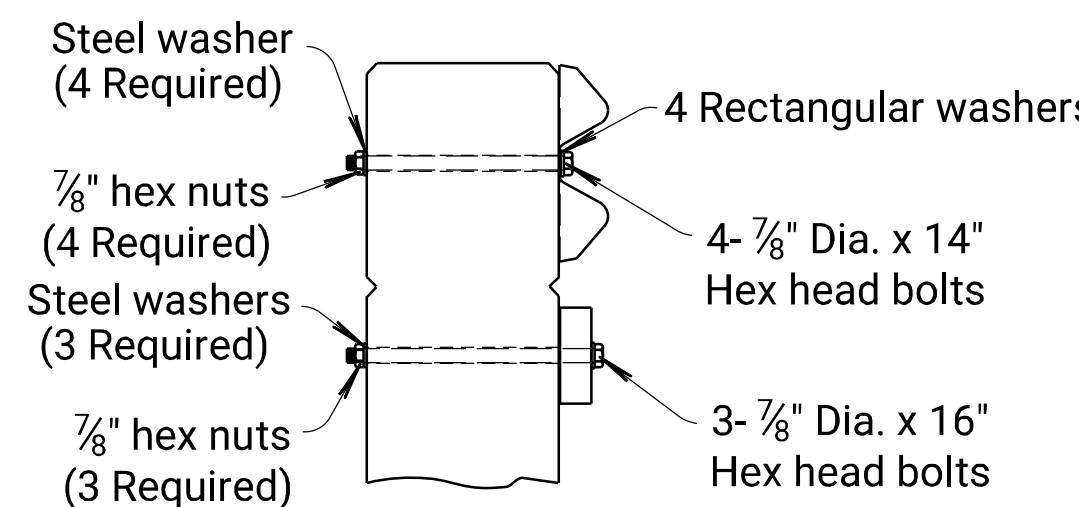


PLAN SPECIAL END SHOE

⌘⌘ One section of the two shall be considered as subsidiary to the bid item "Steel Plate Guardrail".



ELEVATION SPECIAL END SHOE



SECTION D-D

NO.	DATE	REVISIONS	BY	APP'D
14	12-14-10	Revised notes, 28" rail height	S.W.K.	J.O.B.
13	4-02-08	Removed Galvanized callout	S.W.K.	J.O.B.
12	2-06-07	Corrected spelling error	S.W.K.	J.O.B.

KANSAS DEPARTMENT OF TRANSPORTATION

W-BEAM WITH RUBRAIL BRIDGE APPROACH TRANSITION DETAILS

RD616

FHWA APPROVAL 1-1 1-1 1		APP'D. James O. Brewer	
DESIGNED	DETAILED	QUANTITIES	TRACED Bowser
DESIGN CK.	DETAIL CK.	QUAN.CK.	TRACE CK. King

Plotted By: \$\$\$\$USERNAME\$\$\$

Plot Location: \$UNIT\$

File: \$\$\$\$DGN\$SPEC\$

Plot Date: \$\$\$\$SYTIME\$\$\$

20-1458M	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
	KANSAS	43 C-5078-01	2022	9	44

SUMMARY OF QUANTITIES													
Item Location	Excavation		Concrete		Reinforcing Steel (Grade 60)	* Piles (Steel) (HP 10x42)	Test Piles (Special) (HP10x42)	Cast Steel Pile Points	Contractor Furnished PDA	Slope Protection (Shot Rock)	Geotextile Fabric		
	Class I	Class II	(Grade 4.0) (AE) (SW)	(Grade 4.0) (AE)									
	Cu. Yds.	Cu. Yds.	Cu. Yds.	Cu. Yds.	Lbs.	Lin. Ft.	Lin. Ft.	Ea.	Ea.	Cu. Yds.	Sq. Yds.		
Abutment No. 1	41	—	**	—	**	261.6	—	4	—	117.7	62.3		
Pier No. 1	—	45	—	30.1	2,446	378.0	—	6	1	—	—		
Pier No. 2	—	45	—	30.1	2,446	320.0	74	6	1	—	—		
Abutment No. 2	41	—	**	—	**	251.2	—	4	—	162.9	59.4		
Substr. Total	82	90	—	60.2	4,892	1210.8	74	20	2	280.6	121.7		
Superstr. Total	—	—	219.8	—	62,433	—	—		—	—	—		
Total	82	90	219.8	60.2	67,330	1211	74	20	2	281	122		

** Quantities are included in the Superstructure Total Quantity.

† Summary of Piling

Abutment No. 1 4 @ 65.4’
Pier No. 1 6 @ 63.0’
Pier No. 2 5 @ 64.0’
Abutment No. 2 4 @ 62.8’

◆ Summary of Test Piling

Pier No. 2 1 @ 74.0’

*NOTE: Only HP10X42 Steel Piles shall be used on this structure.

GENERAL NOTES

CHANNEL IMPROVEMENT AND EXCAVATION: The Contractor shall excavate the channel and complete the embankment at the bridge site to the limits shown prior to the construction of the new bridge.

EMBANKMENT: Complete the embankment at the abutments as shown on the Bridge Excavation sheet prior to driving the abutment piling or commencing with the abutment footing excavation.

BACKFILL COMPACTION: Compact backfill at the abutments.

BRIDGE EXCAVATION: Elevation 970.25 shall designate the Excavation Boundary Plane of Class I and Class II Excavation; Class I above the plane, Class II below the plane. See the Bridge Excavation sheet for the limits of pay excavation.

SOUNDINGS: Sounding shown on these plans are taken from notes obtained in the field and represent the best information available to Jackson County.

PILING: Drive all piling to penetrate or bear upon the Willard Shale formation. Driving shall stop when in the opinion of the Engineer additional driving may damage the piling. Drive all piling to the Pile Driving Formula Load of:

Abutment No. 1	62.6 tons
Abutment No. 2	62.6 tons
Pier No. 1	95.4 tons
Pier No. 2	95.4 tons

As a minimum drive each pile to the load and penetration, but in no case shall the pile be driven to more than 110% of Pile Driving Formula Driving Load. At any location where problems are experienced, pile damage is suspected, or the Pile Driving Formula Load occurs significantly above the design pile tip elevation, the Pile Driving Analyzer (PDA) equipment shall be used at both piers as indicated on the Construction Layout, Sheet No. 11.

PILING SPLICE LOCATION: Integral pile splice locations and weld testing criteria for Abutments and Piers will follow the "Standard Pile Details" Sheet (BR110).

TEST PILE SPECIAL: Drive the test pile special at the locations directed by the Engineer/Geologist or as shown on Plans. Use Pile Driving Analyzer (PDA) equipment and methods compliant with KDOT Specifications. The test piling shall remain in place as permanent piling. Drive the test pile special piling to the resistance value of the Strength 95.4 ton/pile load divided by Phi shown on the plans.

CONTRACTOR FURNISHED PDA: Use the Pile Driving Analyzer equipment at the locations shown on the Construction Layout. Use Pile Driving Analyzer equipment and methods compliant with KDOT Special Provision. The piling shall remain in place as permanent piling. Drive the piling to the resistance value of (Strength I divided by Phi).

At any location where problems are experienced, pile damage is suspected, or the Pile Driving Formula Load occurs significantly above the design pile tip elevation, the Owner's designated Engineer may request that the Pile Driving Analyzer (PDA) equipment may be used.

DEMOLITION PLANS: This is a Category A Demolition. Submit detailed Demolition Plans to the Owner's designated Engineer per KDOT Specifications. No Demolition work will begin without approved Demolition Plans. A Licensed Professional Engineer is not required.

REMOVAL OF EXISTING STRUCTURES: Removal of existing structure is included in the bid item "Removal of Existing Structure", Lump Sum. All material obtained from the removal of the existing structure shall become the property of the Contractor and removed from the site.

CONCRETE: Superstructure concrete is bid as Concrete (Grade 4.0)(AE)(SW). Substructure concrete is bid as Concrete (Grade 4.0)(AE). Bevel all exposed edges of all concrete with a ¾" triangular molding, except as otherwise noted on the plans. Construction joints are optional with the Contractor, but if used, place only at locations shown, or at locations approved by the Engineer.

REINFORCING STEEL: All reinforcing steel dimensions are to the centerline of bars unless otherwise noted. All reinforcing steel shall conform to the requirements of ASTM A615, Grade 60.

CAMBER: Provide camber as shown on the Camber Diagram unless the Contractor uses either long span steel beam falsework (concrete dead load deflection greater than ¼") or timber falsework with greater than 12'-0" clear span. If either case exists, submit falsework plans that show the additional required camber.

CONSTRUCTION JOINTS: Construction joints shown are optional with the Contractor. If used, place the construction joints at locations shown or at locations approved by the Engineer.

CONSTRUCTION LOADS: Limited traffic is permitted on the new sub-deck, one-course deck or any concrete overlay during the curing period, keep any exposed deck wet during the curing period. See KDOT Specifications Section 710 Table 710-2.

FALSEWORK PLANS: A licensed Professional Engineer shall design the falsework details. Details shall bear the seal of a licensed Professional Engineer. See the Bridge Design Manual, Section 5.1 "Review and Approval of Falsework Plans", for a listing of items to be included on the falsework plan. Submit three sets of details in compliance with KDOT Specifications to the Owner's designated Engineer for review and distribution.

FALSEWORK INSPECTION: This project has falsework plan requirements which are considered "Category 2" by KDOT specifications. If falsework deficiencies or variations from the approved and sealed plans are found, the falsework design Engineer of Record will provide written approval of the changes. If for the convenience of the Contractor the falsework becomes "Category 1" by the use of non-typical supports; then the inspection and review requirement of "Category 1" will be fully enforced, but at no cost to the Owner. "Category 2" falsework inspection is not paid for directly, but is subsidiary to other bid items.

FALSEWORK: Leave the falsework in place for the entire unit until 15 days after the last concrete pour for the unit or longer as directed by the Engineer.

FALSEWORK PLANS AND SHOP DRAWINGS: Use U.S. Customary system of units on falsework plans and shop drawings.

CORRAL RAIL: Build the corral rail after the falsework is struck.

HEADER BOARD: Immediately after the vertical forms on the EWS are removed, protect the exposed EWS by bolting a wooden header (Minimum dimension of 2½" by 7½") to the exposed vertical surface of the EWS. Extend the header board the full width of the EWS or use 1 section of header board for each lane of traffic. Shape the header board to comply with the crown surface of the bridge surface, and install it flush with the concrete wearing surface. This item shall be paid for subsidiary to the bid item "Concrete (Grade 4.0) (AE) (SW)".

CONCRETE PLACING SEQUENCE: The sequence of placing concrete in the slab and curbs shall be as shown, or the Contractor may submit an alternate placing sequence for review. Submit the alternate placing sequence to the Engineer at the Preconstruction Conference. Include the proposed rate of concrete placement in C.Y./h, the plant capacity, placement direction, construction joint location, a description of the equipment used in placing the concrete, proposed admixtures and the quantity of concrete in each placing segment. Any additional cost for the Contractor's alternate plan of placing the concrete, including admixtures, shall be at the Contractor's expense and shall be considered subsidiary to the bid item, "Concrete (Grade 4.0) (AE) (SW)". Approval of the Contractor's alternate sequence is required prior to placement of concrete in the deck.

SLOPE PROTECTION (Shot Rock): Place Slope Protection (Shot Rock) (18") to the limits and thicknesses shown on the plans or as directed by the Engineer.

DRIP LINE PROTECTION: Place a 10 ft. wide mat of geotextile under the rock/rubble embankment on the berm and berm slopes and centered on the drip lines of the slab.

DIMENSIONS: All dimensions shown on the design plans are horizontal dimensions unless otherwise noted. Make necessary allowances for the roadway grade and cross slopes.

CONTRACTOR CONSTRUCTION STAKING: Contractor Construction Staking for clear span bridges requires two independent surveys. See KDOT Specifications.

ASBESTOS INFORMATION: (Not yet determined)

INDEX TO BRIDGE DRAWINGS	
Sheet No.	Drawing
9	General Notes and Quantities
10	Contour Map
11	Construction Layout
12	Abutment Details
13	Pier Details
14-16	Superstructure Details
17	Bill of Reinforcing Steel and Bending Diagrams
Standards	
18	Bridge Excavation
19	Standard Pile Details
20	Supports and Spacers for Reinforcing Steel

DESIGN DATA

DESIGN SPECIFICATIONS: AASHTO Specifications, 8th Edition, 2017 with latest Interim Specifications. Load and Resistance Factor Design.

DESIGN LOADING: HL-93

Design Dead Load includes an allowance of 25 psf for a future wearing surface.

Concrete (Grade 4.0)	f'c	=	4.0 ksi
Concrete (Grade 4.0)(AE)	f'c	=	4.0 ksi
Concrete (Grade 4.0)(AE)(SW)	f'c	=	4.0 ksi
Reinforcing Steel (Grade 60)	fy	=	60 ksi

LRFD DESIGN PILE LOAD:	Strength I	Service I	Phi
Design Loading (Tons/Pile)			
Abutment	62.6	40.2	0.45
Pier	95.4	59.3	0.65

LFD & LRFR RATING FACTORS		
Rating Level		
Truck	Inventory	Operating
HS-20 (36T)	1.27	1.63
HET (110T)		1.36
2002 LFD Rating, 17th Edition AASHTO		
HL-93 Loading	1.04	1.33
NRL	1.23	1.59
2016 Manual for Bridge Evaluation		

6	10/19/15	Added Asbestos Not8221 Option	JPJ	CER
5	2/4/15	Modified Per 2015 Specification	JPJ	CER
4	4/7/14	Current Release	JPJ	CER
3	1/12/14	Added Benchmark	JPJ	CER
2	08/2/12	ADDED NOT3135 & NOT3145	JPJ	TLF
1	04/29/10	ADDED RATING TABLES	JPJ	KFH
NO.	DATE	REVISIONS	BY	APP'D

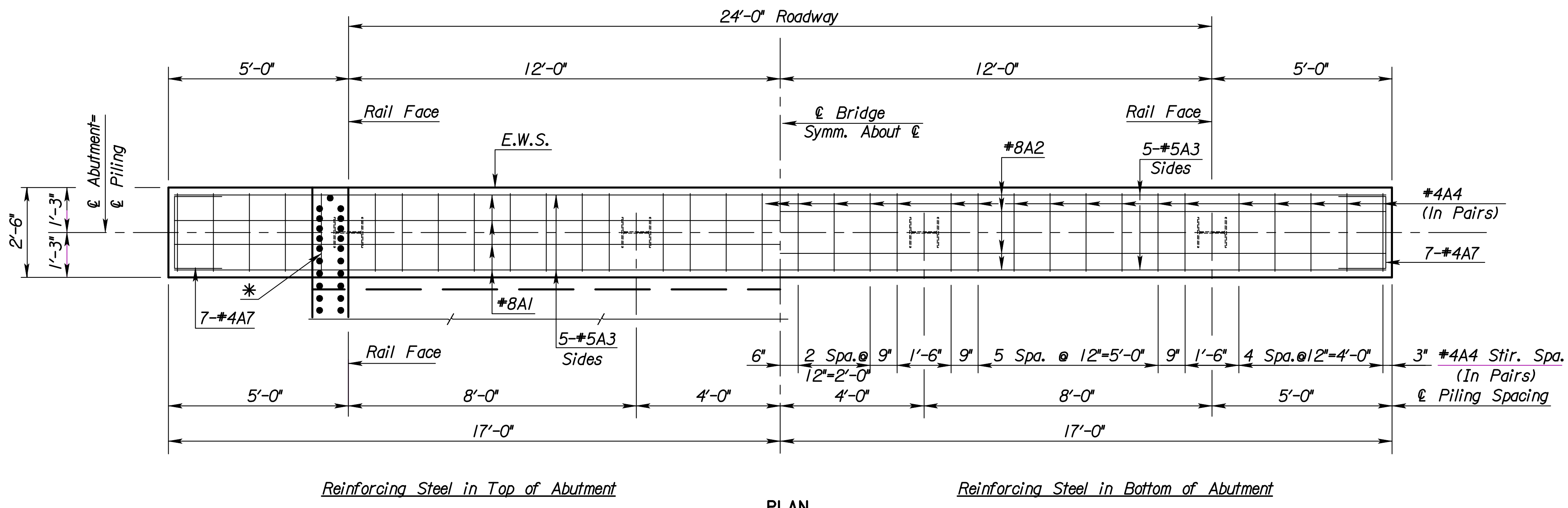
KANSAS DEPARTMENT OF TRANSPORTATION

GENERAL NOTES AND QUANTITIES

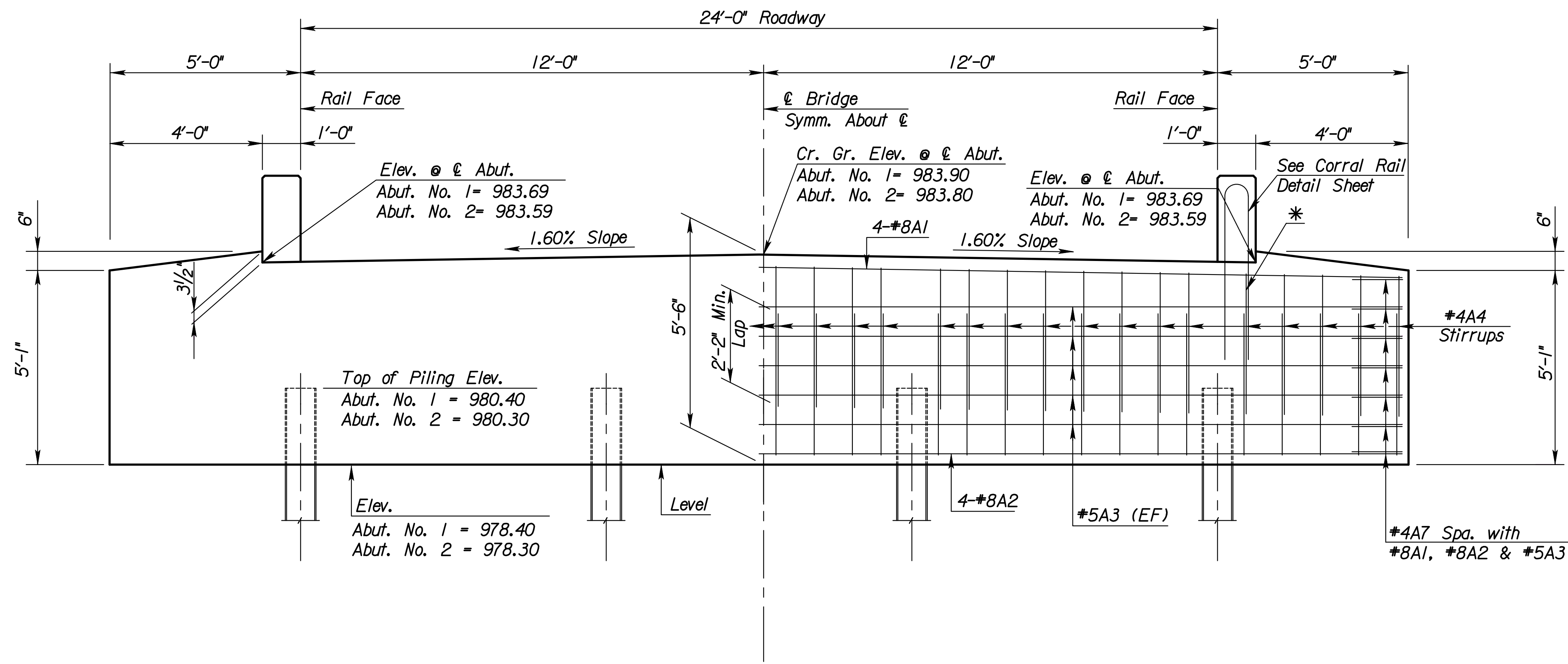
SHEET NO.	OF	SCALE	APP'D
DESIGNED		DETAILED	QUANTITIES
DESIGN CK.		DETAIL CK.	QUAN. CK.


BG CONSULTANTS
 ENGINEERS · ARCHITECTS · SURVEYORS

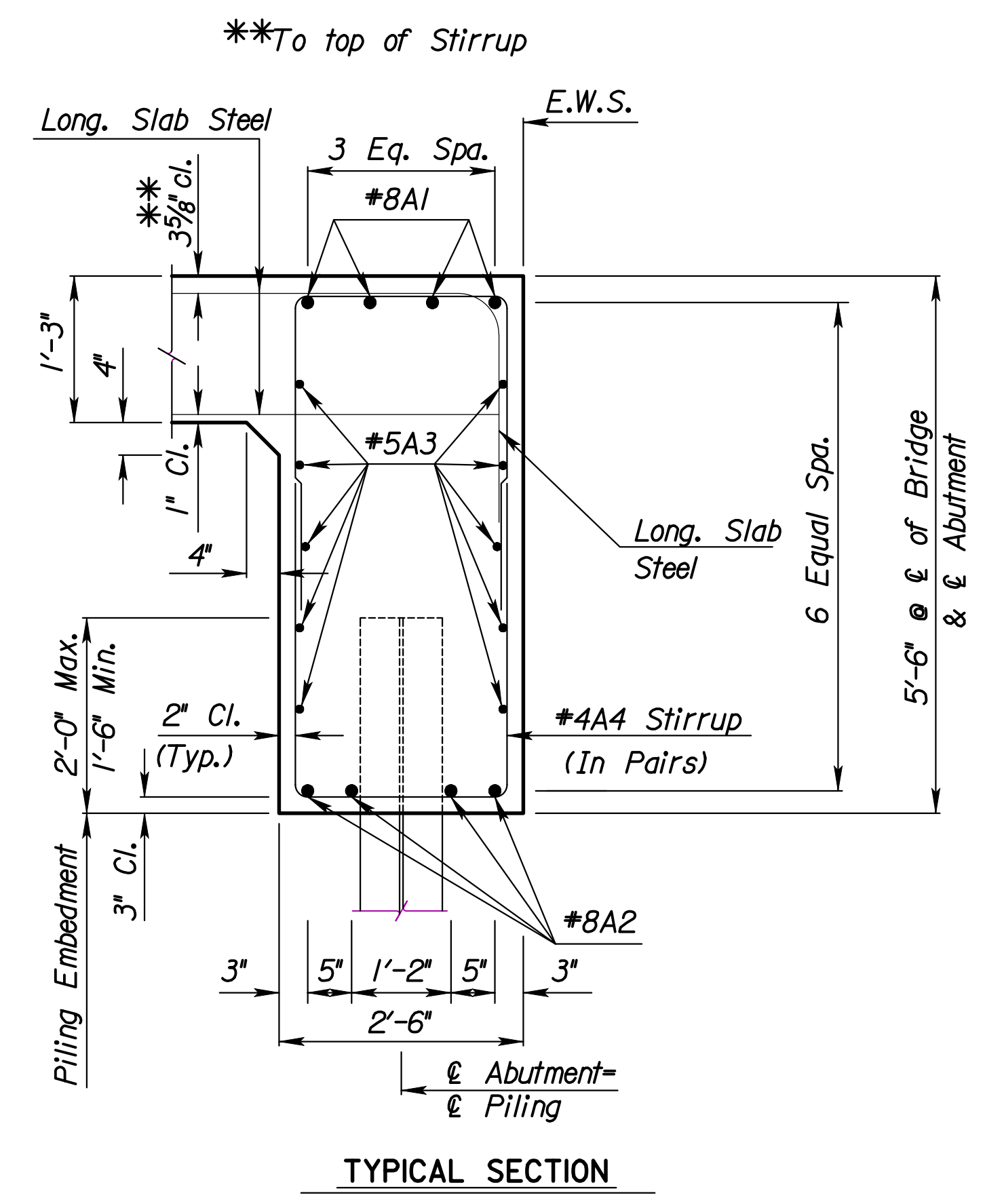
hs1ab-lrfd-500.dgn
LRFD
Roadway Width = 24'-0"
Skew and Direction = 0
Number of Piles = 4



PLAN



ELEVATION
(Along C Abutment)



*Adjust stirrup to avoid conflict with rail bars.

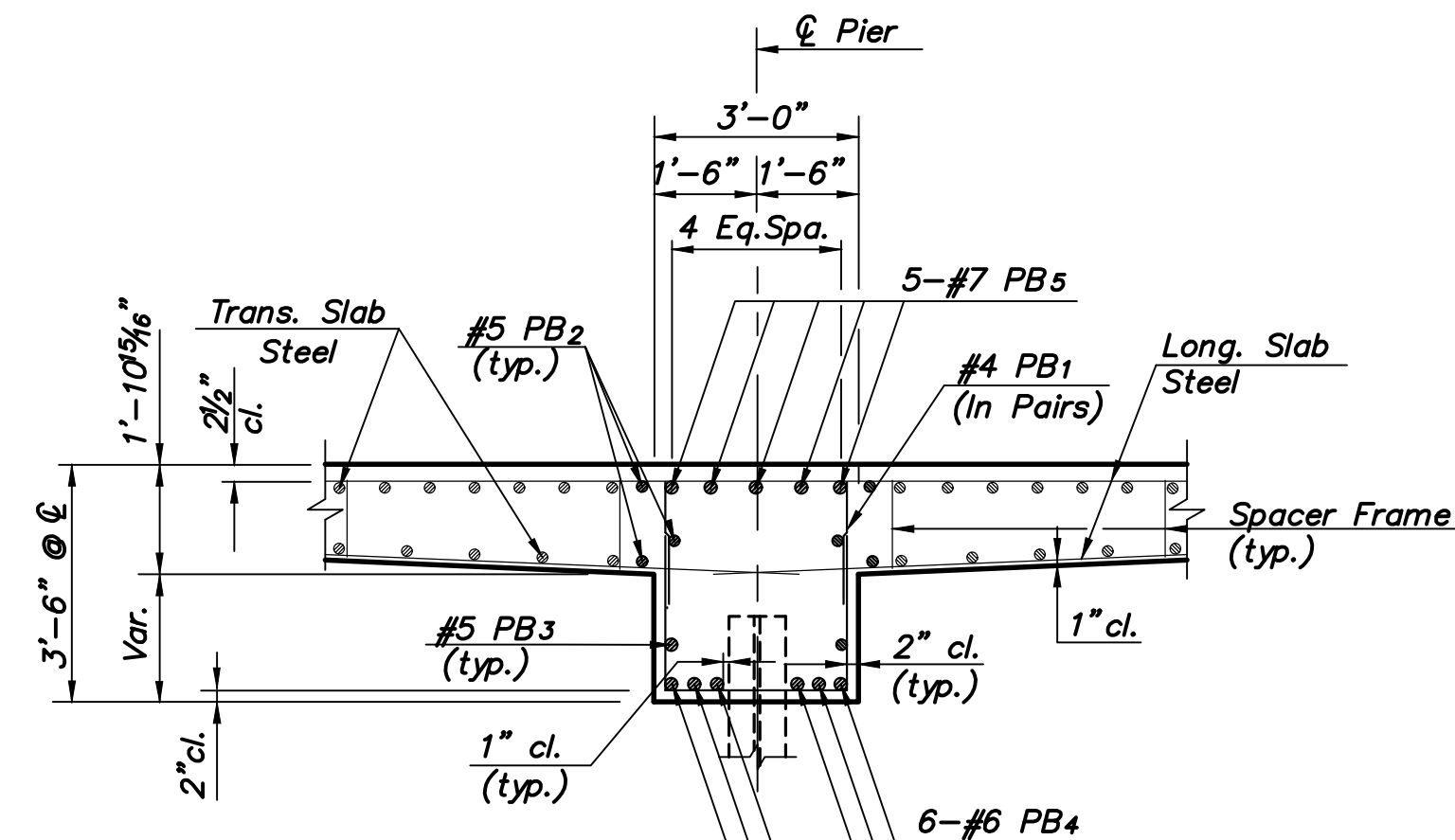
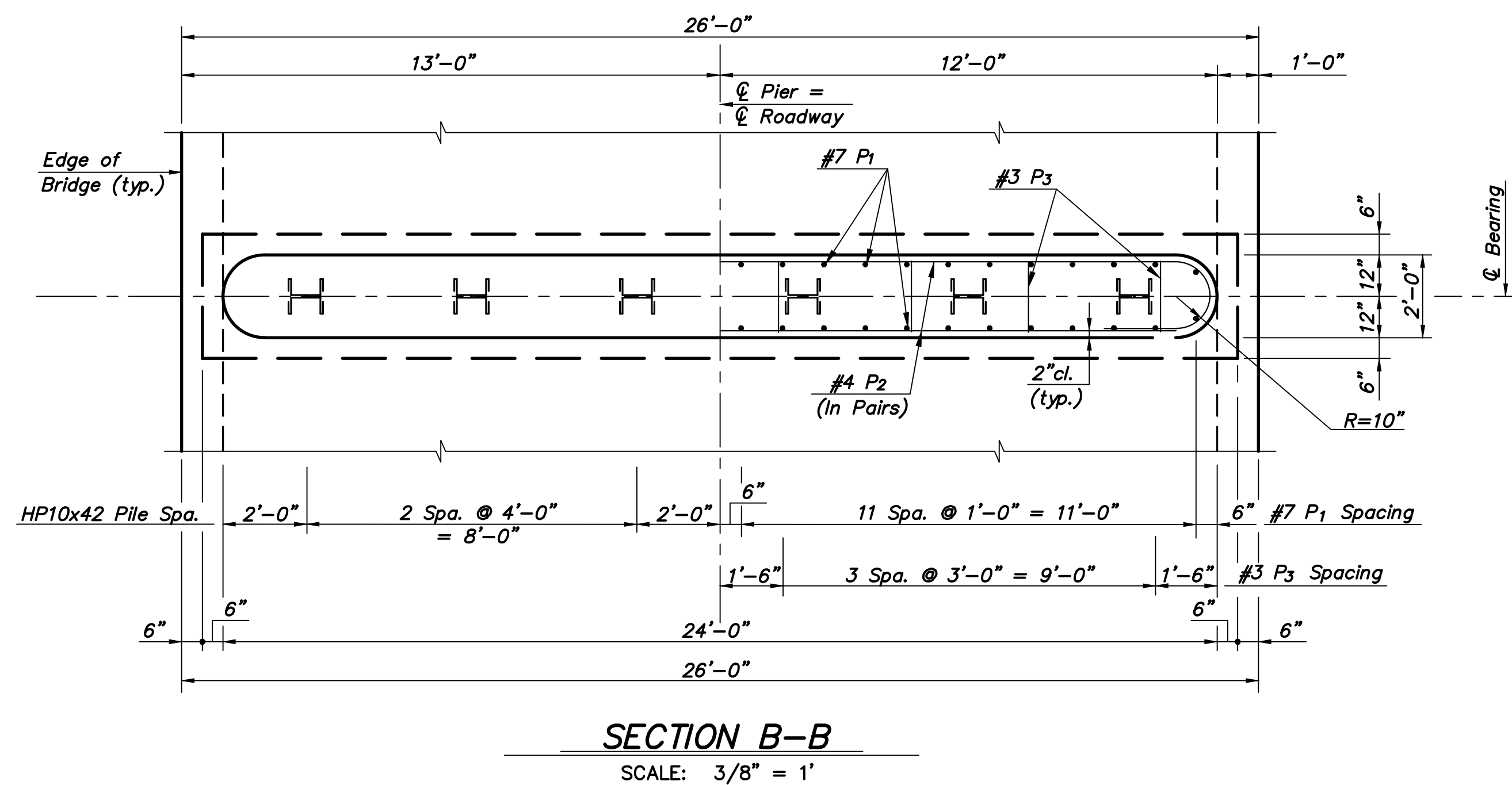
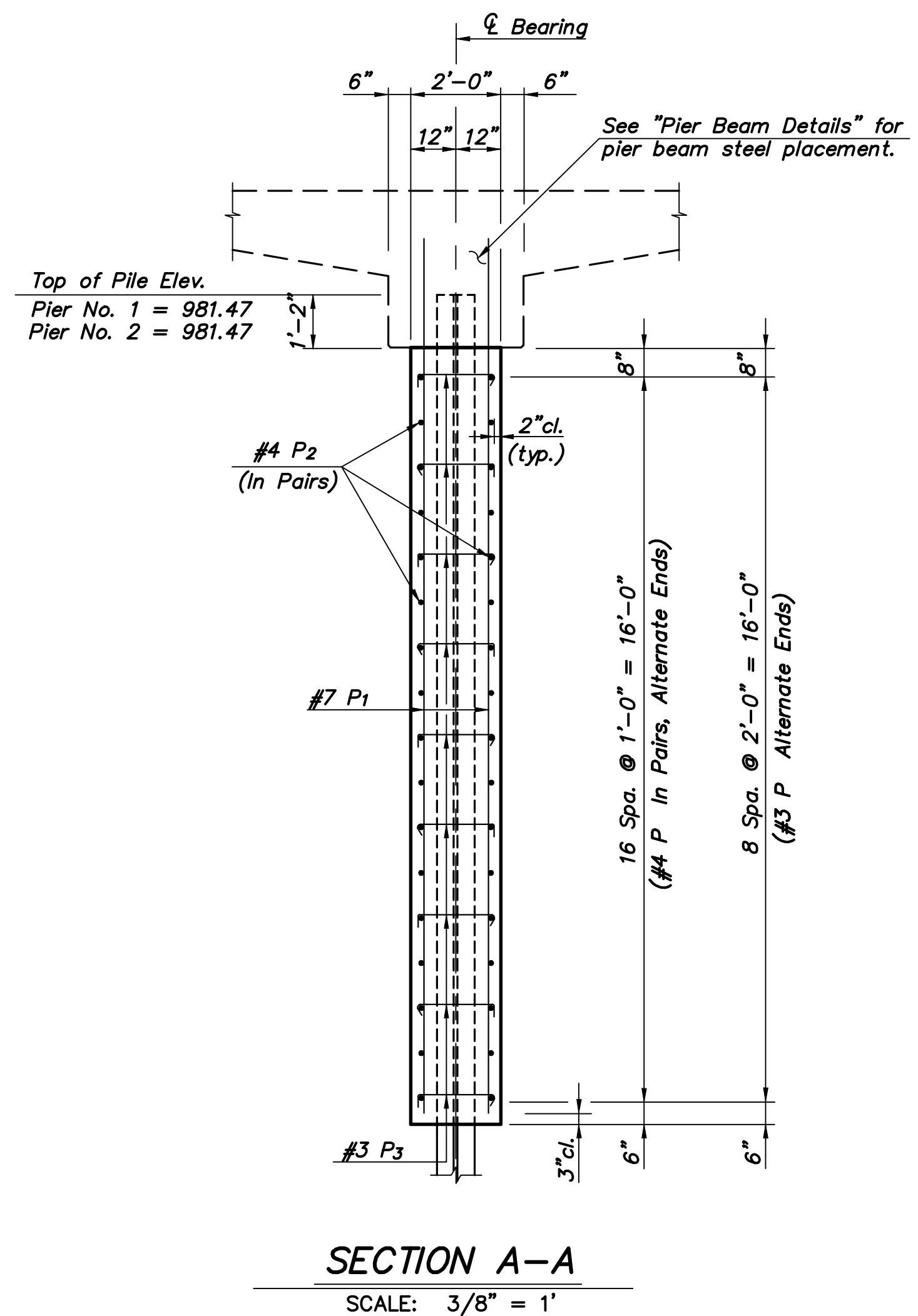
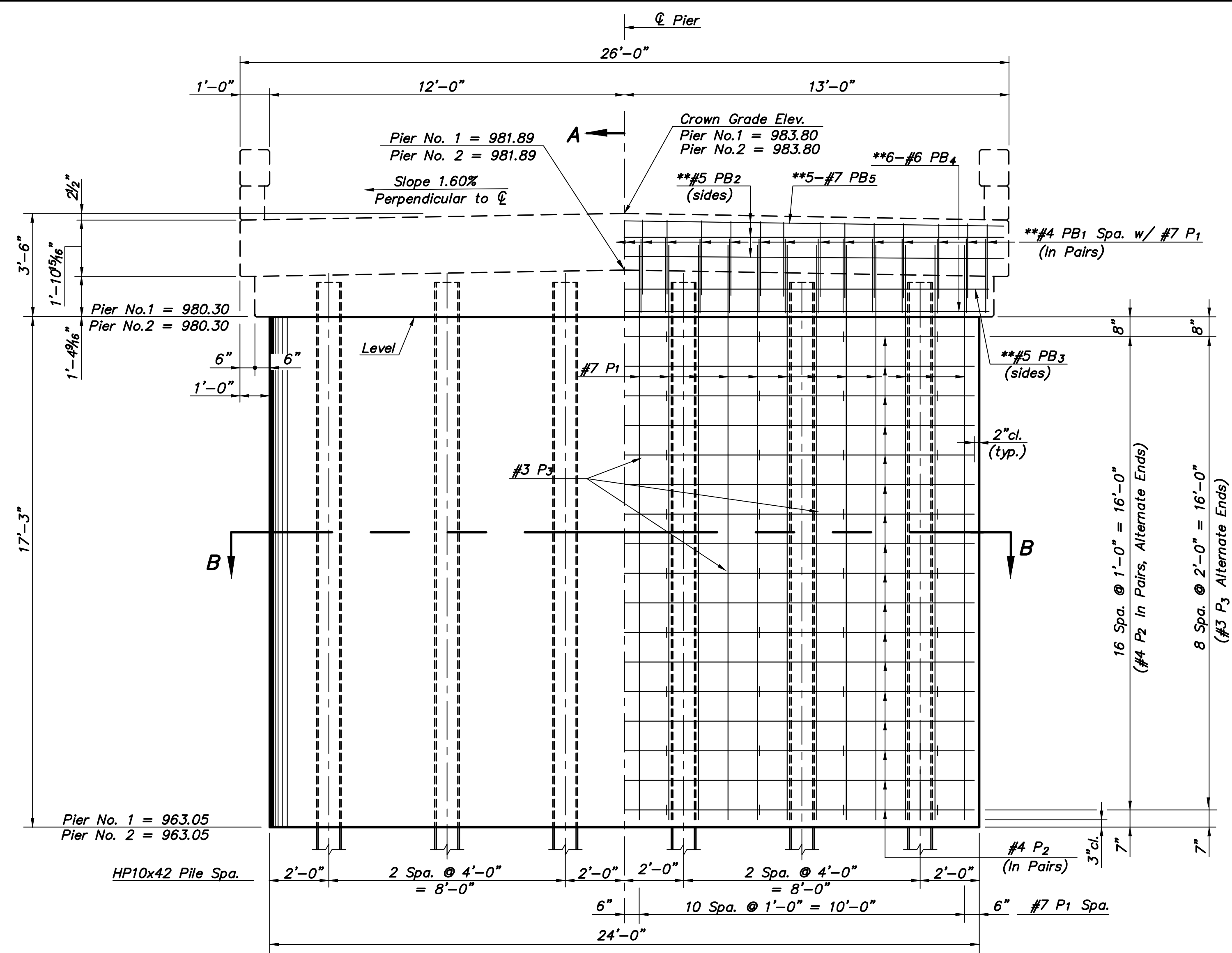
Legend
EF = Each Face

Note: Top of piling elevations are based on 2'-0" maximum embedment.

Max. Factored Design Pile - Load (Strength I) = 62.6 ton/pile
Factored Resistance = 70 tons/pile
Phi = 0.45
Use HPI0x42 Gr. Steel Piles

4	7/29/09	Remove Factored Resistance	DRT	KFH
3	03/24/09	Add Factored Resist.to Pile Loading	DRT	KFH
2	3/6/07	correct Abut. Dim.'A' for 54-72-54	DRT	KFH
1	4/6/06	Adj. Abut. Vol. & DL	DRT	KFH
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION				
ABUTMENT DETAILS				
SHEET NO.	OF	SCALE	APP'D	
DESIGNED	DRT	DETAILED	DRT	QUANTITIES
DESIGN CK.		DETAIL CK.	QUAN. CK.	CADD RCJ



**Note: All PB bars to be placed with deck steel.

Max. Factored Design Pile Load (Strength I) = 95.4 tons/pile
Factored Resistance = 100.7 tons/pile
Phi. = 0.65 (PDA Required)
Use HP10x42 Gr. 50 Steel Piles

JACKSON COUNTY HIGHWAY DEPARTMENT

PIER DETAILS

BG CONSULTANTS
ENGINEERS · ARCHITECTS · SURVEYORS

Irdb516.dgn	Plot 2
Roadway Width = 28'-0"	Longest Span Length = 48'-0"
Stew and Direction = 0'-0"	Total No. of Spans = 3
Loading = HL-93	Rolling Type = Corral

LFD & LRFR RATING FACTOR			
Truck	2 1/2'	Oper.	Inv.
HS-20	1.58	2.64	1.65
HET	1.36	1.37	1.78
LRFR HL-93	1.32	1.72	1.37

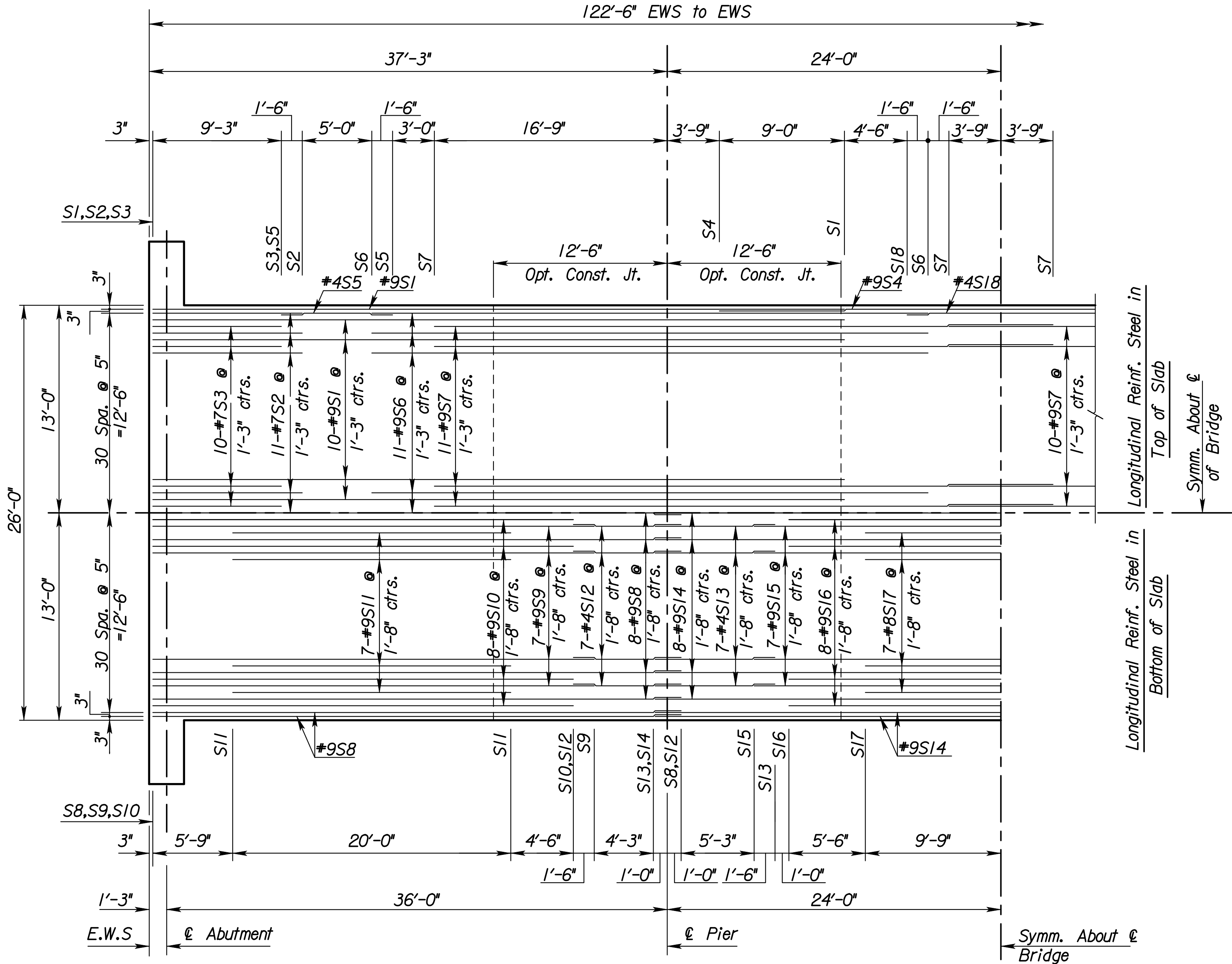
Plotted By: \$USER\$	Plot Location: \$UNIT\$
File: \$\$\$\$\$\$CONSPEC\$\$\$\$\$	
Plot Date: \$\$\$\$SYTIME\$\$\$\$\$	

Note:
See longitudinal section for
transverse reinforcing steel.

Note: 1.0 & 4.0 pts. are taken at  of abutments
2.0 & 3.0 pts. are taken at  of piers

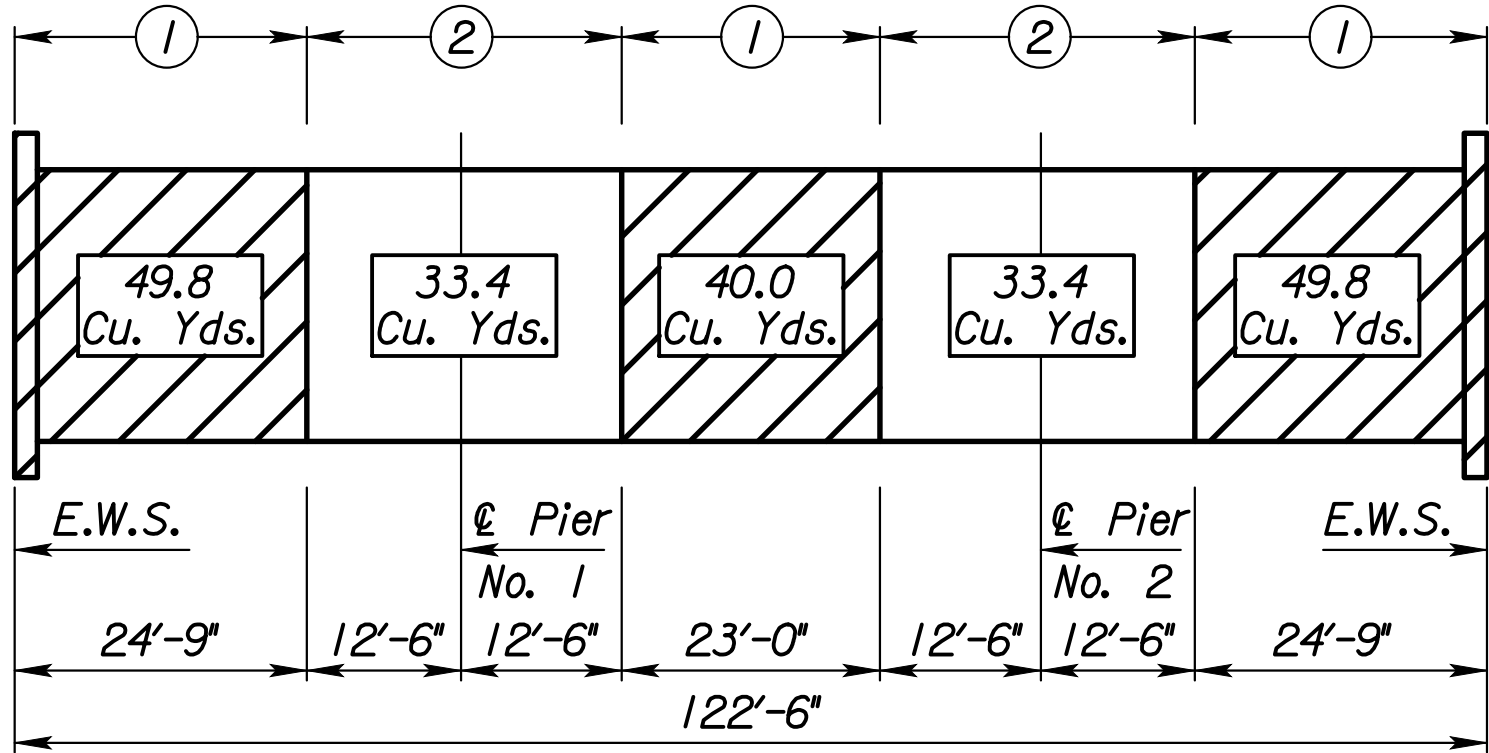
Top of Form Elevation at 10th Points, (ft.)															
1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4	2.5
982.65	982.68	982.68	982.67	982.65	982.60	982.50	982.38	982.22	982.04	981.89	982.11	982.33	982.51	982.61	982.65
2.6	2.7	2.8	2.9	3.0	3.1	3.2	3.3	3.4	3.5	3.6	3.7	3.8	3.9	4.0	
982.61	982.51	982.33	982.11	981.89	982.04	982.22	982.37	982.49	982.58	982.62	982.63	982.62	982.60	982.55	

Note: Elevations are taken at Crown Grade. Note: The change in elevation from Crown Grade to the Edge of Slab is -0.208'



HALF PLAN

20-1458M	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
KANSAS		43 C-5078-01	2022	14	44



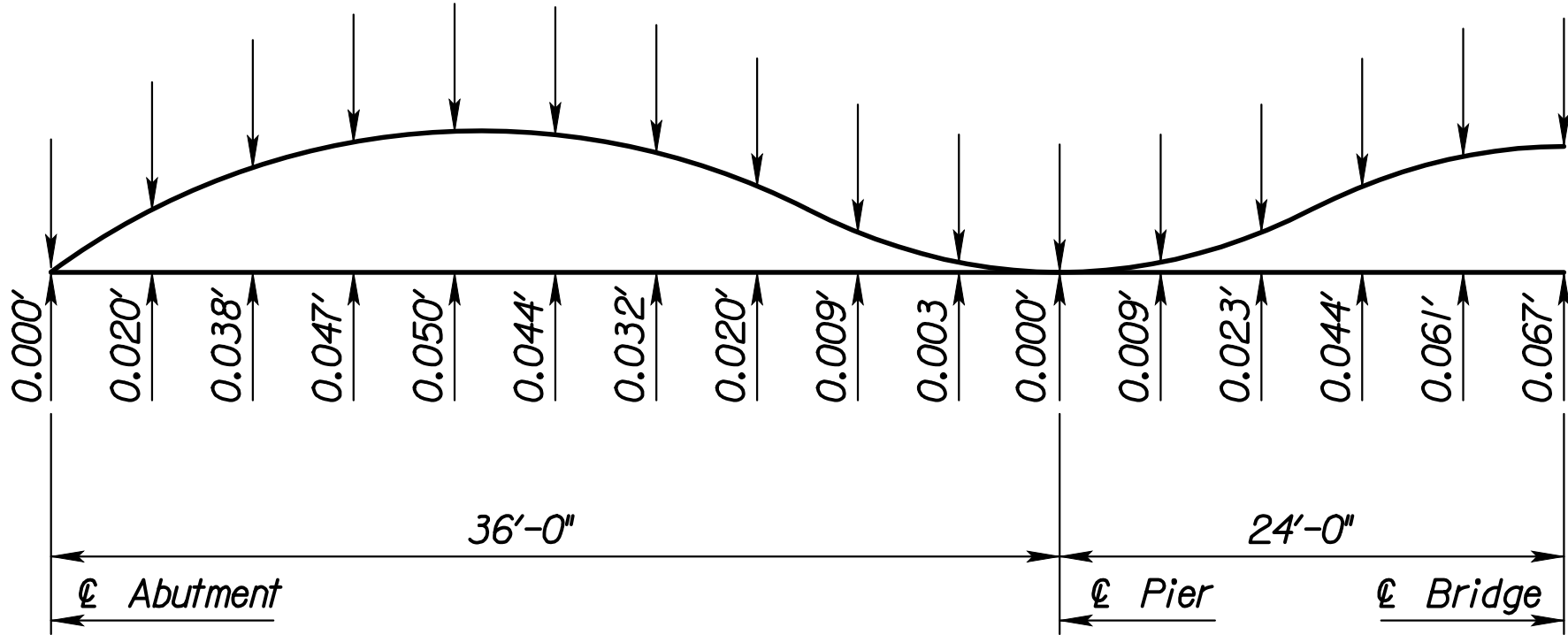
CONCRETE PLACING SEQUENCE DIAGRAM

CONCRETE PLACING SEQUENCE

When long span steel beams having a concrete dead load deflection greater than 1/4" are used or when timber falsework with greater than 12'-0" clear span is used, follow the placing sequence shown. Segmental, combined or continuous pours are allowed, but stop a discontinuous pour at a construction joint short of a pier.

When timber falsework with 12'-0" or less clear span is used, the Contractor, subject to the approval of the Engineer, may use a continuous pour or may discontinue the pour at any construction joint shown.

The Contractor may place the corral rail continuously from one end of the bridge to the other.



DEAD LOAD CAMBER DIAGRAM AT TENTH POINTS

Long Term Deflections = Initial Deflections x 3.5
(Initial Deflections Based on $E_c = 3.644 \times 10^6$ p.s.i.)
(camber values in feet)

4	03/12/12	ADDED TOF Elevation Table	JPJ	TLE
3	02/08/11	ADDED QUANTITIES	JPJ	TLF
2	02/05/09	update LFD RF & Camber	DRT	KFH
1	02/11/08	Chg'd Neg Mo. Steel		
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION

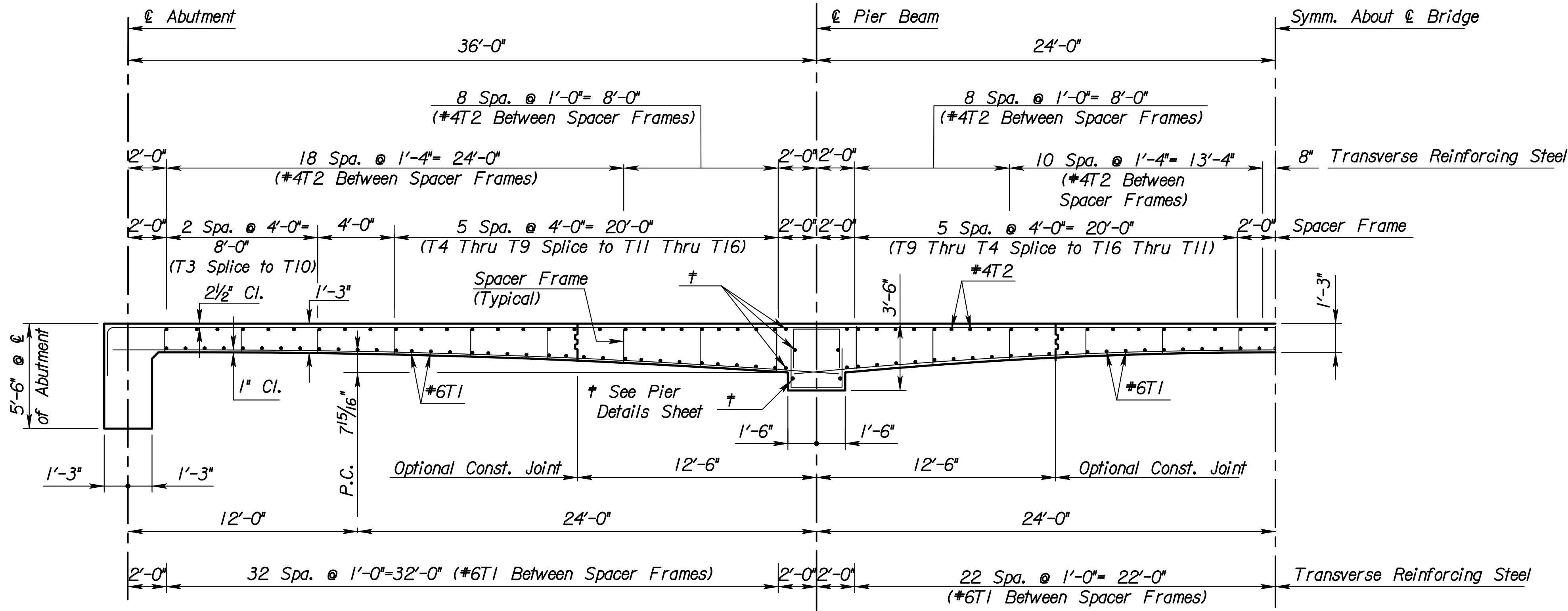
SUPERSTRUCTURE DETAILS

SHEET NO.	OF	SCALE	APP'D
DESIGNED	DRT	DETAILED	DRT
QUANTITIES	BRW	CADD	RCJ
DESIGN CK.	CEM	DETAIL CK.	CEM
QUAN. CK.		CADD CK.	

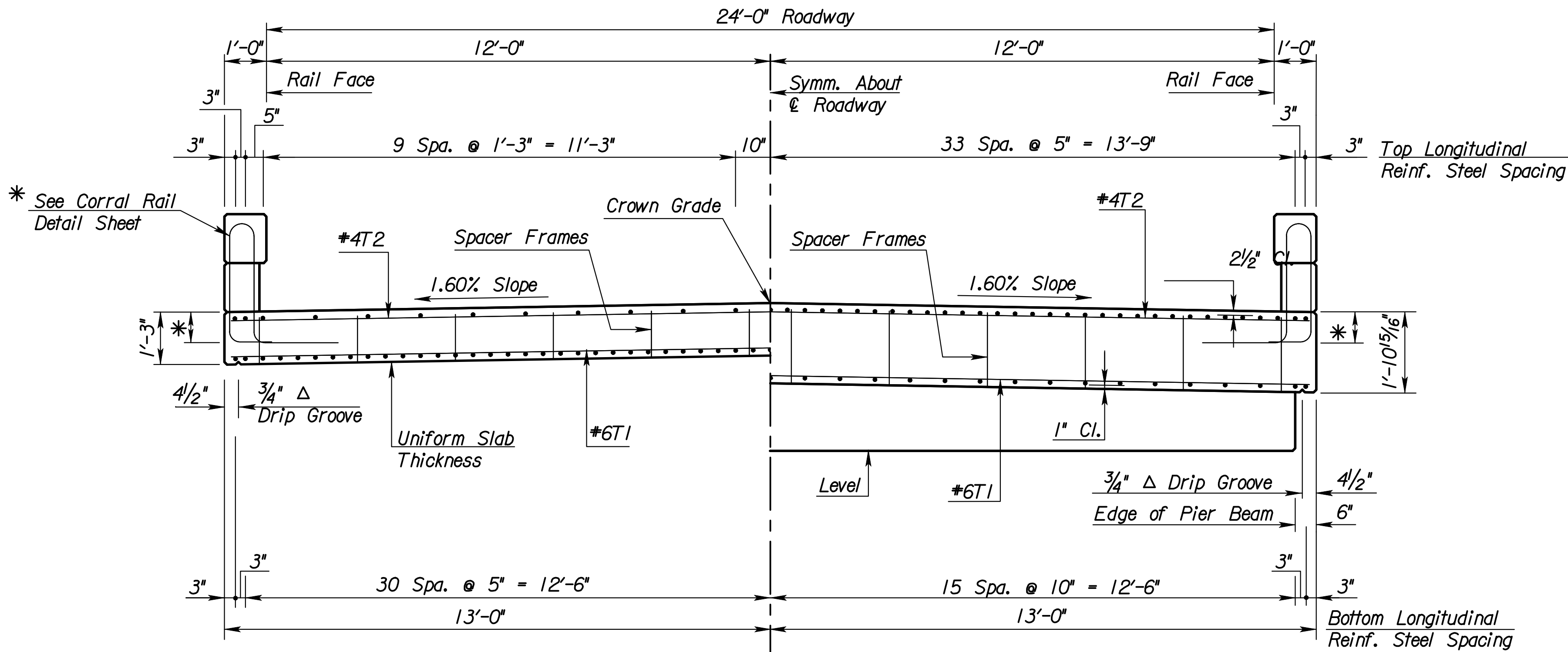
17F0W516.dgn
Plot 3
Longest Span Length = 48'-
Roadway Width = 28'-
Slew and Direction = 0'-
Loading = HL-93
Total No. of Spans = 3
Railway Type = Corral

Plotted By: \$\$\$USERNAME\$\$\$
File: \$\$\$DMSPEC\$\$\$
Plot Date: \$\$\$SYTIME\$\$\$
Plot Location: \$\$\$UNIT\$\$\$

20-1458M	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
	KANSAS	43 C-5078-01	2022	15	44



HALF LONGITUDINAL SECTION ALONG ϵ BRIDGE

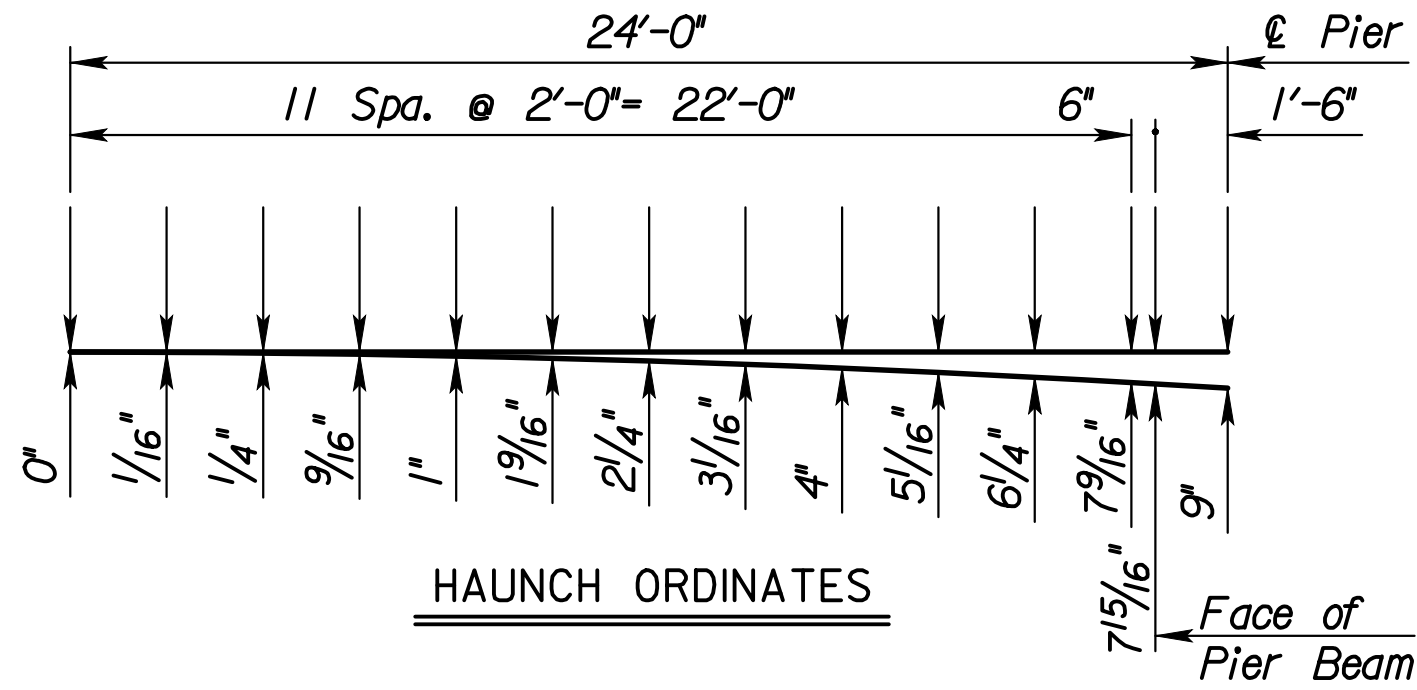


Half Section Near Mid-Span

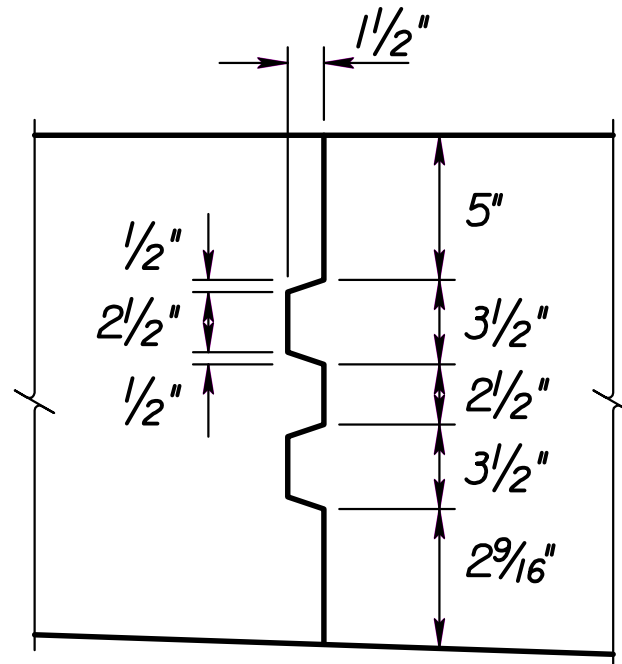
TYPICAL SECTION OF SLAB

Half Section at Face of Pier Beam

* See Corral Rail Detail Sheet.



HAUNCH ORDINATES



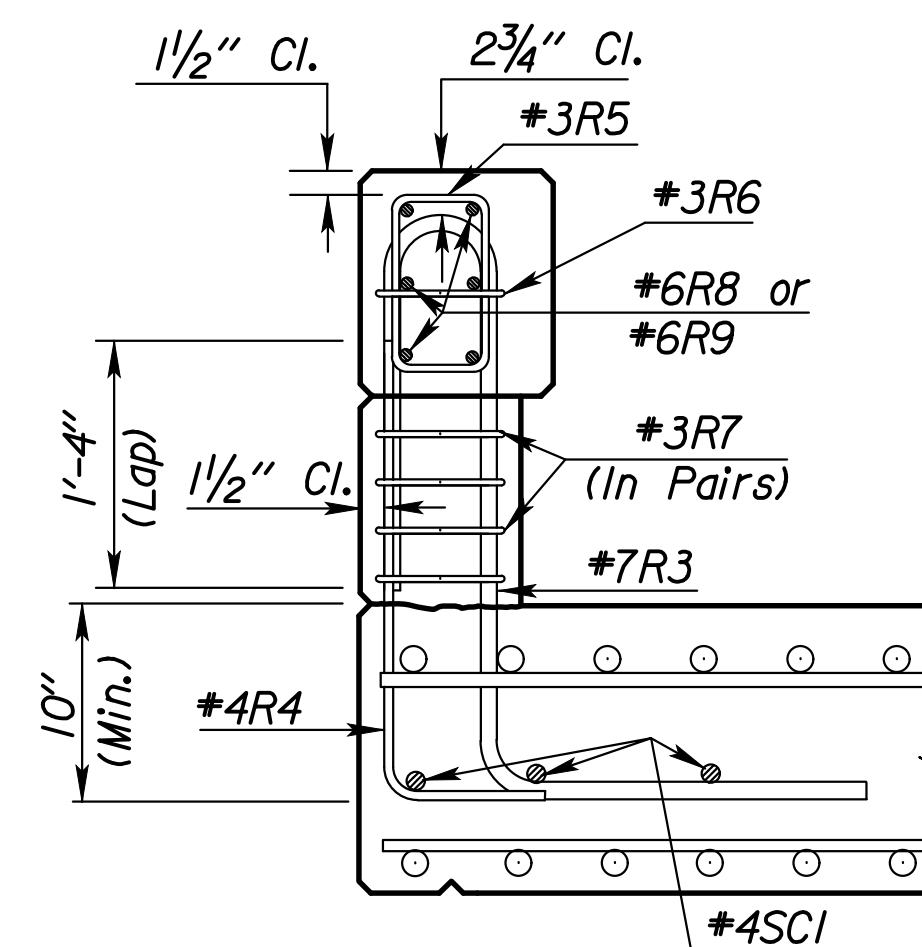
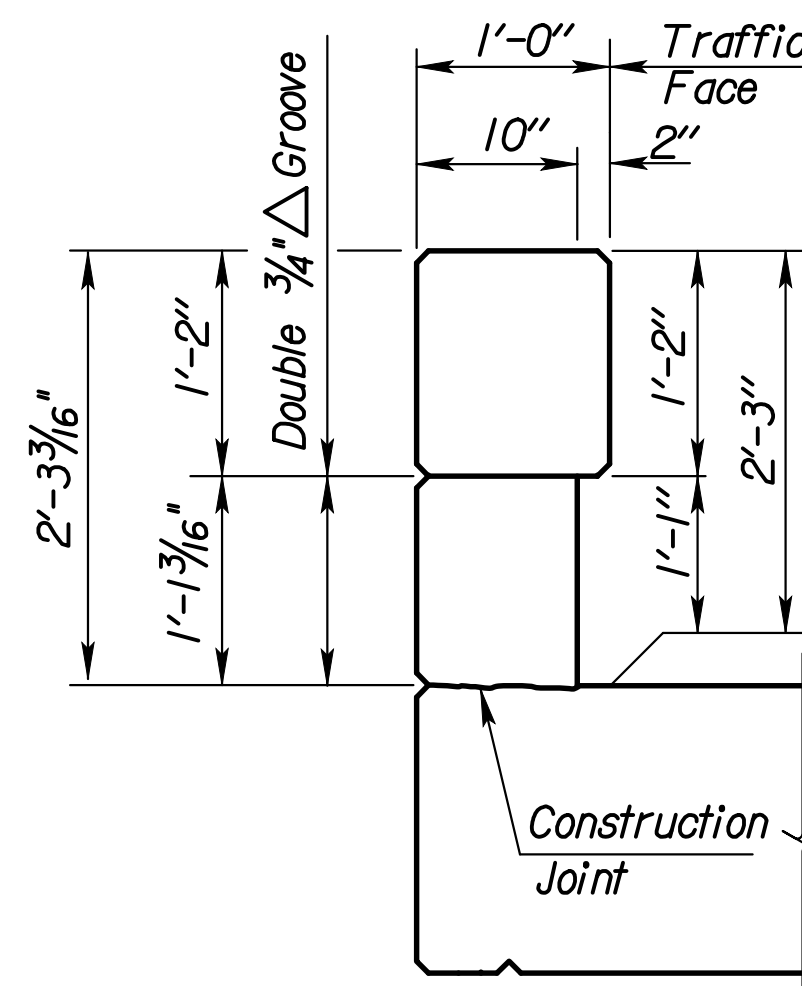
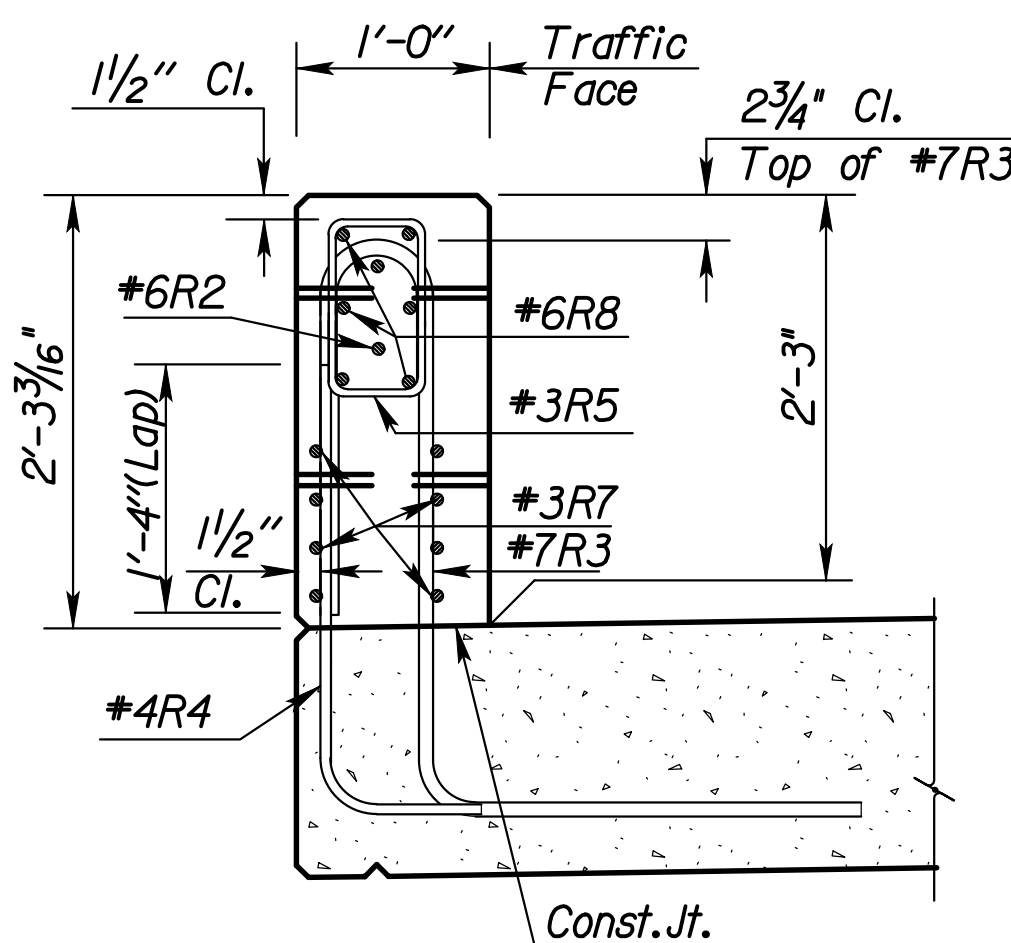
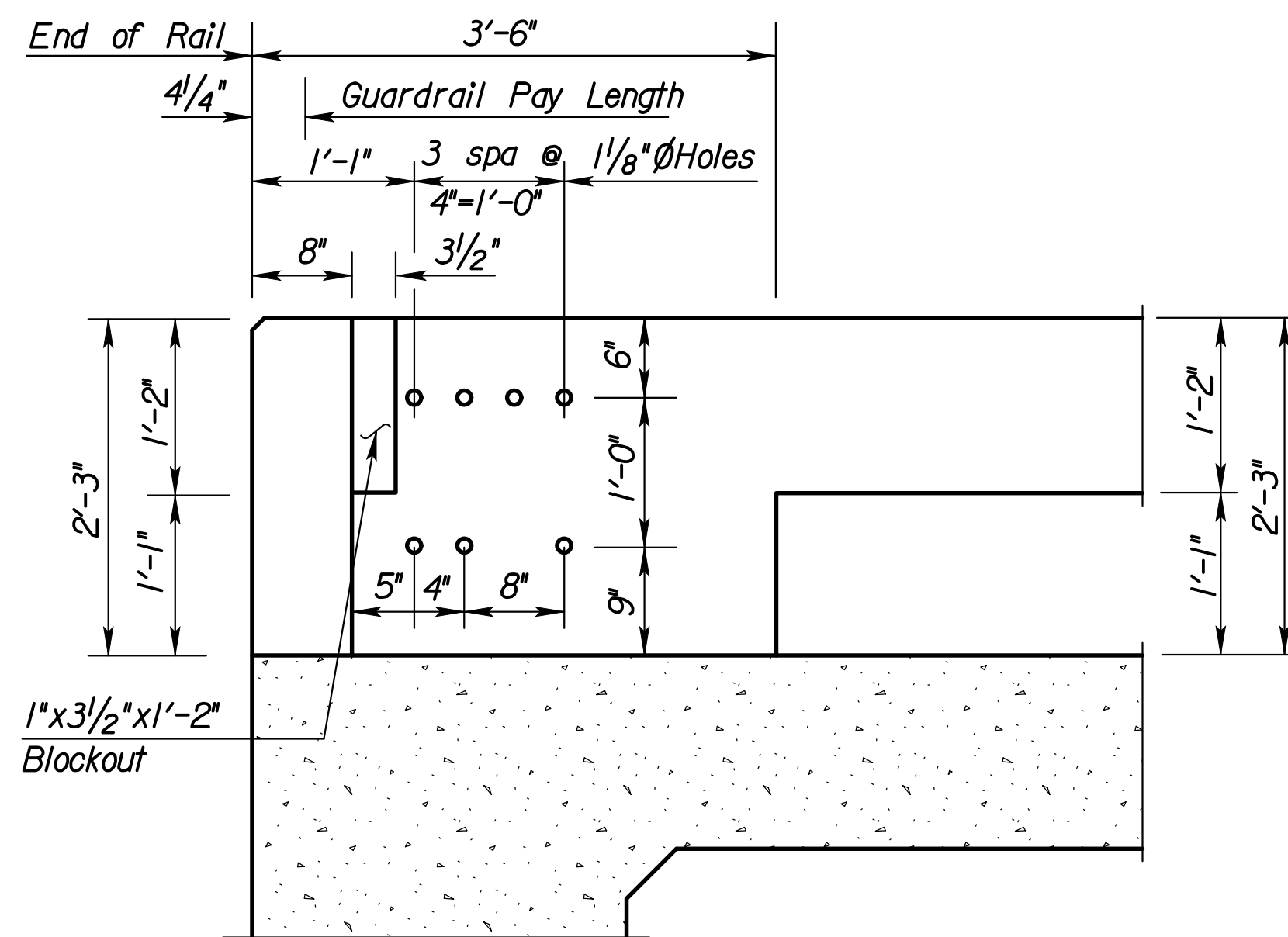
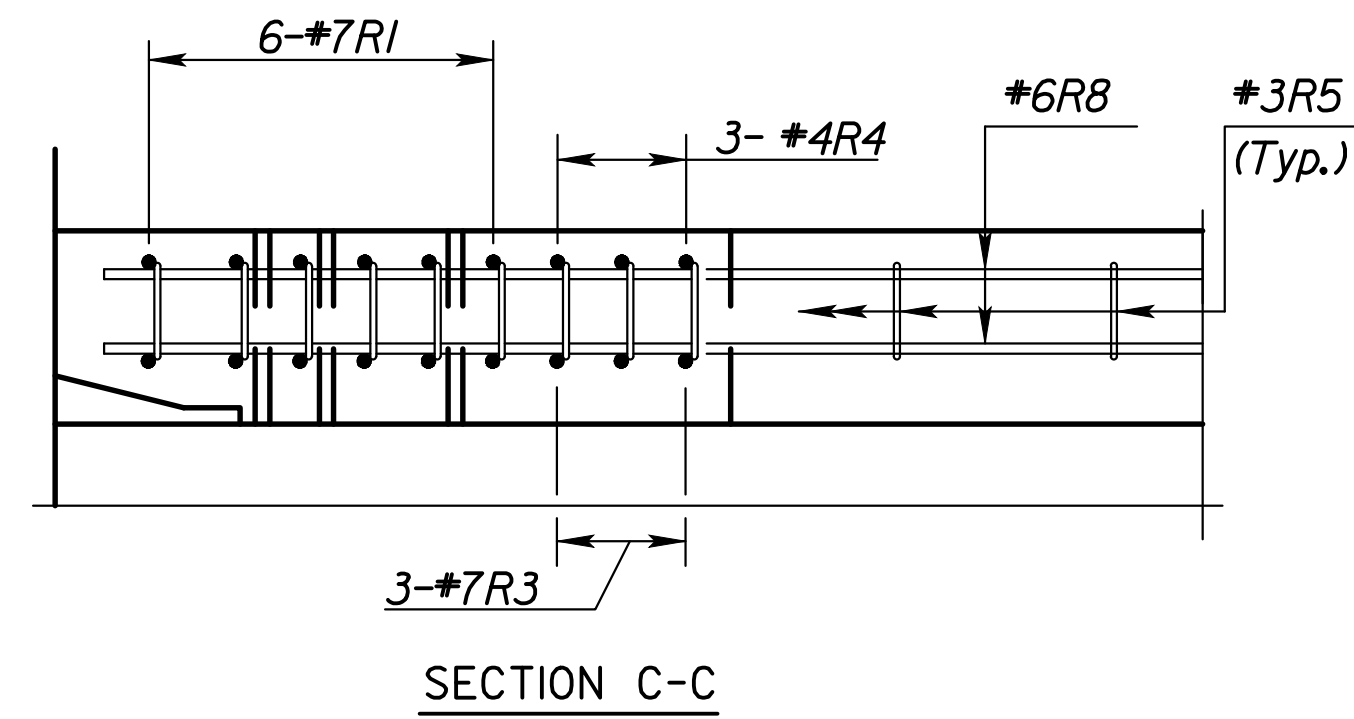
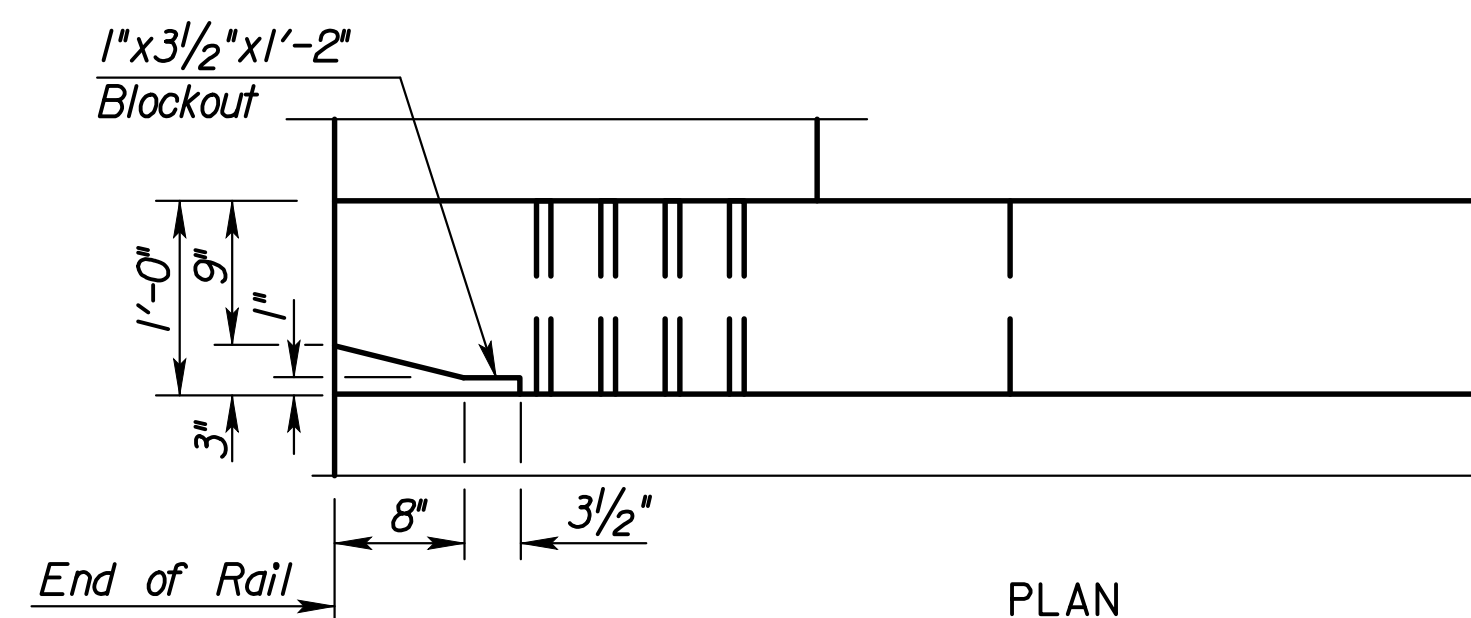
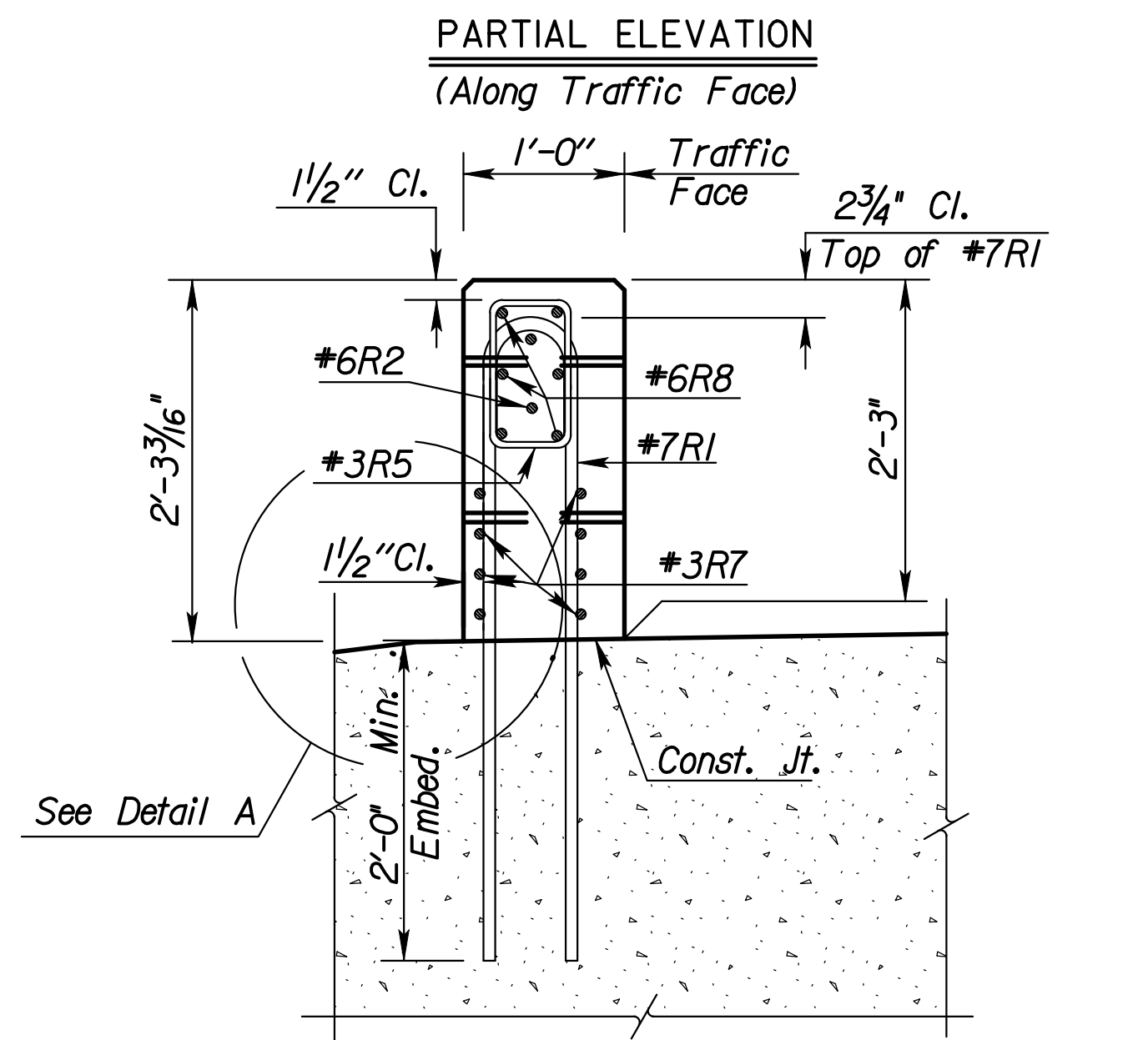
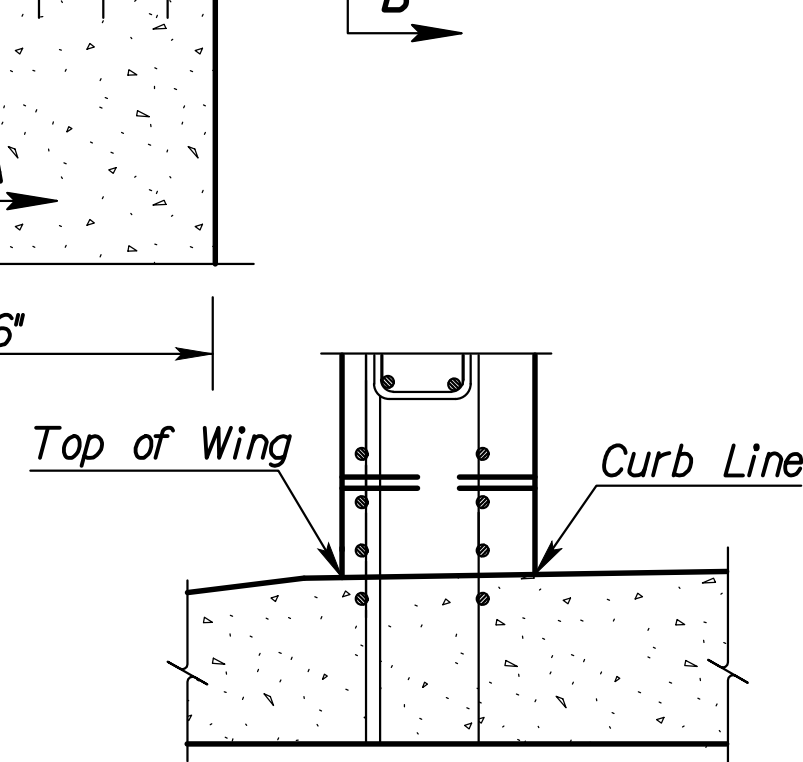
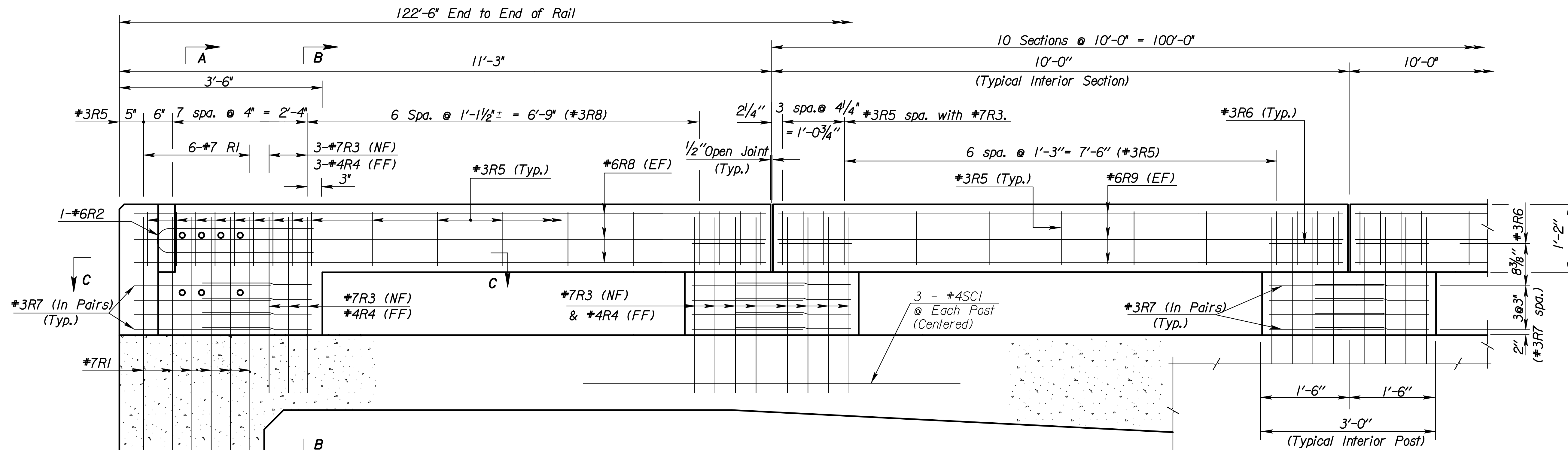
TRANSVERSE SLAB
CONSTRUCTION JOINT
(Optional)

4					
3					
2					
1	02/11/08	Chg'd Neg. Mo. Steel			
NO.	DATE	REVISIONS	BY	APP'D	

KANSAS DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE DETAILS

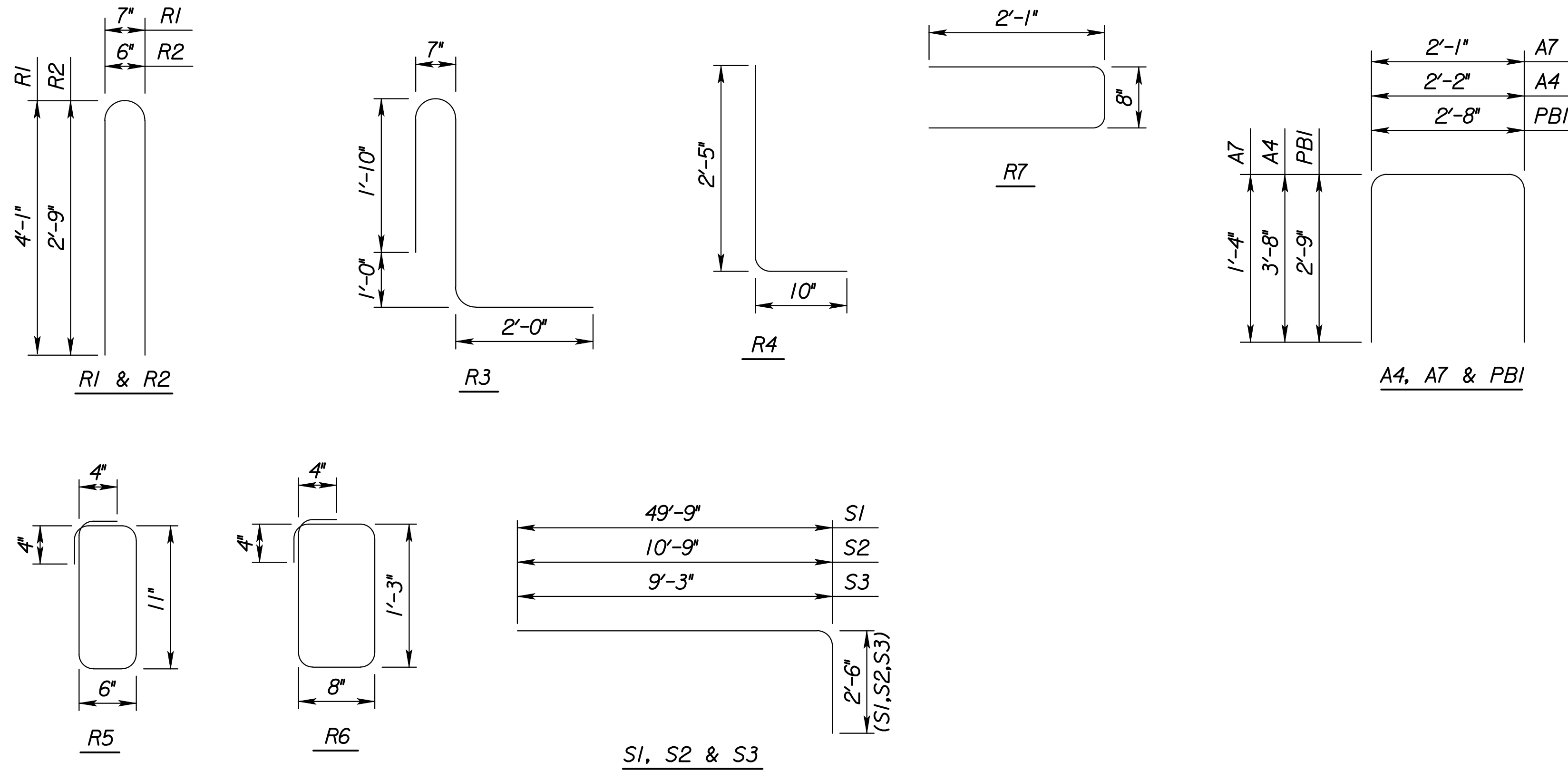
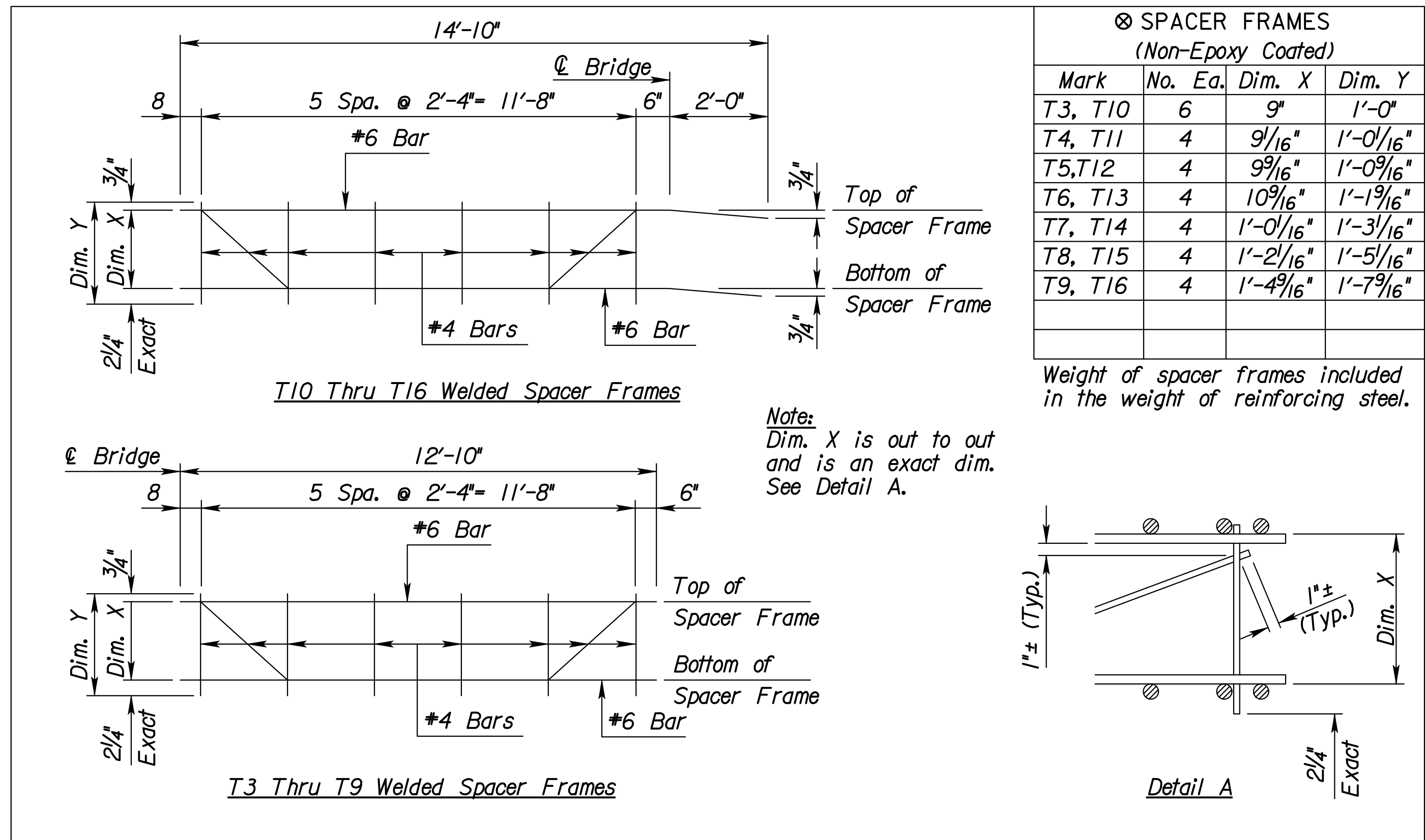
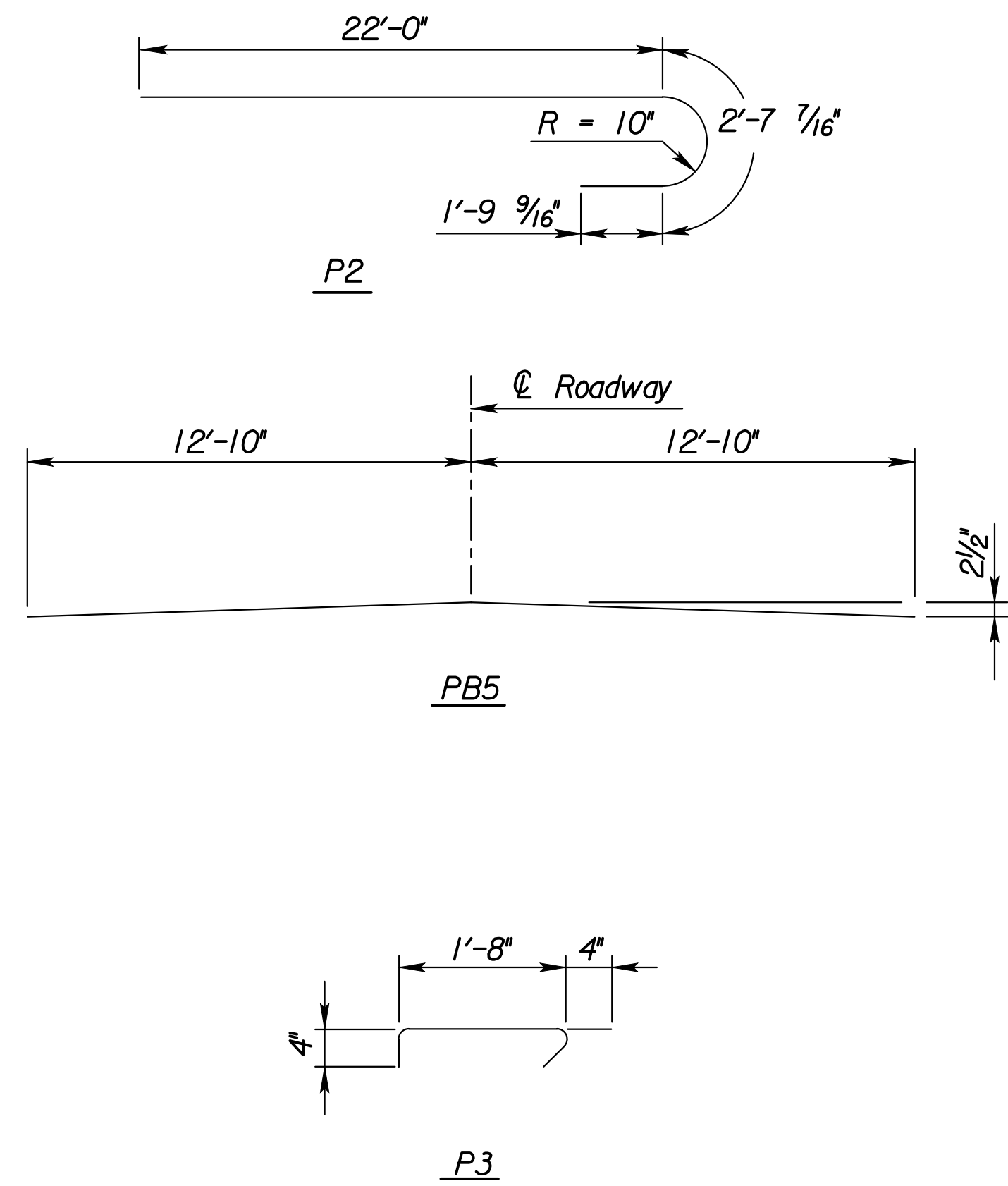
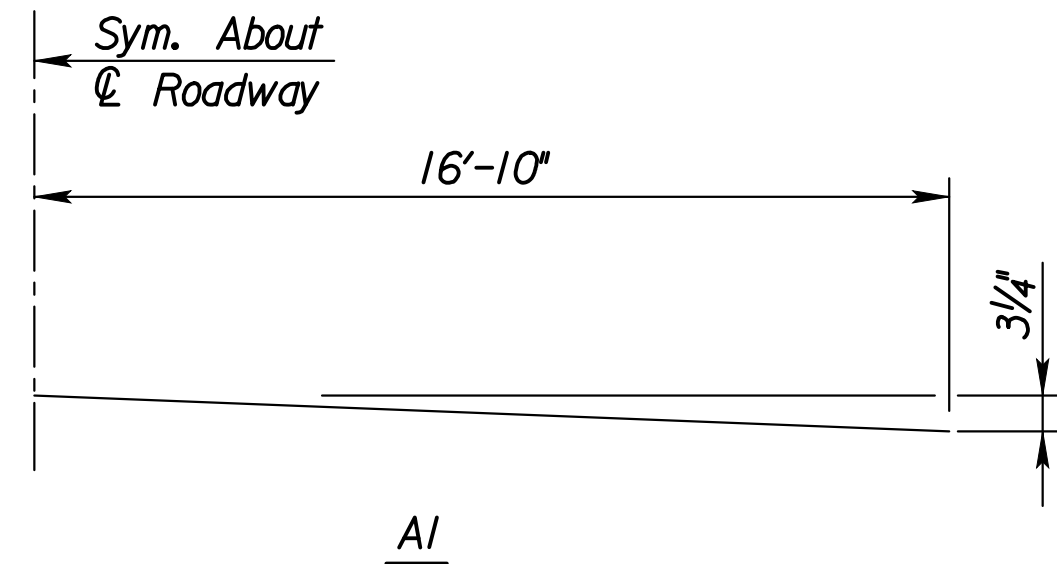
SHEET NO.	OF	SCALE	APP'D		
DESIGNED	DRT	DETAILED	DRT	QUANTITIES	CADD
DESIGN CK.	CEM	DETAIL CK.	CEM	QUAN. CK.	CADD CK.



LEGEND
NF = Near Face
FF = Far Face
EF = Each Face

3					
2					
1	6-30-05	Current Release			
NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
27' KANSAS CORRAL RAIL (W-BEAM WITH RUBRAIL) R.C. HAUNCHED SLAB (Without Curb)					
SHEET NO.	OF	SCALE	APP'D		
DESIGNED		DETAILED	QUANTITIES	CADD	
DESIGN CK.		DETAIL CK.	QUAN. CK.	CADD CK.	

Plotted By: \$USER\$NAME\$	Plot Location: \$UNIT\$
File: \$\$\$\$SPEC\$\$\$\$	
Plot Date: \$\$\$\$CUTIME\$\$\$\$	

[illegible]

BENDING DIAGRAMS
(All dimensions are out to out of bars.)

20-1458M	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
	KANSAS	43 C-5078-01	2022	17	44

		BILL OF REINFORCING STEEL Non-Epoxy Coated - Grade 60							
		Straight Bars				Bent Bars			
		Mark	Size	Number	Length	Mark	Size	Number	Length
Superstructure	Abutment - Deck - Rail	S4	#9	2	40'-6"	S1	#9	44	52'-3"
		S6	#9	42	40'-0"				
		S7	#9	44	44'-6"	A1	#8	8	33'-8"
		S8	#9	38	38'-0"				
		S9	#9	28	31'-9"				
		S10	#9	32	30'-3"	R1	#7	24	8'-6"
		S11	#9	28	20'-0"	R3	#7	188	7'-0"
		S14	#9	19	50'-0"	S2	#7	42	13'-3"
		S15	#9	14	35'-6"	S3	#7	40	11'-9"
		S16	#9	16	30'-6"				
						R2	#6	4	5'-10"
		S17	#8	14	19'-6"				
		A2	#8	8	33'-8"	A4	#4	136	9'-6"
						A7	#4	28	4'-9"
		R8	#6	24	10'-11"	R4	#4	188	3'-3"
		R9	#6	120	9'-8"				
		T1	#6	81	25'-8"	R5	#3	332	3'-6"
						R6	#3	44	4'-6"
		A3	#5	20	33'-8"	R7	#3	208	4'-10"
	S5	#4	2	8'-0"	T3-T16			⊗	
	S12	#4	28	7'-9"					
	S13	#4	28	8'-9"					
	S18	#4	2	13'-6"					
	SC1	#4	66	6'-6"					
	T2	#4	62	25'-8"					
Pier Beam	PB4	#6	12	24'-8"	PB5	#7	10	25'-8"	
	PB2	#5	12	25'-8"	PB1	#4	104	8'-2"	
	PB3	#5	4	24'-8"					

⊗ See Bending Diagram

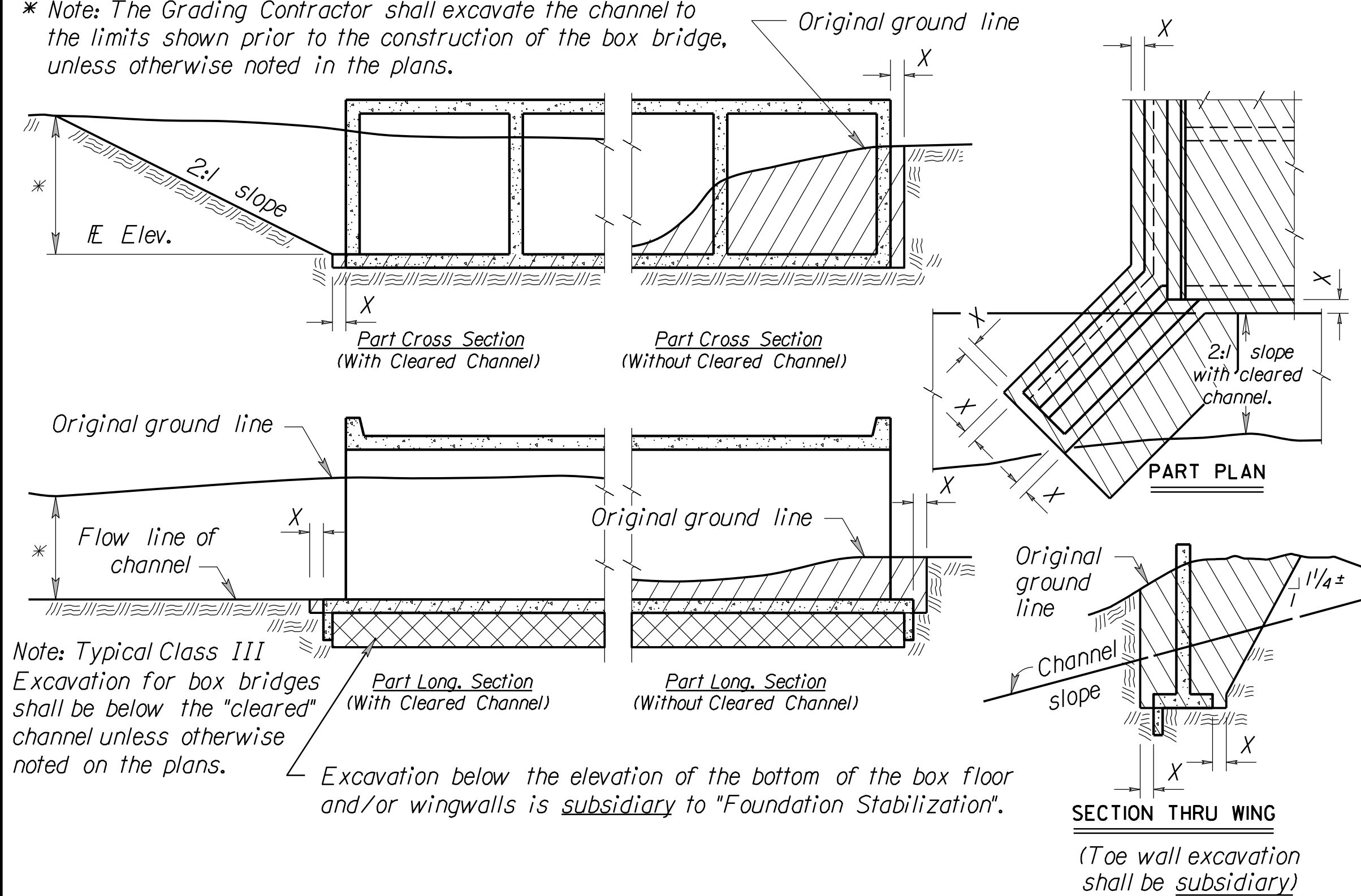
3				
2				
1	7/30/09	corrected Qty. on note to Designer	DRT	KFH
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION

BILL OF REINFORCING STEEL
AND
BENDING DIAGRAM

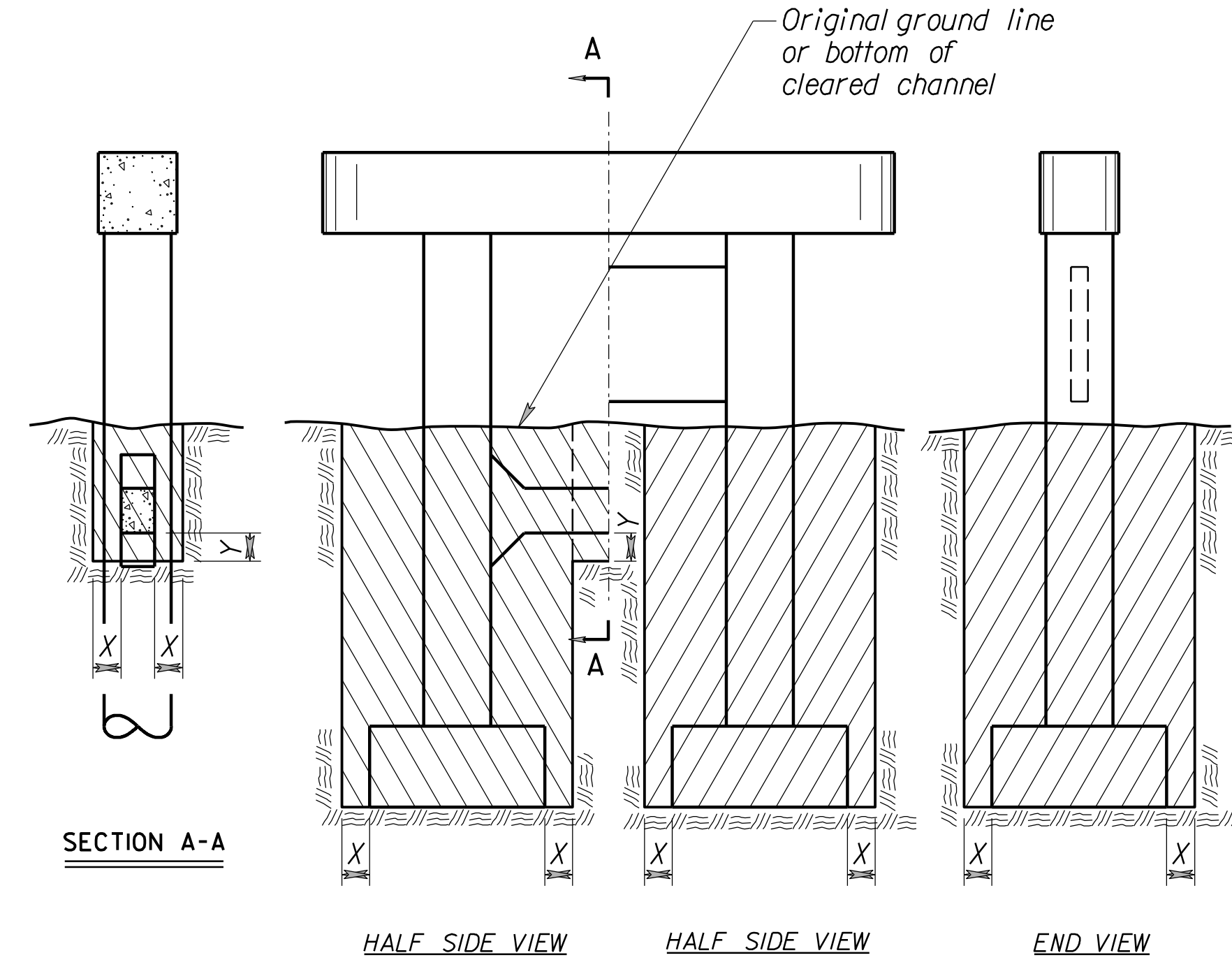
SHEET NO.	OF	SCALE	APP'D	
DESIGNED	DRT	DETAILED DRT	QUANTITIES	CADD RCJ
DESIGN CK.		DETAIL CK.	QUAN. CK.	CADD CK.

* Note: The Grading Contractor shall excavate the channel to the limits shown prior to the construction of the box bridge, unless otherwise noted in the plans.



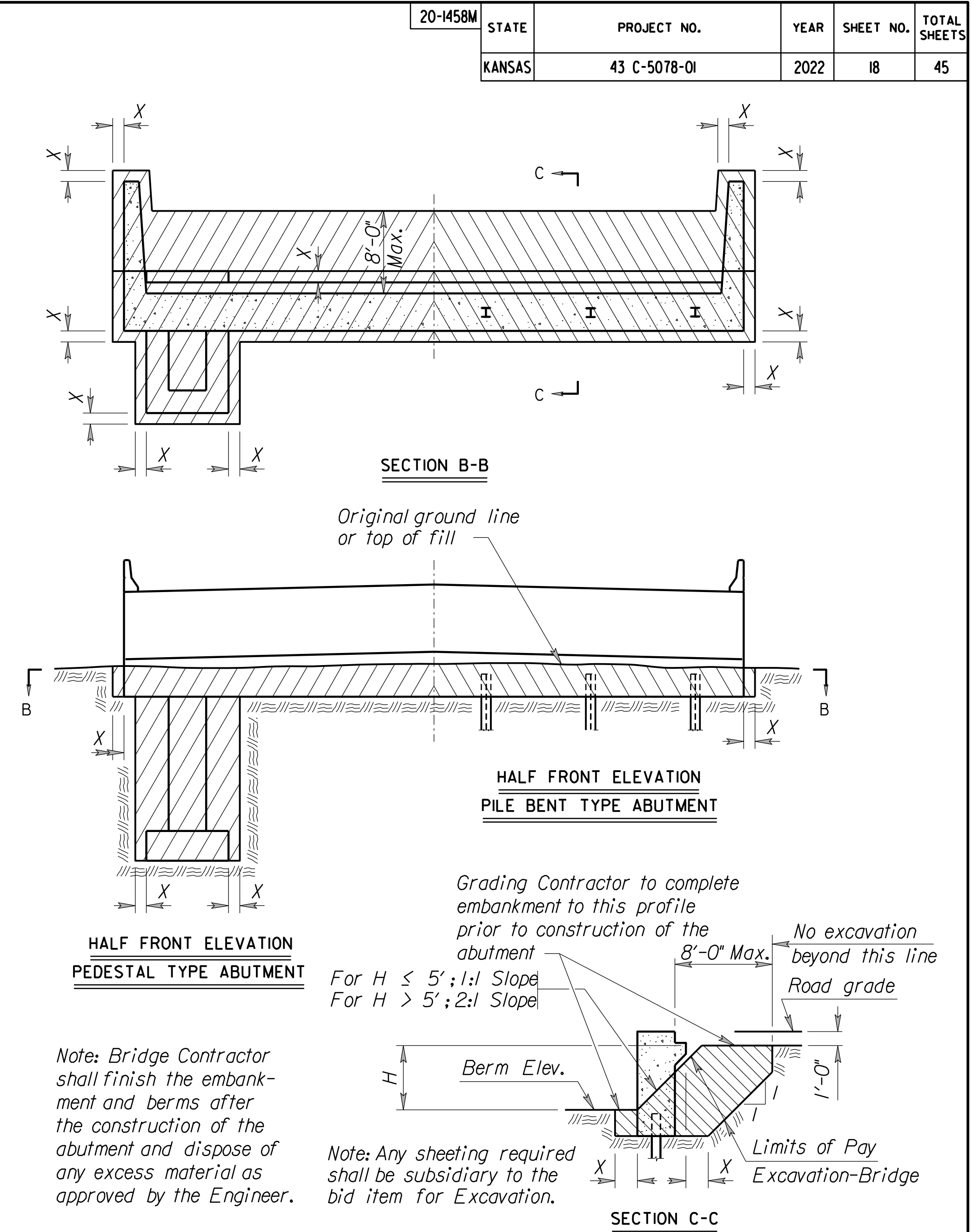
EXCAVATION DETAILS FOR REINFORCED CONCRETE BOX CULVERT

Note: Excavation for culverts less than bridge length and the additional excavation for "Embedded Structures" shall not be paid for as Class III Excavation, but shall be subsidiary to Grade 4.0 Concrete.



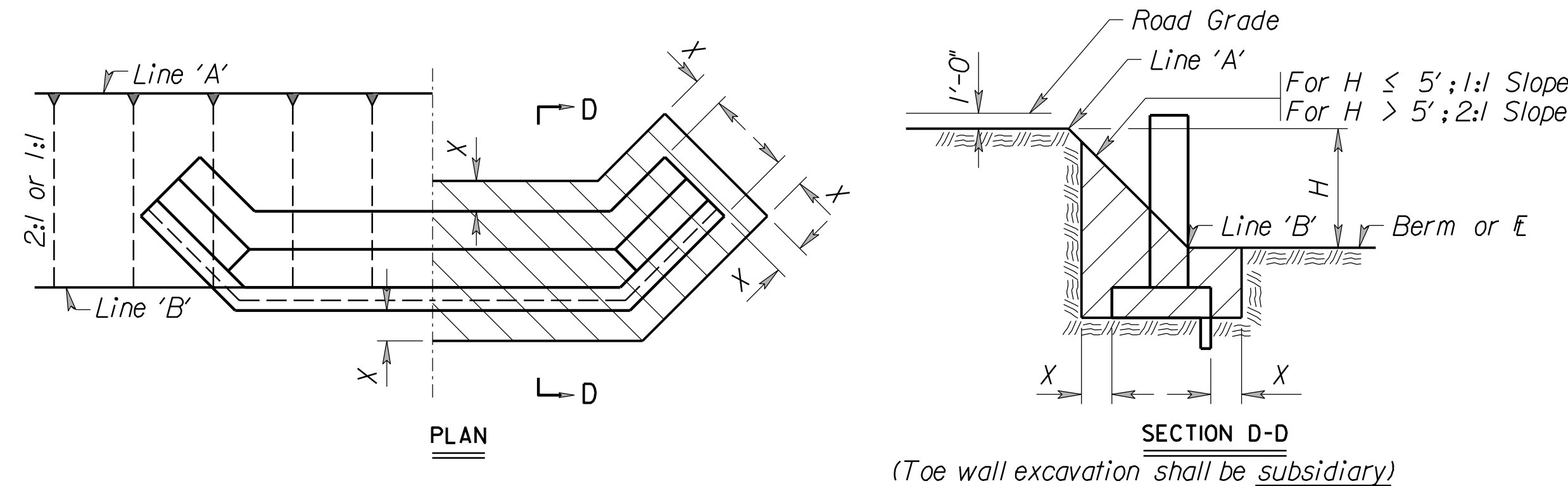
EXCAVATION DETAILS FOR TYPICAL PIERS

See detail when rock or shale (rock) is encountered.

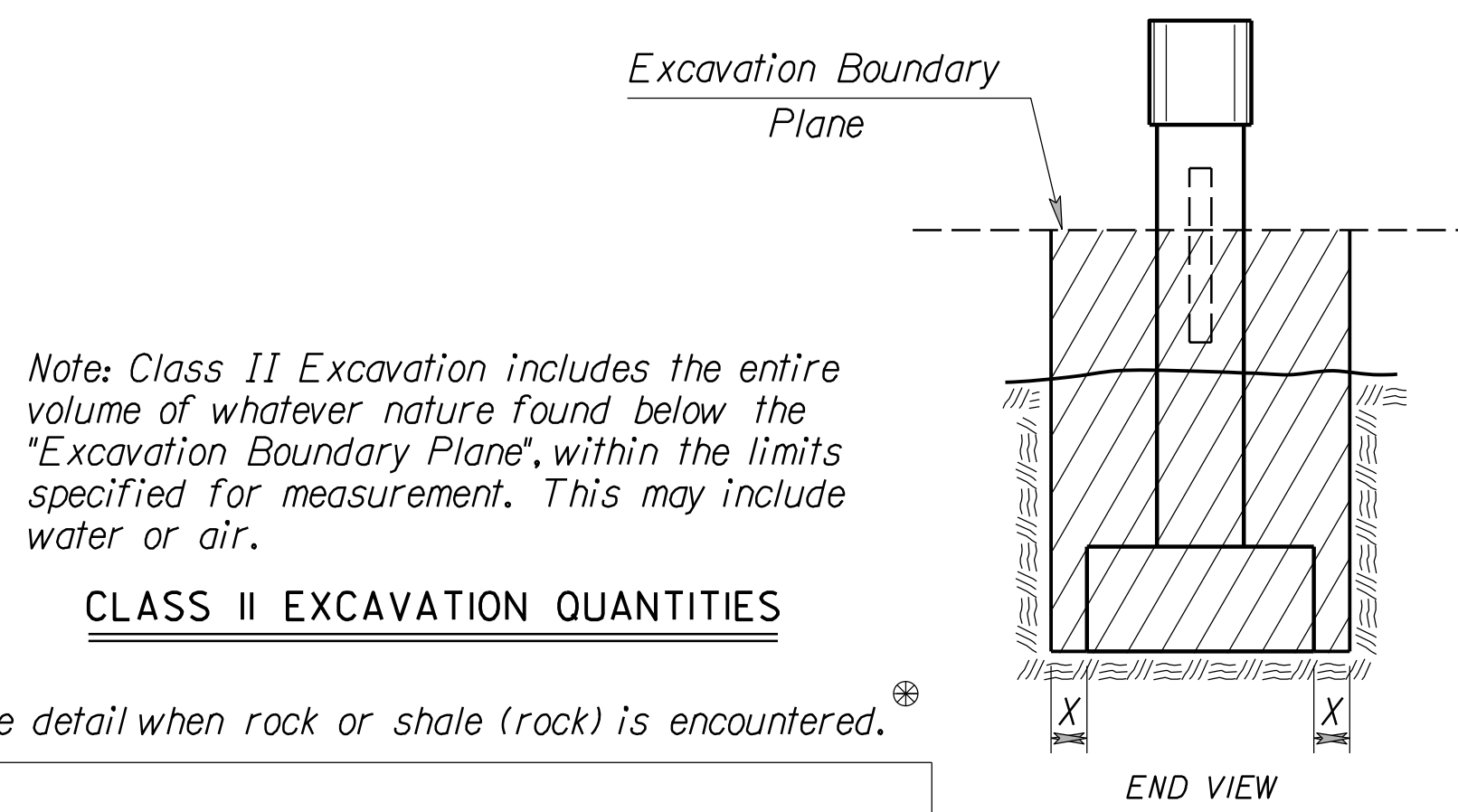


EXCAVATION DETAILS FOR TYPICAL ABUTMENTS

See detail when rock or shale (rock) is encountered.

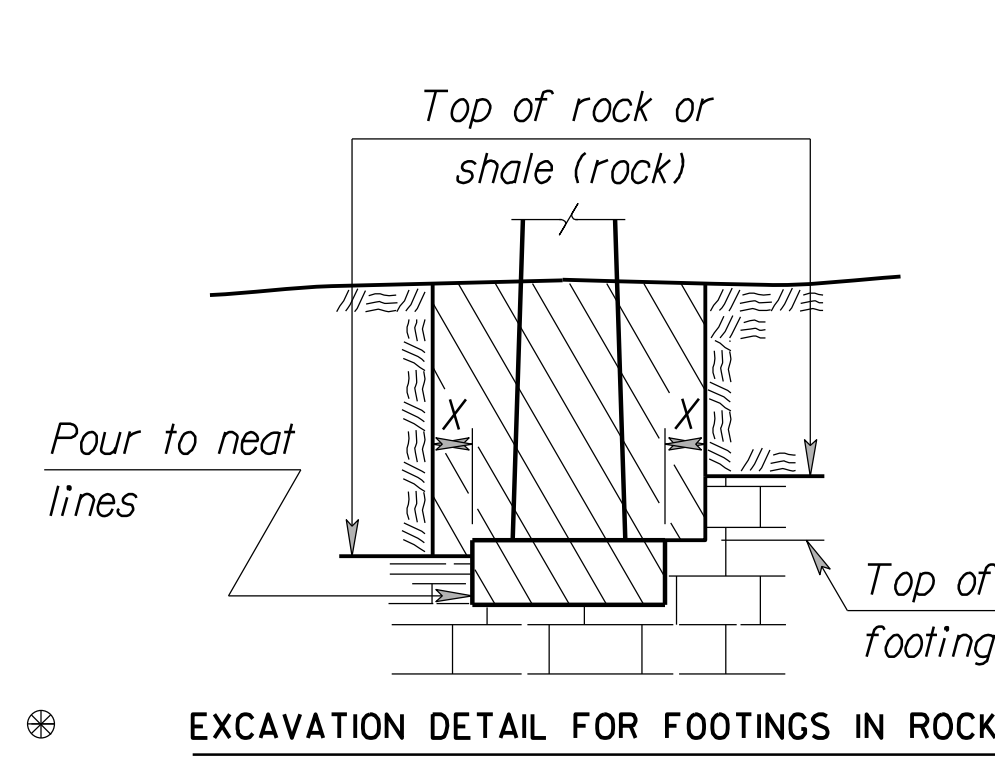


EXCAVATION DETAILS FOR ABUTMENTS WITH FLARED WINGWALLS



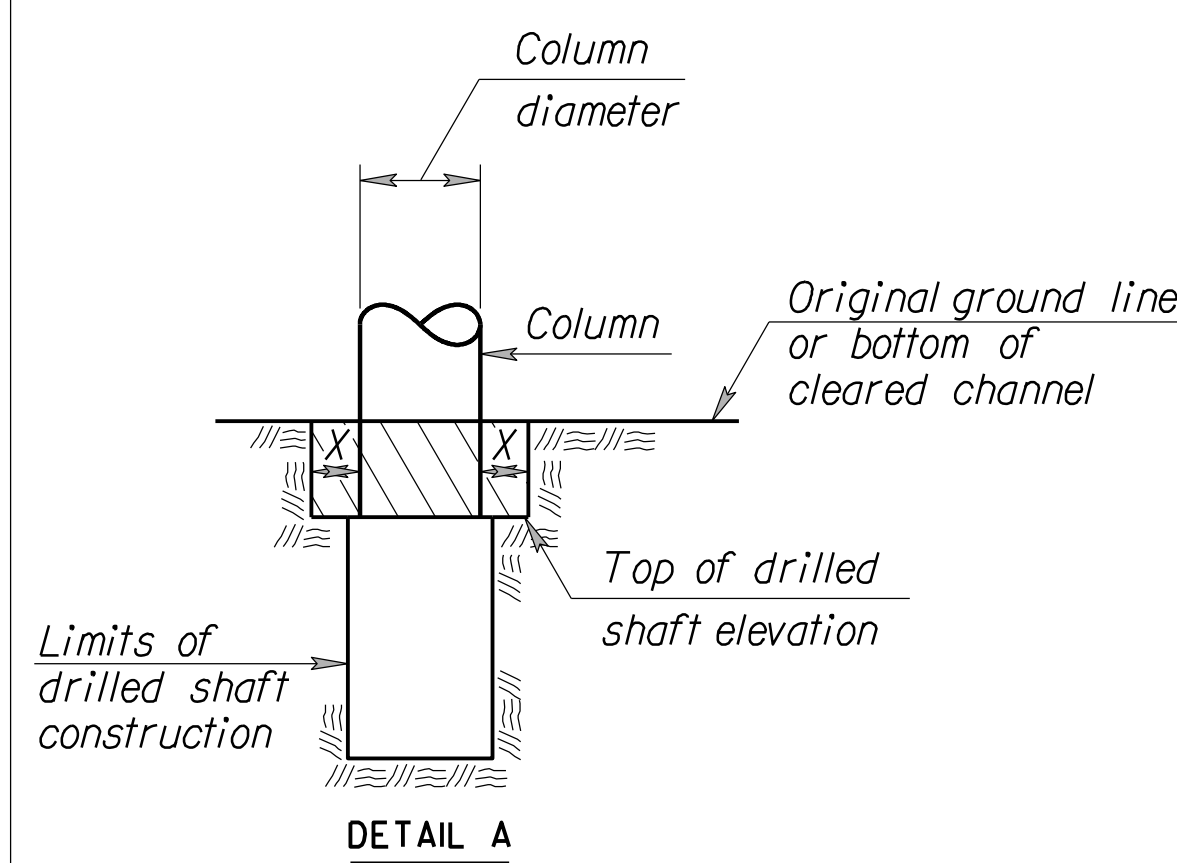
CLASS II EXCAVATION QUANTITIES

See detail when rock or shale (rock) is encountered.

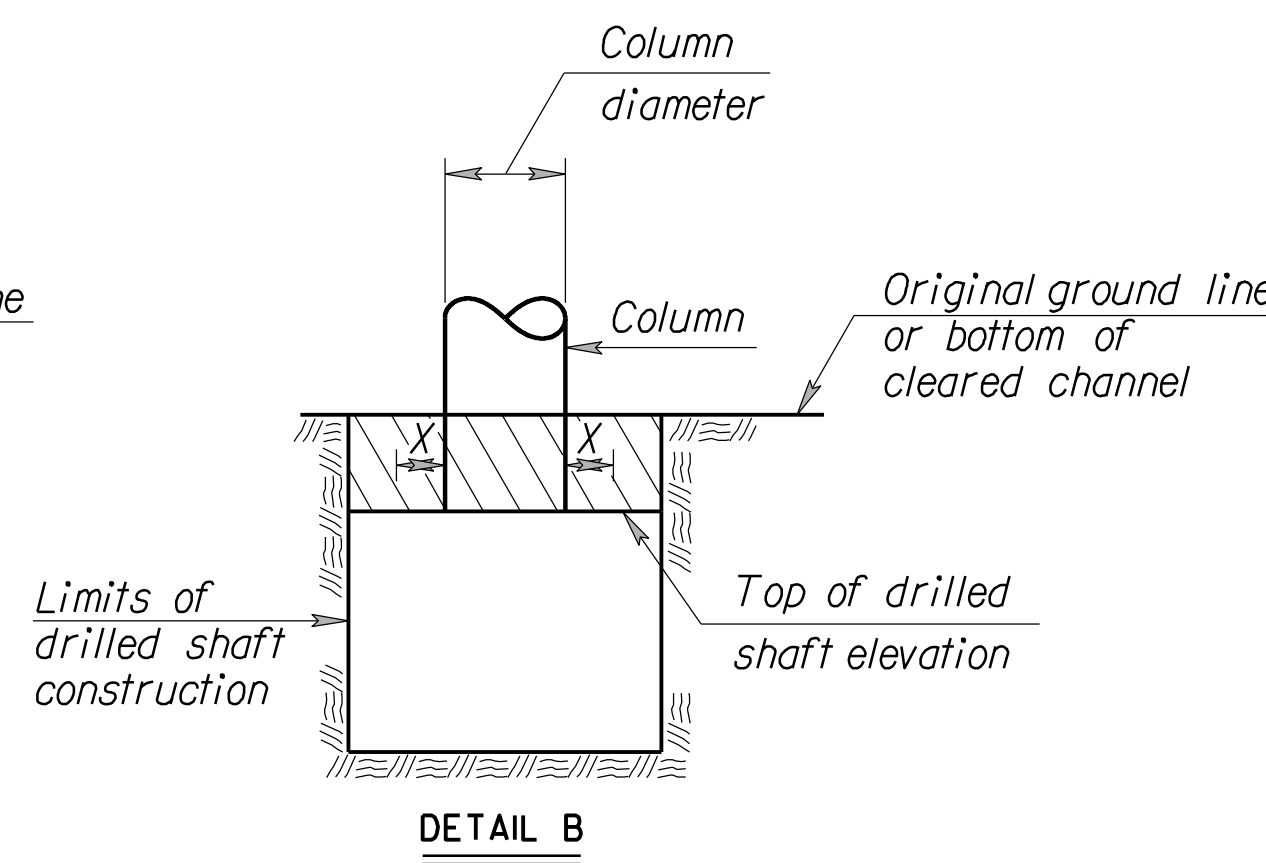


EXCAVATION DETAIL FOR FOOTINGS IN ROCK OR SHALE (ROCK) (Piers and Abutments)

Note: Excavation below top of rock, hard shale or below top of footing, whichever is lower, shall be to neat lines of the concrete construction.



DETAIL A



DETAIL B

DRILLED SHAFT DETAILS

Note: Whenever the limits of the drilled shaft construction are greater than the Column Diameter + 2X, the limits of Class I, II or III Excavation shall be the limits of the drilled shaft construction. (See Detail B)

Note: All bridge excavation shall be computed on the basis of the cross-hatch areas and boundary lines indicated on this sheet and the Excavation Boundary Plane on the Construction Layout.

Sides of trenches in hard or compacted soil including embankments shall be shored, sheeted, braced or otherwise supported when the trench is more than 5 feet in depth and 8 feet or more in length. In lieu of the shoring, the sides of the trench above the 5 foot level may be sloped to preclude collapse. The slope for average soils shall be 1:1. If the angle of repose of the soil is less, flatter slopes shall be required.

Dimension "X" shall be 2'-0" unless indicated otherwise on the general plans.
Dimension "Y" shall be 1'-6" unless indicated otherwise on the general plans.

NO.	DATE	REVISIONS	BY	APP'D
7				
6	8-15-12	Embedment Excavation Subsidiary	JPJ	TLF
5	5-15-12	Revised Wing Excavation	JPJ	TLF
4	3-3-10	Revised Wing Excavation	JPJ	TLF
3	10-16-06	Revised 'Foundation Stab.' Note	JPJ	KFH
2	10-19-04	Concrete - Class to Grade	RAM	KFH
1	4-10-02	Added 'Foundation Stab.' Note	RAM	KFH

KANSAS DEPARTMENT OF TRANSPORTATION

BRIDGE EXCAVATION (LRFD)

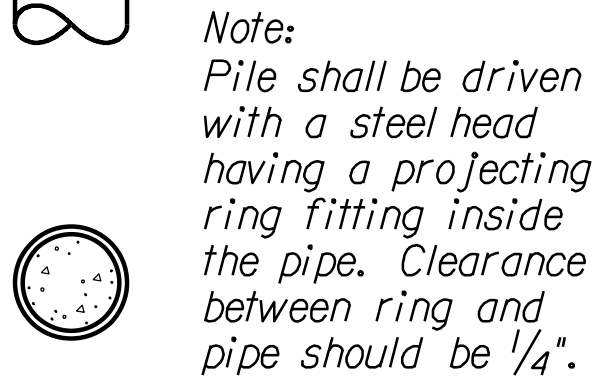
BR100B

DESIGNED	4/17/10 APP'D	TERRY L. FLECK
DETAIL CK.	RDR QUANTITIES	CADD
DESIGN CK.	LRR QUAN CK.	CADD CK.

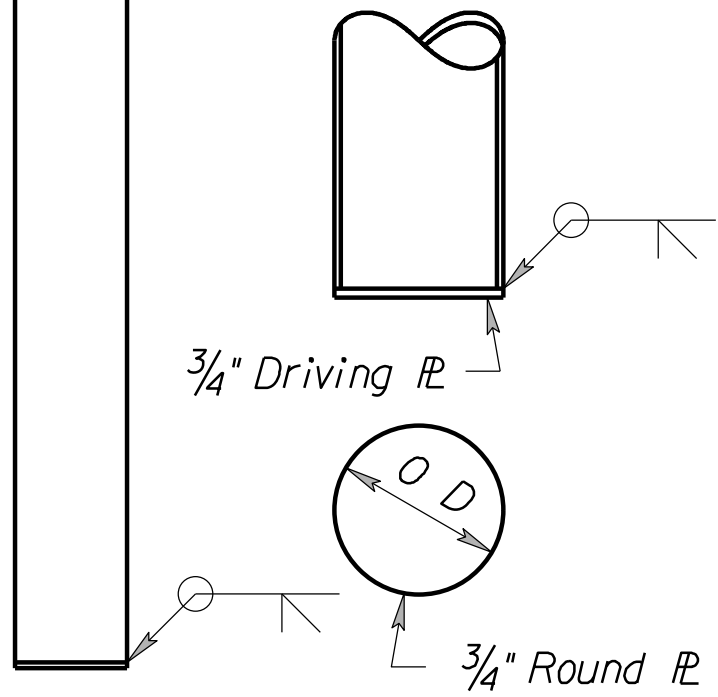
Std. Base File: br100.dgn
Plotted By: unfiled
File: W:\M-20\20-1458M\CAD\Drawing Set\18-br100b.dgn
Plot Date: 8/30/2021

O D 10 3/4" T. = ††
O D 12 3/4" T. = ††
O D 14" T. = ††

†† See the Geology Report or "Summary of Quantities" for Pipe Pile wall thickness



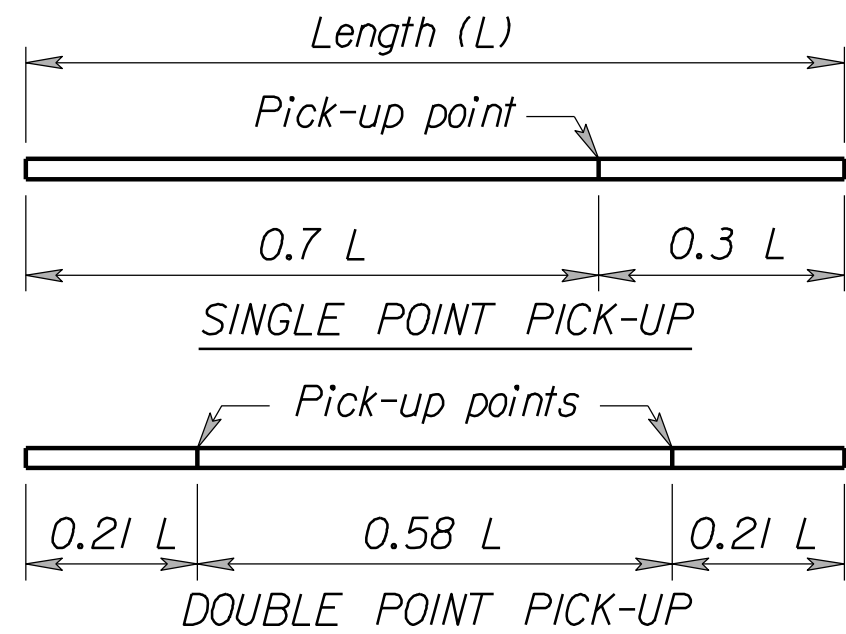
Note: Pile pipe may be spiral welded, longitudinal welded, or seamless steel pipe.



PLAIN ROUND CAST-IN-PLACE CONCRETE PILES

CAST STEEL PILE POINT

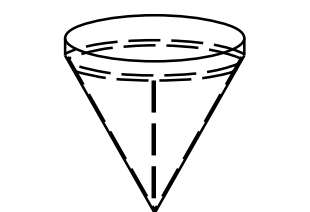
The pile point shall be a one-piece unit of cast steel. Weld pile points in accordance with manufacturers recommendations to each steel pile before driving.



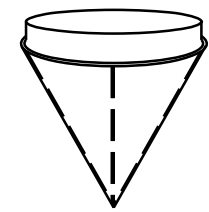
PICK-UP POINTS FOR PRESTRESSED PILING

Max. length - 55' single point pick-up
Max. length - 80' double point pick-up

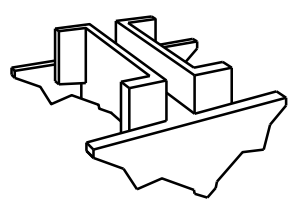
Note: Piles shall be marked at Pick-up points to indicate proper points for attaching handling lines.



Outside Flange



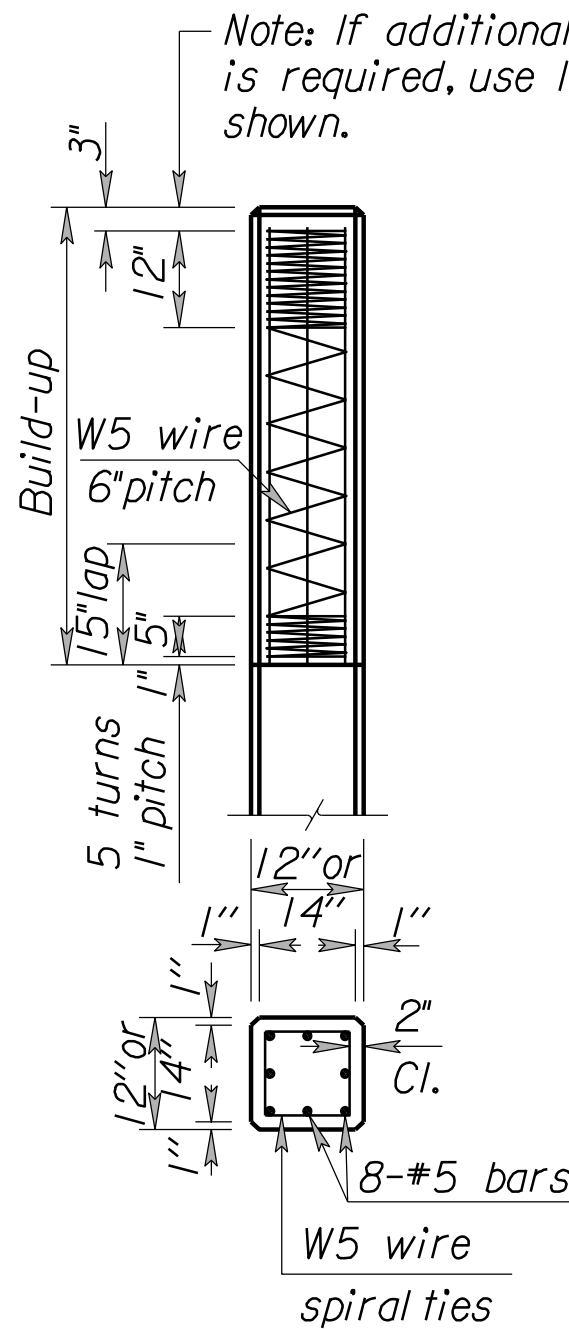
Inside Flange



H-Pile Point

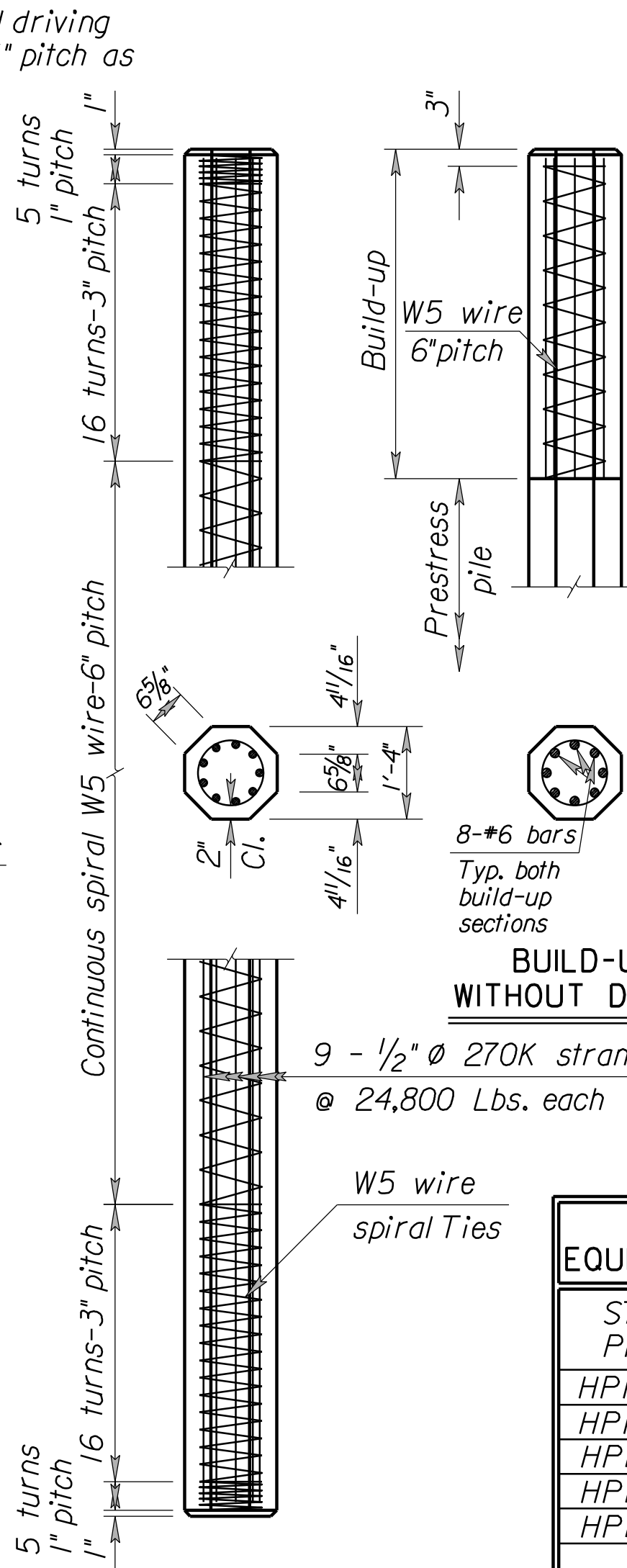
PIPE PILE POINT

Note: If additional driving is required, use 1" pitch as shown.



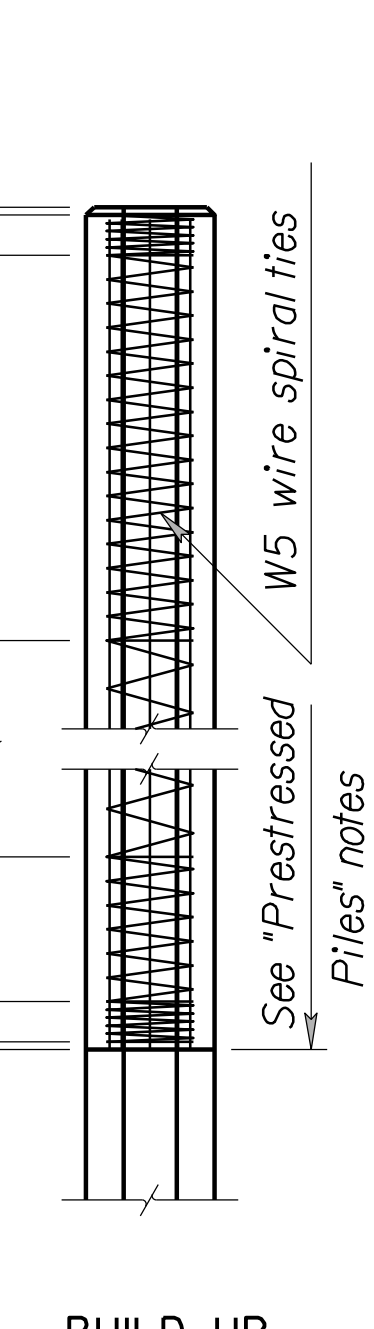
BUILD UP SECTION

8 - 3/8" Ø 270K strands
@ 16,000 Lbs. each
12" x 12" piles
8 - 1/2" Ø 270K strands
@ 22,700 Lbs. each
14" x 14" piles



BUILD-UP WITHOUT DRIVING

9 - 1/2" Ø 270K strands
@ 24,800 Lbs. each



BUILD-UP WITH DRIVING

FOR INFORMATION ONLY EQUIVALENT POINT BEARING PILES		
STEEL PILES	CONCRETE PILES	
	Pipe	Pre-stress
HPI10x42	10 3/4"	
HPI12x53	12 3/4"	
HPI14x73	14	12
HPI14x102		14
HPI14x117		16

16" PRESTRESSED CONCRETE PILES

Weld Symbology Definition

Use grinder to bevel edges of splice as shown in weld symbology and drawing. In addition to bevels, produce clean, bare, and shiny surfaces at and around the splice welding location.

Use E7018, 7016, or 7015 series welding rod (electrode) for all welding applications during pile splicing. See General Notes for proper storage of welding rod.

Lay full penetration root weld from beveled side of splice.

Back gouge root weld from side opposite of root welding application making sure to remove all foreign materials, porous steel, and inclusions from root weld. Finish welding the non beveled side of the splice.

Finish welding beveled side of the splice while removing slag, foreign materials, porous steel, and inclusions in between welding passes, use of a grinder may be needed.

Verify that enough filler metal has been correctly placed in all weld locations to obtain a flush or convex surface with no concavity produced upon completion of the final welds.

SPLICES: Splices for steel piles and shell piling shall be in accordance with details shown on this sheet and the Standard Specifications.

For integral pile bent abutments and piers, if a pile splice is required, do not locate the pile splice within a region extending 2'-0" above and 10'-0" below the bottom of the concrete web wall. For abutments, locate the pile splice at least 10'-0" below top of fill.

With the approval of the Engineer, one splice per bent may be allowed in the region described above without testing. If additional splices are anticipated, based on the geology, the Contractor prior to driving, will locate the splice so that the splice will not fall within the regions described above.

† For integral pile bent abutments and piers, if a splice is located within the regions described above, then the Contractor will test the welds by Radiograph (RT) test methods. Repair and retest any welds not passing the test(s). Each weld tested will have written confirmation of results. Report these results to the Engineer. This work is not paid for directly, but is subsidiary to "Piles".

* Minimum as required by welding process.

BG = Backgouge

GENERAL NOTES

PRESTRESSED PILES: Fabricate prestressed concrete pile splices in accordance with the Manufacturer's recommendations subject to the approval of the Engineer.

Method of attachment of pile to build-up may be by any of the methods* given in the notes on "Alternate Methods. If mild reinforcing steel is used for attachment, the area shall be no less than that used in the build-up.

ALTERNATE METHODS: Method of attachment of a pile to build-up may be by any of the following methods:

1. Cut off at least 2'-0" of pile and expose a minimum of 2'-0" of strands.
 2. Cast 8-#6, or 8-#5 bars (equally spaced) into pile head. All bars shall extend into pile head and project from pile head a minimum of 2'-0".
 3. Drill 8 holes in pile head (equally spaced) for installation of 8 grouted dowel bars of same size and length as in 2.
 4. Provide cored holes for bars as in 3.
- No bars or strands are to extend from head of pile or build-up into footing or pile cap unless approved by the Engineer.

TEST PILES: Drive test piles where called for on the bridge plans. The test piles located within the limits of the substructure will become a part of the bridge pile system.

DRIVING FORMULA: Driving formula shall conform to the Standard Specifications.

MEASUREMENT AND PAYMENT: Measurement and payment for all piles shall comply with the Standard Specifications.

The following items are covered in Division 1000 of the Standard Specifications:

REINFORCEMENT: Use reinforcing steel conforming to ASTM A615, Grade 60. Hoops and spirals may be either plain or deformed bars.

PRESTRESSING STEEL: Use uncoated seven-wire stress relieved or low relaxation prestressing strand conforming to ASTM A416, Gr. 270.

SPECIFICATIONS: Standard Specifications for State Road and Bridge Construction as currently used by the Kansas Department of Transportation. The following items are covered in Division 700 of the Standard Specifications:

CONCRETE: Concrete for cast-in-place shall be f'c = 3,500 PSI.. Concrete for prestressed shall be f'c = 5,000 PSI.

WELDING: All field welding shall meet the requirements of the Standard Specifications.

Use only Shielded Metal Arch Welding SMAW (stick welding) for pile splices.

Use only low hydrogen E7018, 7016, or 7015 series welding rod (electrode) for all welding applications during pile splicing. See General Notes or proper storage of welding rod, welding filler rod (electrode) for field welding of splices.

New electrode are to be purchased for each KDOT project. The electrode shall arrive on the project in factory hermetically sealed containers opened and labeled with indelible ink in front of the engineer. The label shall include the current date and the project number. If the container seal is questionable or shows signs of damage the electrode is to be dried in an oven at least one hour at a temperature of 700°F to 800°F.

Upon removal from intact hermetically sealed factory packaging or the drying oven the electrode is to be placed in a storage oven with a minimum temperature of 250°F.

When electrodes are removed from the hermetically sealed container or storage oven and exposed to the atmosphere for less than 4 hours place into the storage oven for at least 4 hours before removing for use.

If electrode is exposed to the atmosphere for 4 hours or more (or 9 hours for moisture resistant electrodes designated with an R in their labeling) then electrode can be dried in a drying oven at a temperature of 450°F to 550°F.

If the electrode is exposed to the atmosphere for 4 hours or more a second time or the rod becomes wet discard rod.

CAST-IN-PLACE SHELLS: Steel shells for cast-in-place piles shall conform to the requirements of the Standard Specifications.

All piles driven without a mandrel shall be of the minimum thicknesses shown. Piles driven with a mandrel shall be of sufficient strength and thickness to withstand driving without injury and to resist harmful distortion and/or buckling due to soil pressure after the mandrel is removed.

Remove, replace or correct to the satisfaction of the Engineer improperly driven, broken or otherwise defective pipe piles. Otherwise drive an additional pile at no extra cost.

The Contractor shall maintain a light suitable for visual inspection of the pile on the job at all times prior to and during the filling of the pipe.

STEEL PILE: Steel pile shall conform to the requirements of the Standard Specifications.

PILE POINTS: Pile points shall conform to the dimensions shown and to requirements of the Standard Specifications.

PAINT: All paint shall comply with the Standard Specifications, or as specified on the plans.

MILL TEST REPORTS: Steel piles test reports and steel shell test reports shall comply with the Standard Specifications.

NO.	DATE	REVISIONS	BY	APP'D
4	08-16-18	Add splice web section, clarify note	MLL	JPJ
3	09-15-15	Clarify Notes		JPJ CER
2	06-18-12	Clarify fc, rod type, use and weld	JPJ	TLF
1	1-5-09	Pile Splice Location and Weld Test	JPJ	KFH

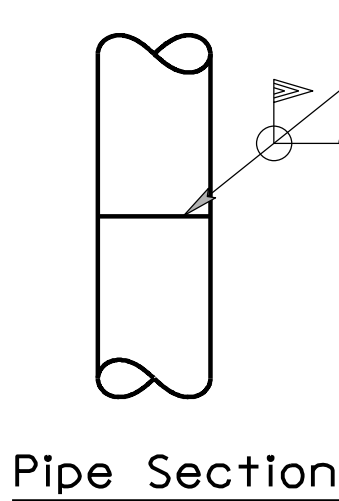
KANSAS DEPARTMENT OF TRANSPORTATION

STANDARD PILE DETAILS

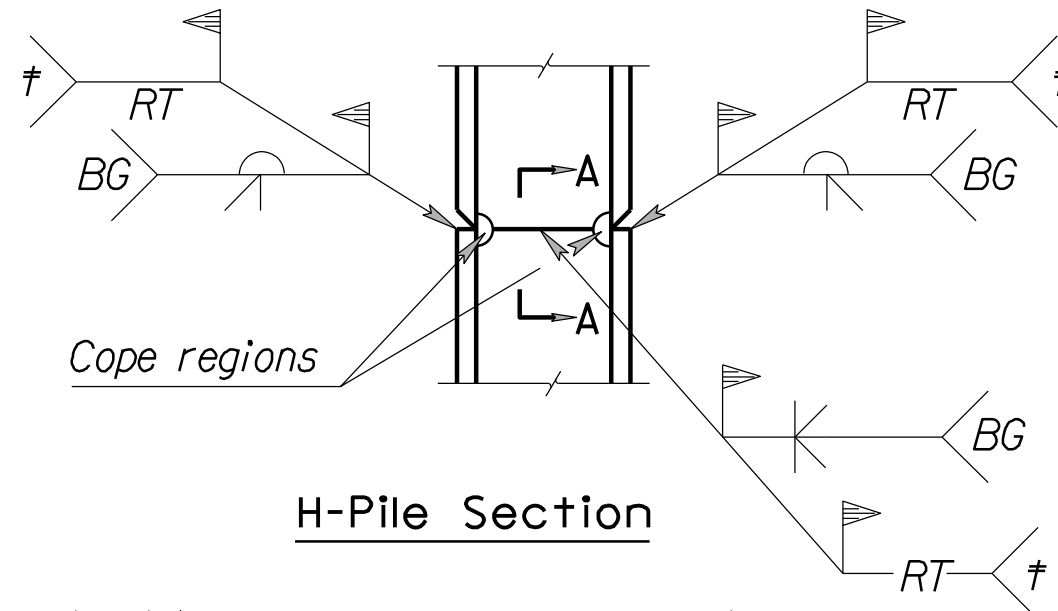
BRIIO

FHWA APPROVAL	IO-04-12	APP'D	Terry L. Fleck
DESIGNED JPJ	QUANTITIES	CADD	RAA
DESIGN CK.	DETAIL CK.	QUAN. CK.	CADD CK.

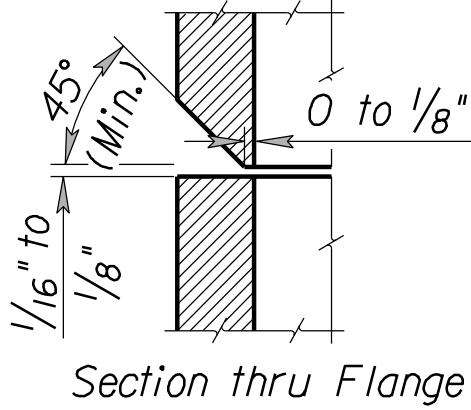
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Plot Location: br110.dgn
File: WAM-20V20-1458MCAADDrawing SetV9-br110.dgn
Plot Date: 8/30/2021



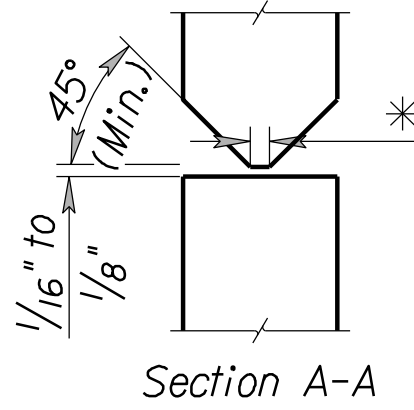
Pipe Section



H-Pile Section



Section thru Flange



Section A-A
(Thru web)

PILE SPLICE DETAILS

GENERAL NOTES

Reference is made to the latest edition of the CRSI "Manual of Standard Practice" for recommended industry practices concerning reinforcing steel.

Use only the following types of bar supports:

- 1) Wire Bar Supports:
- a) Epoxy coated reinforcing: Class 1 Protection

b) Non-epoxy coated reinforcing: Class 1, 2, or 3 Protection
- 2) Plastic Bar Supports
- 3) Supplementary bars

When securing epoxy coated reinforcement, use tie wires or metal clips that are epoxy or plastic coated.

Do not weld reinforcing steel to bar supports or to other reinforcing steel. Shop weld spacer frames for haunched slabs.

Tie bars at all intersections around the perimeter of each mat and at not less than 2'-0" centers or at every intersection, whichever is greater.

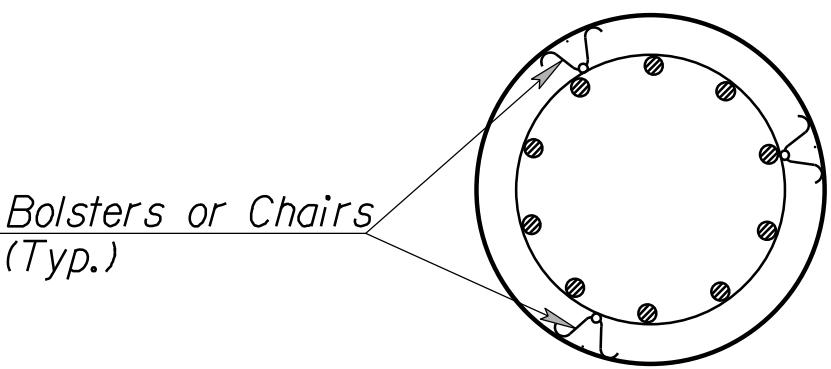
Where more than one length of bar support is required, lap the end legs so they are locked or tied together.

Use proper height supports to maintain the distance between the reinforcing and the formed surface or the top surface of deck slabs within 1/4" of that indicated on the plans.

Spacings shown are maximums. Use sufficient supports, as determined by the Engineer, to retain the reinforcing steel in position.

Construct any platforms, required for the support of workers and/or equipment during concrete placement, directly on the forms and not on the reinforcing steel.

Designs and arrangements of Supports or Spacers other than as shown on this sheet, may be used with the permission of the Engineer.



SECTION A-A

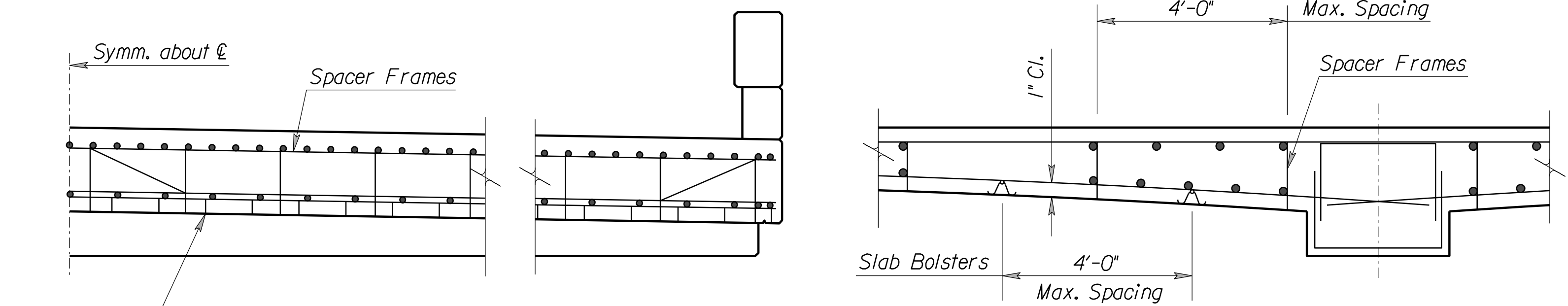
5	11-10-10	Column Bar Supports Req'd	JPJ	TLF
4	12-01-05	Drilled Shaft Spiral Steel Placement	JPJ	KFH
3	8-21-00	Added Pre-Cast Panel Detail	RAM	KFH
2	12-20-99	Added Haunched Slab Bolsters	RAM	KFH
1	12-09-99	Revised Drilled Shaft Clearance	RAM	KFH
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION

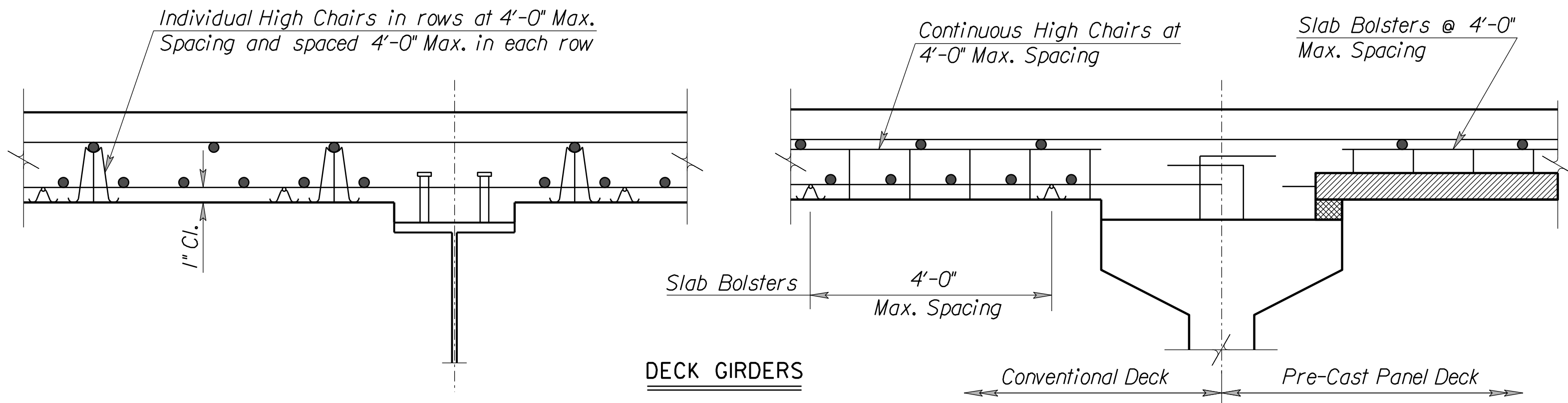
SUPPORTS AND SPACERS
FOR
REINFORCING STEEL

BR120

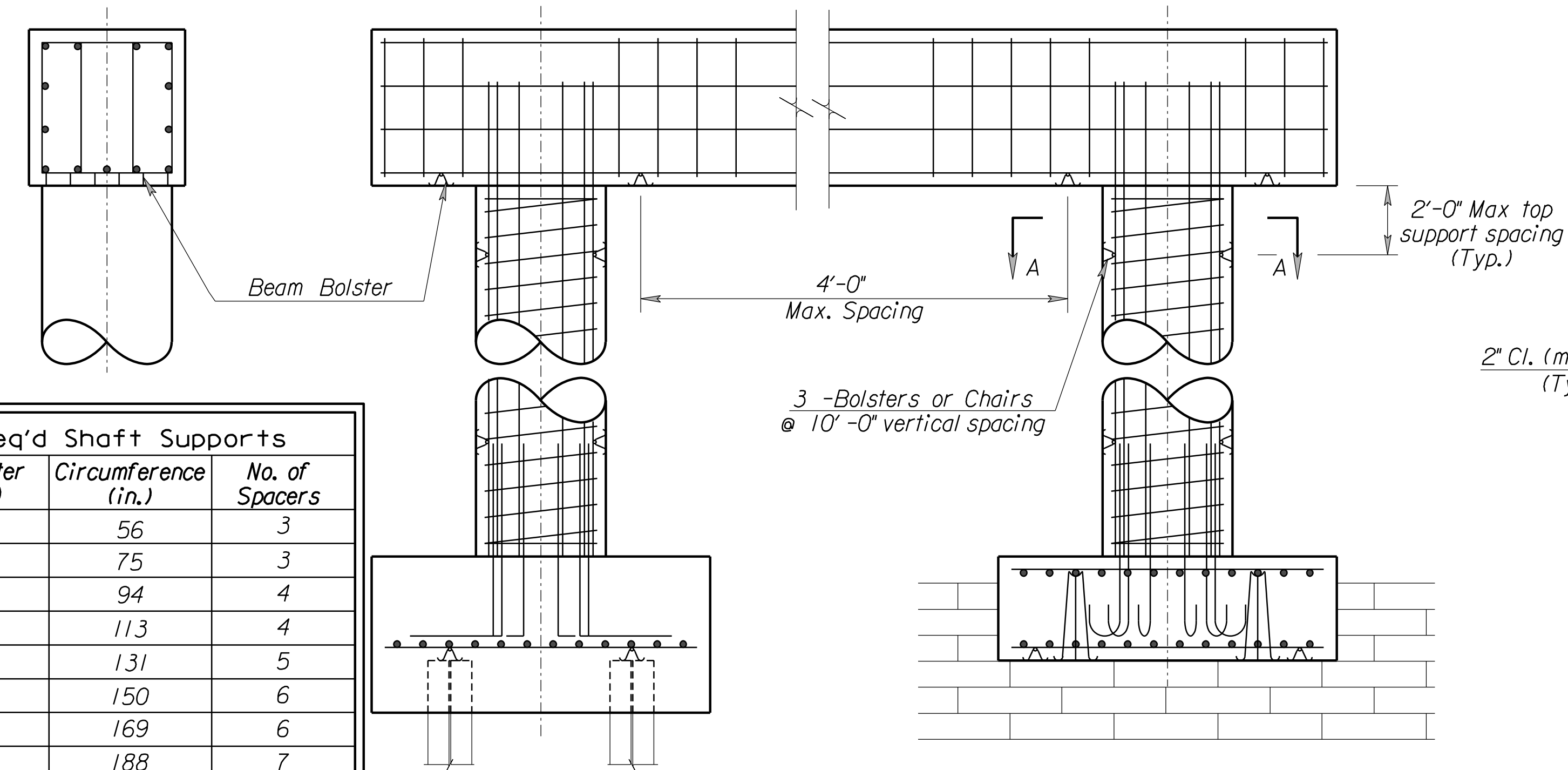
FHWA APPROVAL	11-17-10	APP'D	Terry L. Fleck
DESIGNED	RAM	DETAILED	RAA
DESIGN CK.	LRR	DETAIL CK.	RAM



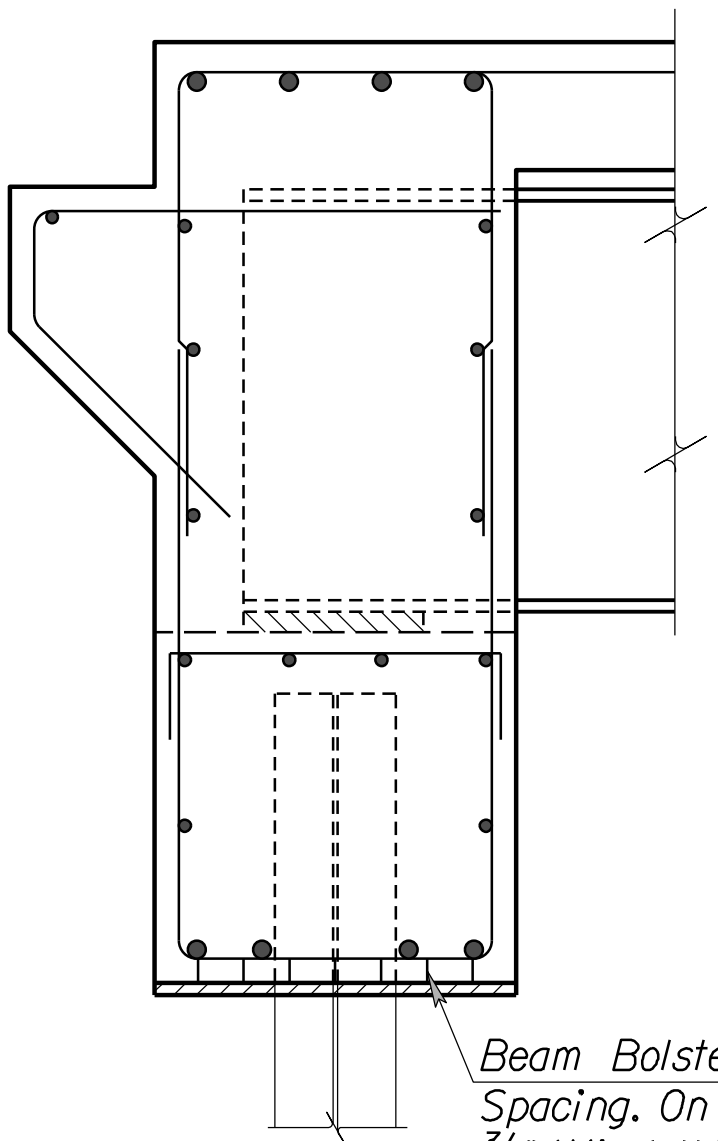
CONTINUOUS HAUNCHED SLAB



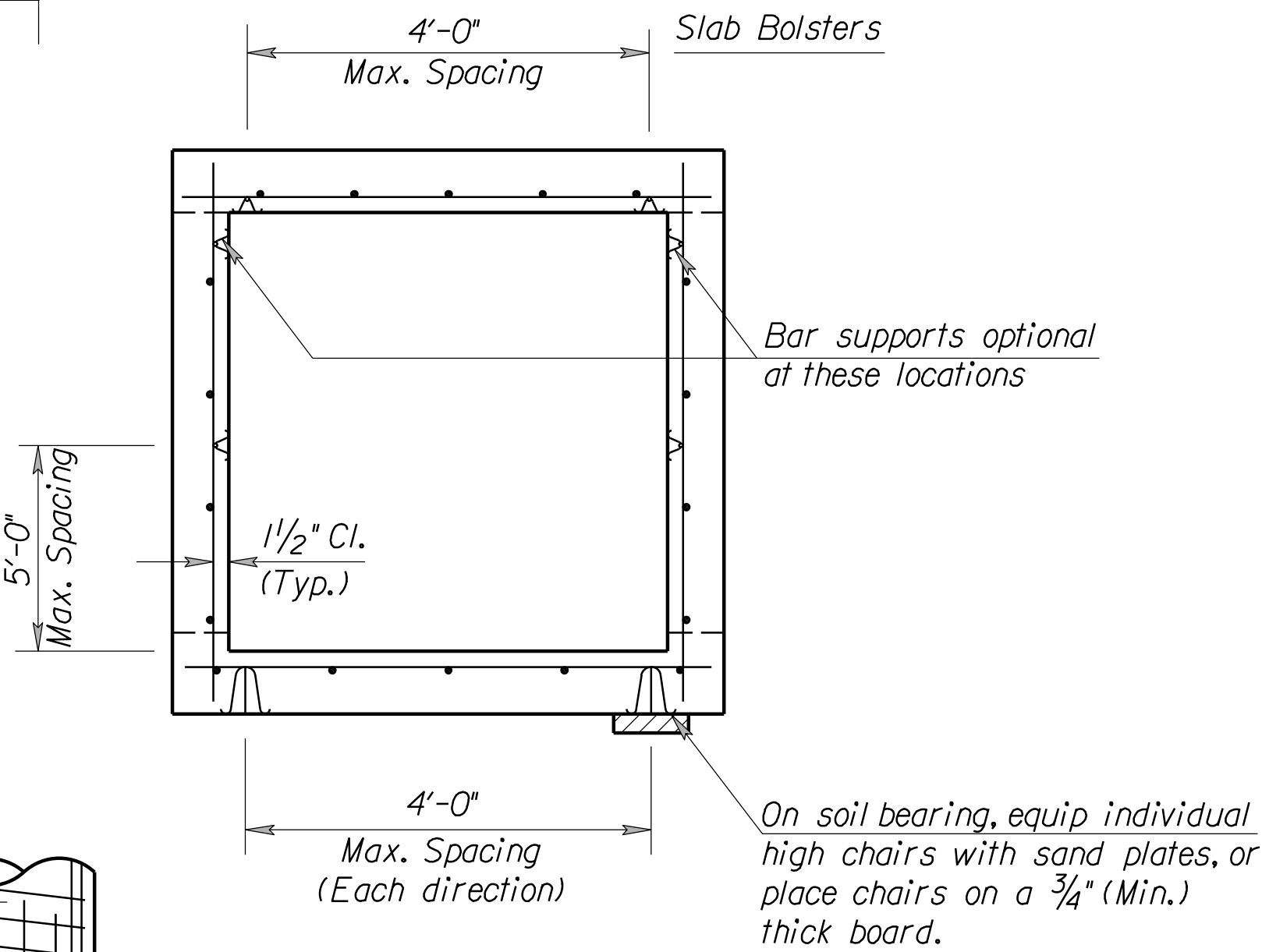
DECK GIRDERS



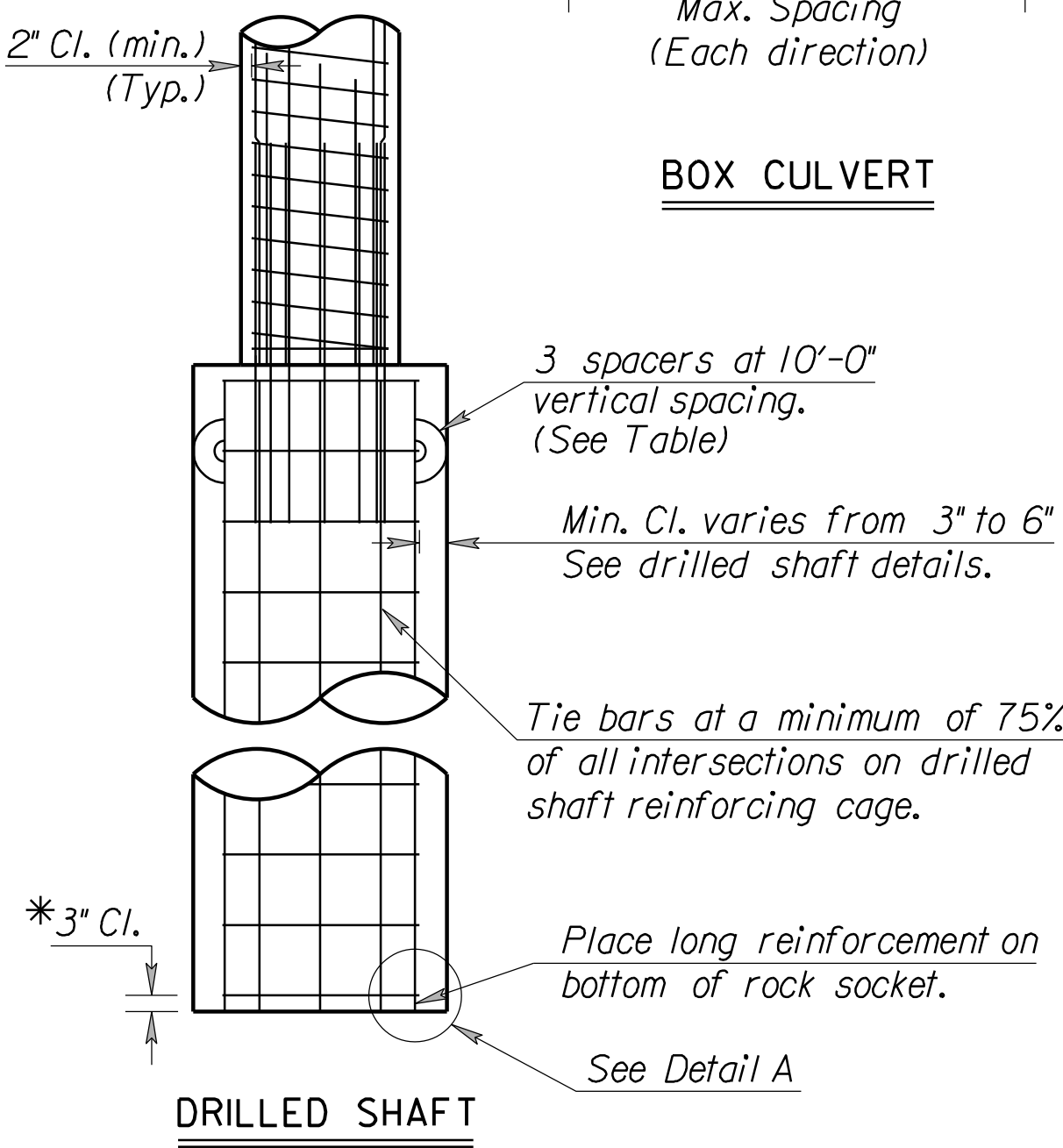
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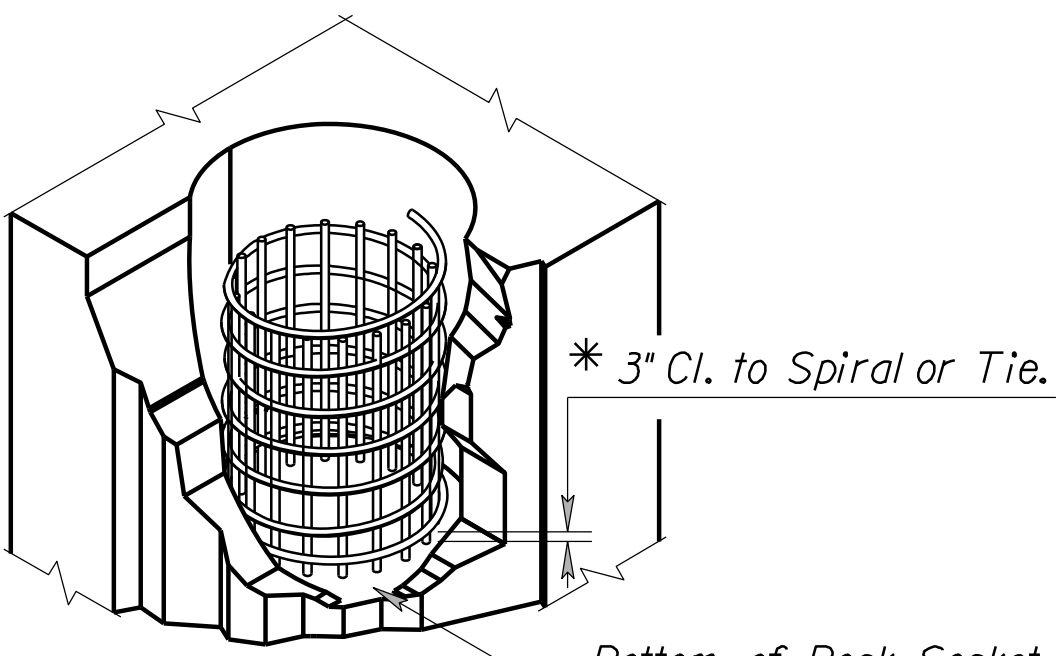
ABUTMENT



BOX CULVERT



DRILLED SHAFT



DETAIL A

* Note: Longitudinal reinforcing steel is placed on the bottom of the rock socket. Maintain 3" clearance from the bottom of rock socket to the first spiral or tie bar.

Req'd Shaft Supports		
Diameter (in.)	Circumference (in.)	No. of Spacers
18	56	3
24	75	3
30	94	4
36	113	4
42	131	5
48	150	6
54	169	6
60	188	7
66	207	7
72	226	8
78	244	9
84	263	9
90	282	10
96	301	11
102	320	11
108	339	12

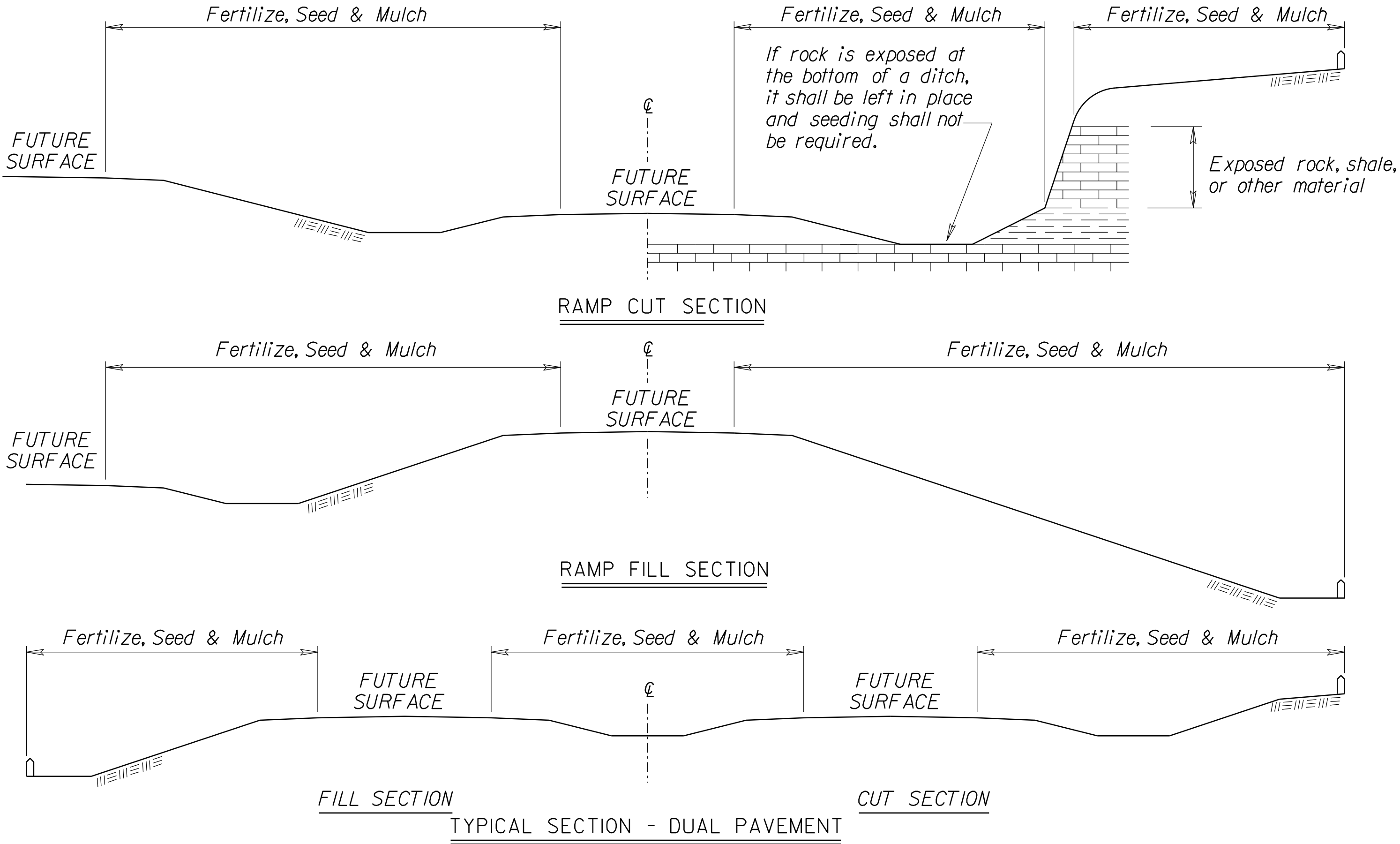
Std. Base File:

Plot Location:

Plotted By: unfiled

File: M:\M-20\20-1458M\CADD\Drawing Set\22-14852a.dgn

Plot Date: 12/3/2021



FERTILIZER: A ratio and application rate that equals or exceeds the required minimum rate per acre of N, P₂O₅, K₂O listed in Summary of Quantities will be acceptable.

- * - N = Nitrogen Rate of Application
- ** - P₂O₅ = Phosphorous Rate of Application
- *** - K₂O = Potassium Rate of Application

The Contractor will be required to finish areas of excavation, borrow and embankment in accordance with the specifications. Areas that require installation or construction of temporary water pollution control items will be finished in reasonable close conformity to the alignment, grade and cross section shown on the plans or as established by the Engineer.

CLT = Construction Limit Tract. This area is defined by the entire disturbed area of the project that requires seeding and erosion control measures to be placed. Any impervious areas (i.e. pavement, gravel, riprap, etc.) shall not be included in this measurement.

Slope = Defined by the area of the project that requires Class 1 erosion control material to be placed. This area shall be seeded using the Soil Erosion Mix prior to placement of the material. Drilling seed is preferred, however, broadcasting is acceptable if drilling is not possible.

Channel = Defined by the area of the project that requires Class 2 erosion control material to be placed. This area shall be seeded using the Soil Erosion Mix prior to placement of the material. Drilling seed is preferred, however, broadcasting is acceptable if drilling is not possible.

GENERAL NOTES

The entire disturbed area, excepting the paved or surfaced areas, steep rocky slopes and areas of undisturbed native sod or other desirable vegetation shall be fertilized (limed when required), seeded, and mulched. Soil preparation shall conform to the Standard Specifications.

Temporary seeding shall be done during any time of the year that the soil can be cultivated. After the temporary seeding has been completed on the entire project, permanent seeding shall be done during the normal seeding season.

MULCHING: Mulch shall be spread uniformly over all disturbed areas and punched in the soil, unless otherwise noted on the plans. The rate of application per acre, thickness in place, for the mulching materials is generally as follows:

$1\frac{3}{4} - 2\frac{1}{4}$ Tons per Acre = $1\frac{1}{2}$ " loose depth spread uniformly over acre.

Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards. Other vegetative mulches are acceptable only with the Engineer's concurrence.

The above rate is a guide. It will be at the discretion of the Engineer to determine what rate is sufficient for adequate protection of newly seeded areas.

SUMMARY OF SEEDING / EROSION CONTROL QUANTITIES					
P.L.S. RATE/ ACRE		ACRES		BID ITEM	QUANTITY
CLT	SL/CH	CLT	SL/CH		
200		0.48		Temporary Fertilizer (I3-I3-I3)	96
				Temporary Seed (Canada Wildrye)	LB
				Temporary Seed (Grain Oats)	LB
				Temporary Seed (Sterile Wheatgrass)	LB
	169.9		0.48	Soil Erosion Mix	81.6
				Erosion Control (Class 1, Type C)	710
				Erosion Control (Class 2, Type Y)	SO YD
				Sediment Removal (Set Price)	1
				Synthetic Sediment Barrier	LF
				Temporary Berm (Set Price)	1
				Temporary Ditch Check (Rock)	CU YD
				Temporary Inlet Sediment Barrier	EACH
				Temporary Sediment Basin	CU YD
				Temporary Slope Drain	LF
				Temporary Stream Crossing	EACH
				Biodegradable Log (9")	18
				Biodegradable Log (12")	18
				Biodegradable Log (20")	35
				Filter Sock (****)	LF
				Geotextile (Erosion Control)	SO YD
				Silt Fence	35
				SWPPP Design †	LS
				SWPPP Inspection †	EACH
				Water Pollution Control Manager †	EACH
900 lbs / acre				Mulch Tacking Slurry	LB
2 tons / acre				Mulching	TON
				Water (Erosion Control) (Set Price)	1
					MGAL

NOTE: When seeding less than 1 acre, temporary and permanent seeding shall be combined and seeded at the same time. There is no seasonal restriction for seeding projects less than 1 acre.

NOTE: Projects less than 1 acre shall be bid as "Seeding" by the lump sum. See Permanent Seeding Summary of Seeding Quantities sheet LA850 for further details.

Geotextile (Erosion Control) shall be removed prior to placement of permanent slope protection.

Regreen and Quick Guard are the approved sterile wheatgrass products.

† If the total disturbed area of the project, not just the seeding area, is 1 acre or more, then these bid items must be included.

**** List size of material.

The amount of mulch and mulch tacking slurry in the bid quantities is estimated. (Acres of Seeding X 1.5 X 2 Tons/Acre). The estimated quantity includes mulching associated with both temporary and permanent seeding operations. The total mulch and mulch tacking slurry required shall be determined in the field. The bid item for mulching and mulch tacking slurry shall be paid for according to the Standard Specifications.

Quantities for all erosion control items are estimated to give full flexibility for compliance with the NPDES permit. Final quantities will be determined in the field.

SOIL EROSION MIX		
PLS RATE	NAME	QTY (lb)
200	Fertilizer (I3-I3-I3)	*
0.5	Seed (Blue Grama Grass Seed) (Lovington)	0.24
4.5	Seed (Buffalograss) (Treated)	2.16
20	Seed (Canada Wildrye Grass)	9.60
45	Seed (Perennial Ryegrass)	21.60
2.6	Seed (Prairie Junegrass)	1.25
6.3	Seed (Side Oats Grama Grass) (El Reno)	3.02
20	Seed (Sterile Wheatgrass) (Regreen/Quick Guard)	9.60
45	Seed (Tall Fescue) (Endophyte Free)	21.60
6	Seed (Western Wheatgrass) (Barton)	2.88
20	Seed (Grain Oats)	9.60
Total (lb)		81.55

The Soil Erosion Mix is to be placed under the Class 1 and/or Class 2 erosion control material.

The Soil Erosion Mix consists of the Shoulder Area of the Permanent Seed Mix used on the project.

20-1458M	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
	KANSAS	43 C-5078-01	2022	22	44

* Fertilizer quantity for Soil Erosion Mix is included in the Recap Table above.

3	08/03/20	Added Note	MRD	ML
2	12/01/17	Revised Standard	MRD	SHS
1	06/01/17	Revised Standard	MRD	SHS
NO.	DATE	REVISIONS	BY	APP'D

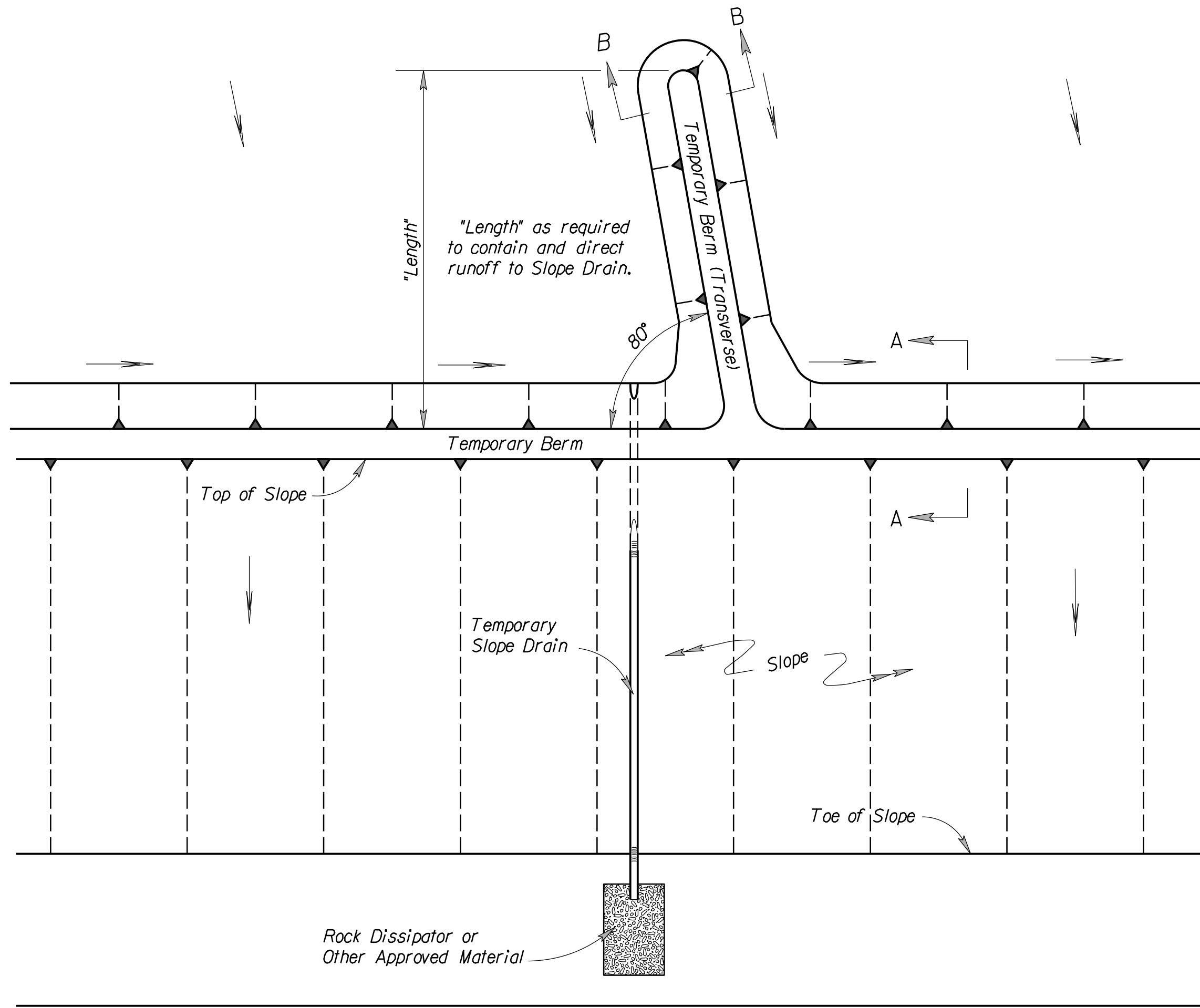
KANSAS DEPARTMENT OF TRANSPORTATION

TEMPORARY EROSION AND POLLUTION CONTROL

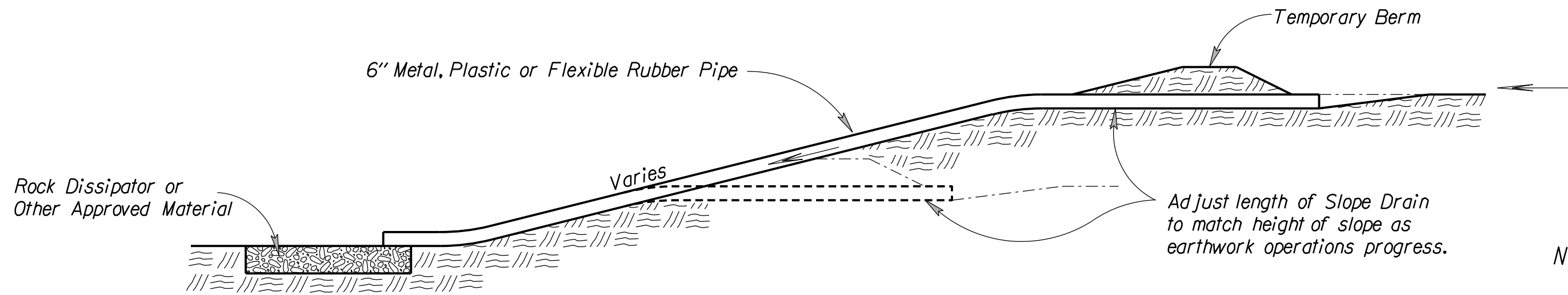
LA852A

FHWA APPROVAL		1/26/2018	APP'D	Scott H. Shields
DESIGNED	MRD	DETAILED	MRD	QUANTITIES
DESIGN CK.	SHS	DETAIL CK.	SHS	QUAN. CK.

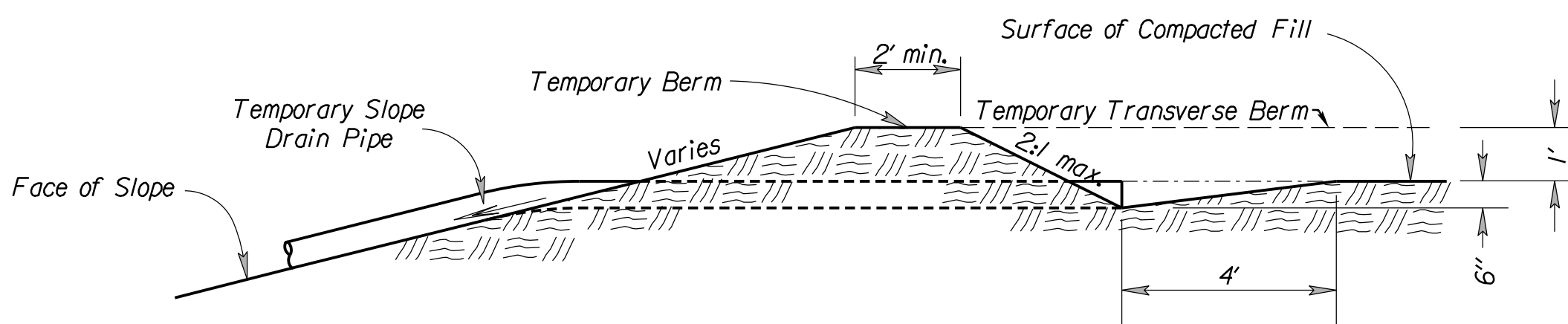
20-1458M	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
	KANSAS	43 C-5078-01	2022	24	44



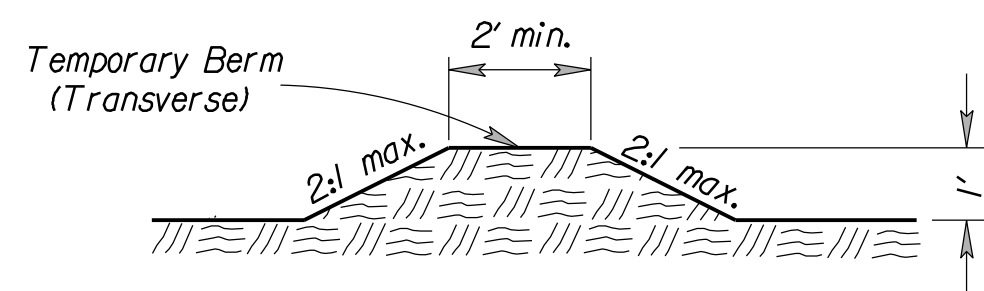
TYPICAL PLAN VIEW OF
TEMPORARY BERM AND
TEMPORARY SLOPE DRAIN
NO SCALE



TYPICAL PROFILE OF TEMPORARY SLOPE DRAIN
NO SCALE

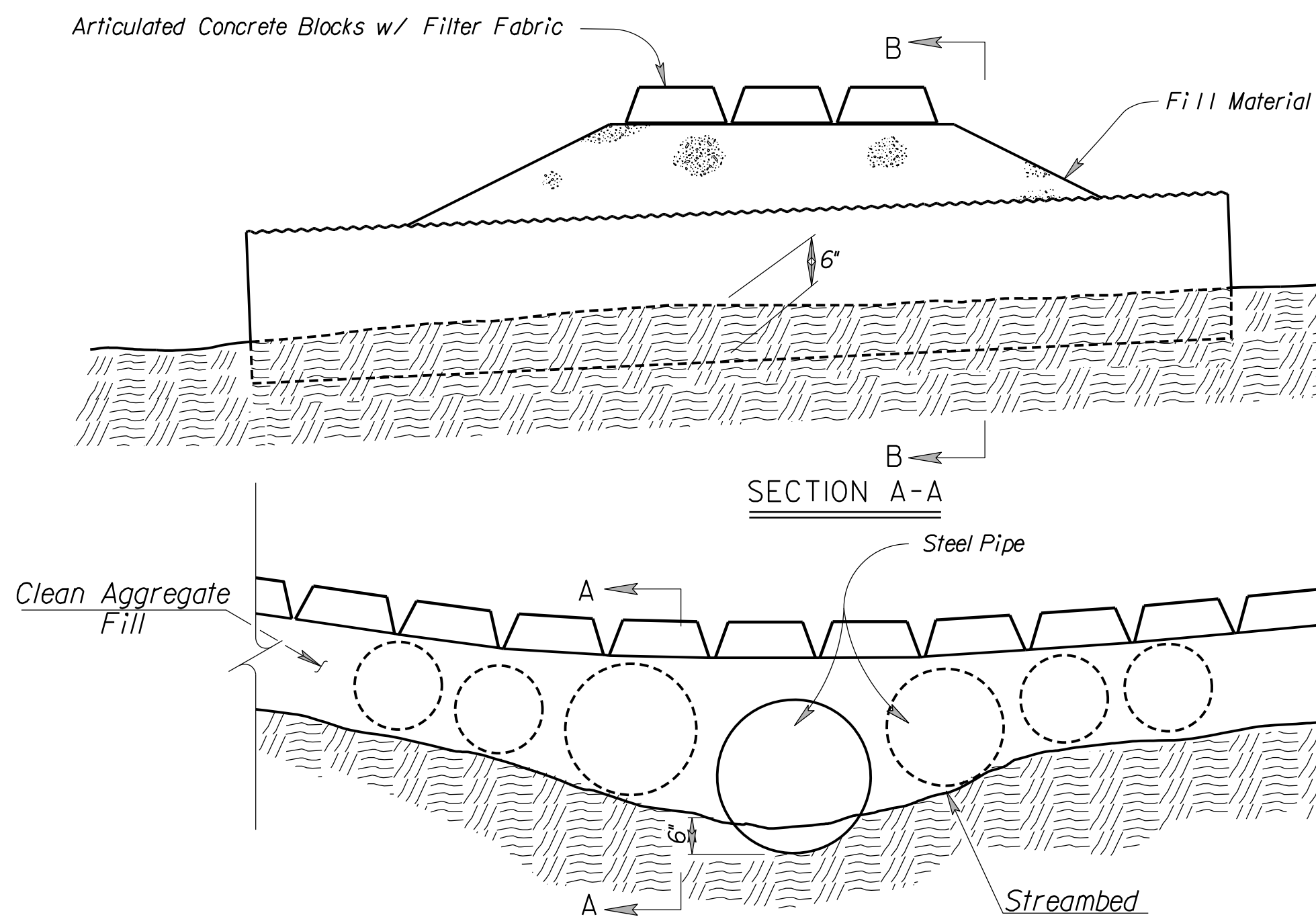


SECTION A-A
NO SCALE



SECTION B-B
NO SCALE

TYPICAL PROFILE OF TEMPORARY BERM
NO SCALE

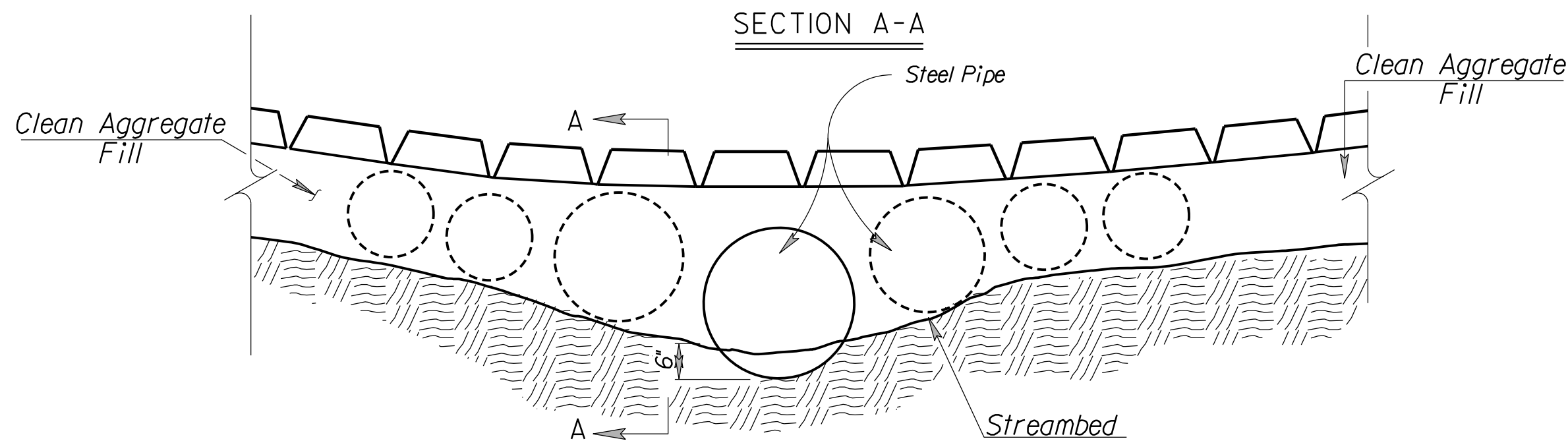


Pipe size may vary

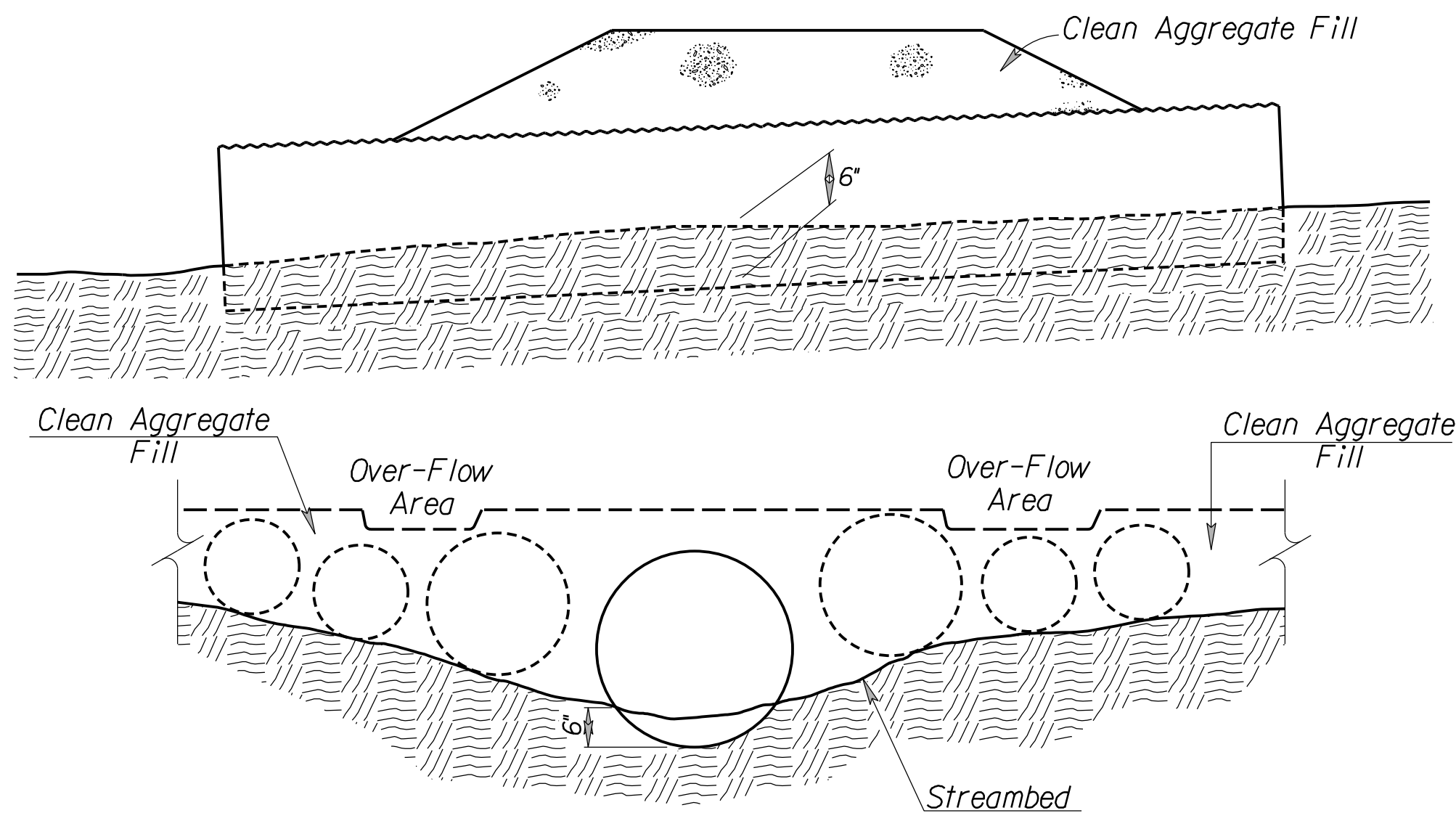
Place 1 pipe buried 6" into stream bottom, in the lowest point of the channel to allow the passage of aquatic organisms, with additional pipes placed along the remainder of the stream channel bottom such that ordinary high water (OHW) flows designated in the Contract Documents shall flow through the pipes without overtopping the crossing.

See KDOT Specifications for more information

SECTION A-A



TEMPORARY STREAM CROSSING (ARTICULATED CONCRETE BLOCKS)
NO SCALE



Pipe size may vary

Place 1 pipe buried 6" into stream bottom, in the lowest point of the channel to allow the passage of aquatic organisms, with additional pipes placed along the remainder of the stream channel bottom such that ordinary high water (OHW) flows designated in the Contract Documents shall flow through the pipes without overtopping the crossing.

See KDOT Specifications for more information

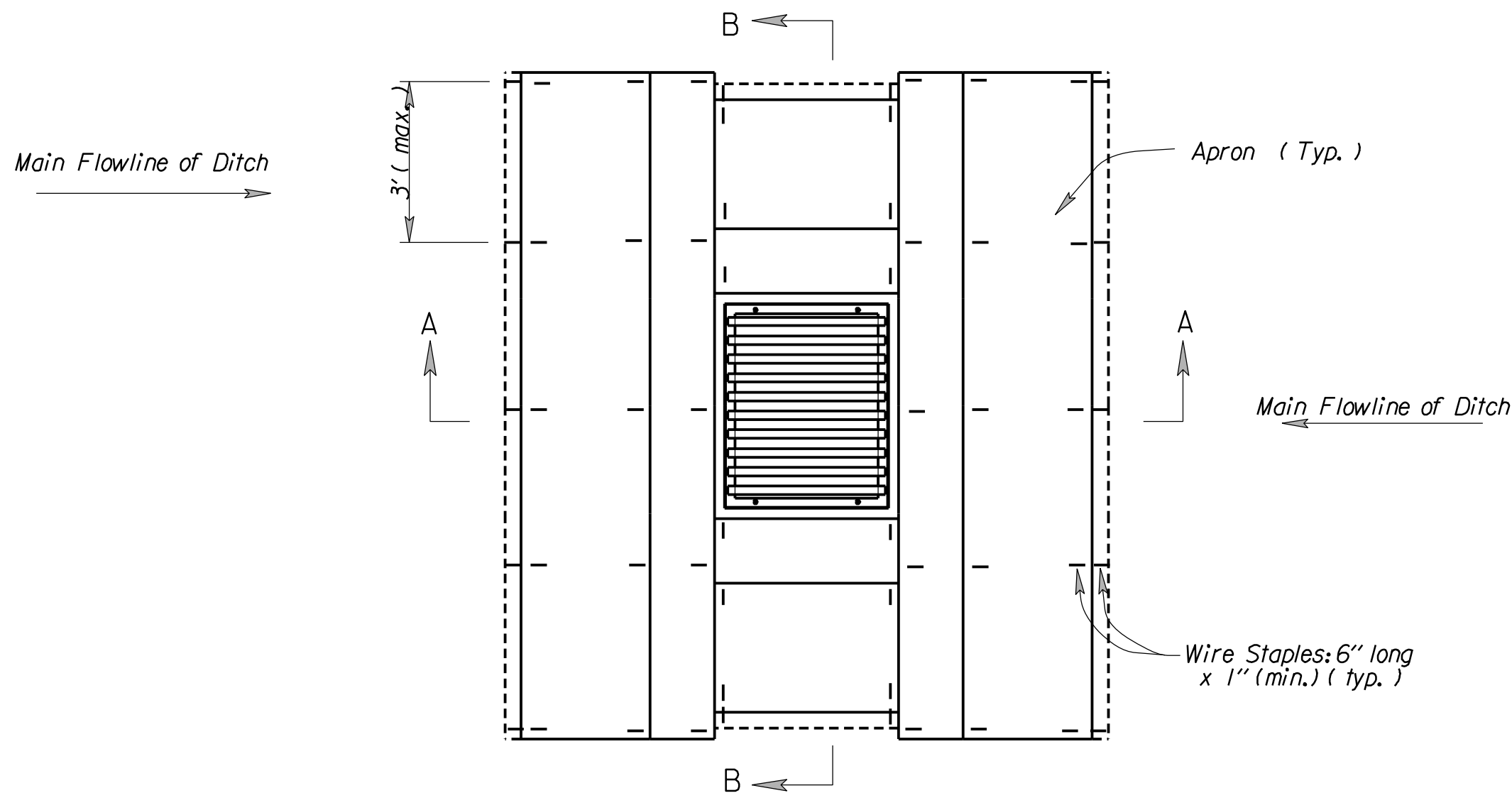
SECTION B-B

TEMPORARY STREAM CROSSING (AGGREGATE)
NO SCALE

3	6/11/13	Revised Standard	MRM	SHS
2	11/01/10	Revised Standard	MRM	SHS
1	10/15/10	Revised Standard	WCL	RDR
NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION TEMPORARY EROSION AND POLLUTION CONTROL TEMPORARY SLOPE DRAIN TEMPORARY STREAM CROSSING (AGGREGATE) TEMP. STREAM CROSS. (ARTC. CONC. BLOCKS) LA852B				
DESIGNED	MRM	DETAILED	QUANTITIES	CADD
DESIGN CK.	SHS	DETAIL CK.	QUAN. CK.	CADD CK.

Std. Base File:
Plotted By: unfiled
File: W:\M-20-20-1458M\CADD\Drawing Set\24-LA852b.dgn
Plot Date: 8/30/2021

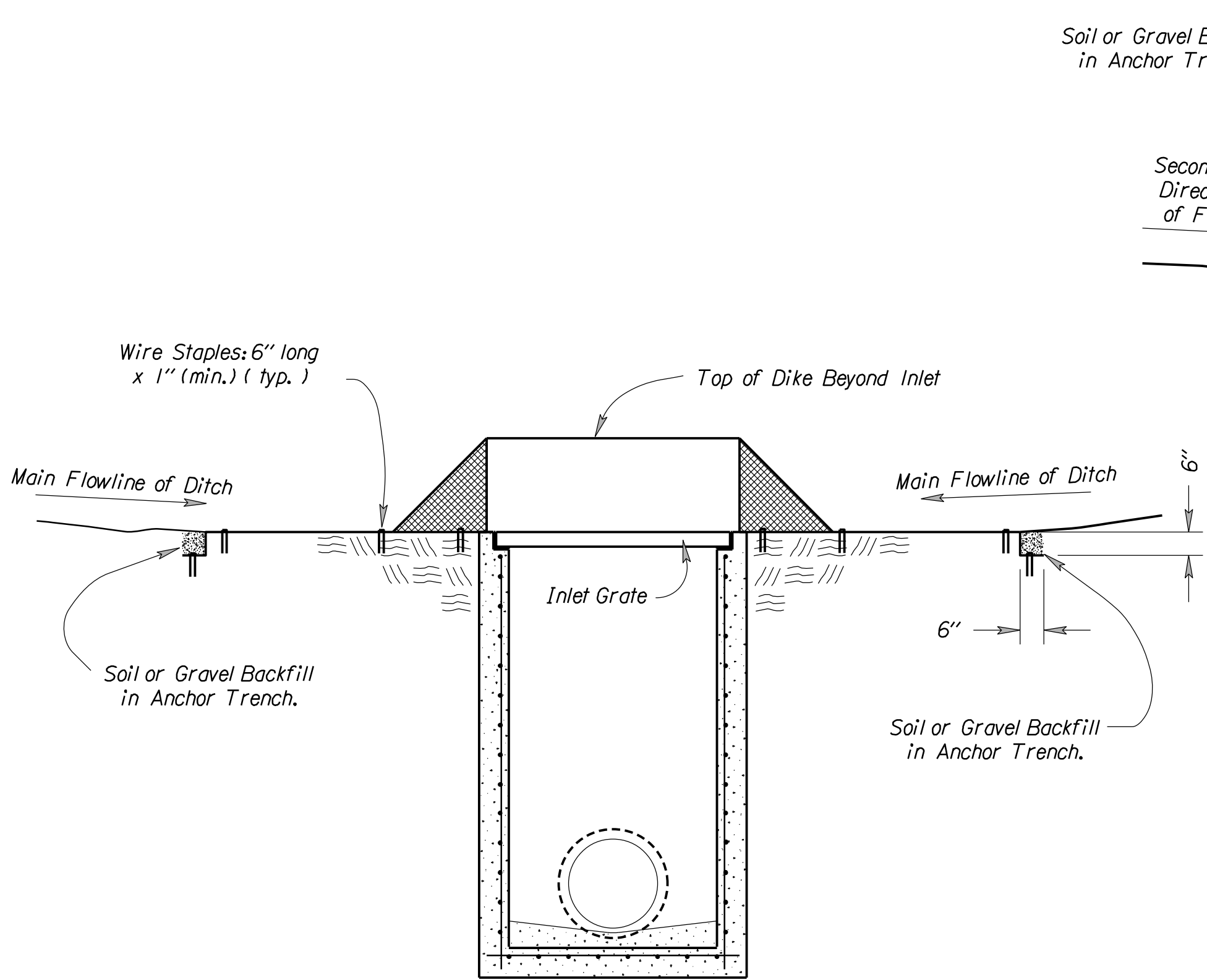
Plot Location:



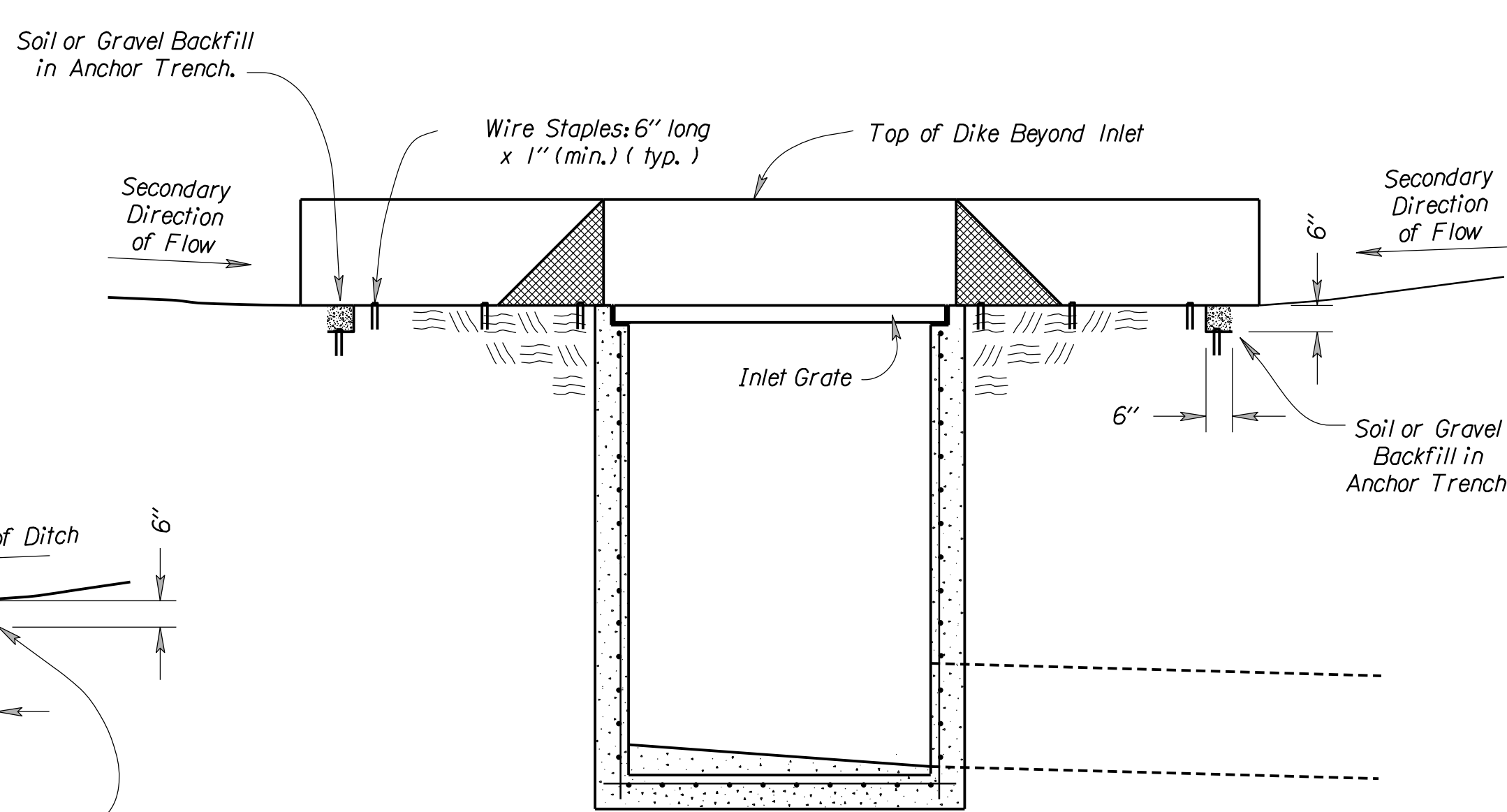
PLAN

TEMPORARY INLET SEDIMENT BARRIER
(TRIANGULAR SILT DIKE METHOD)

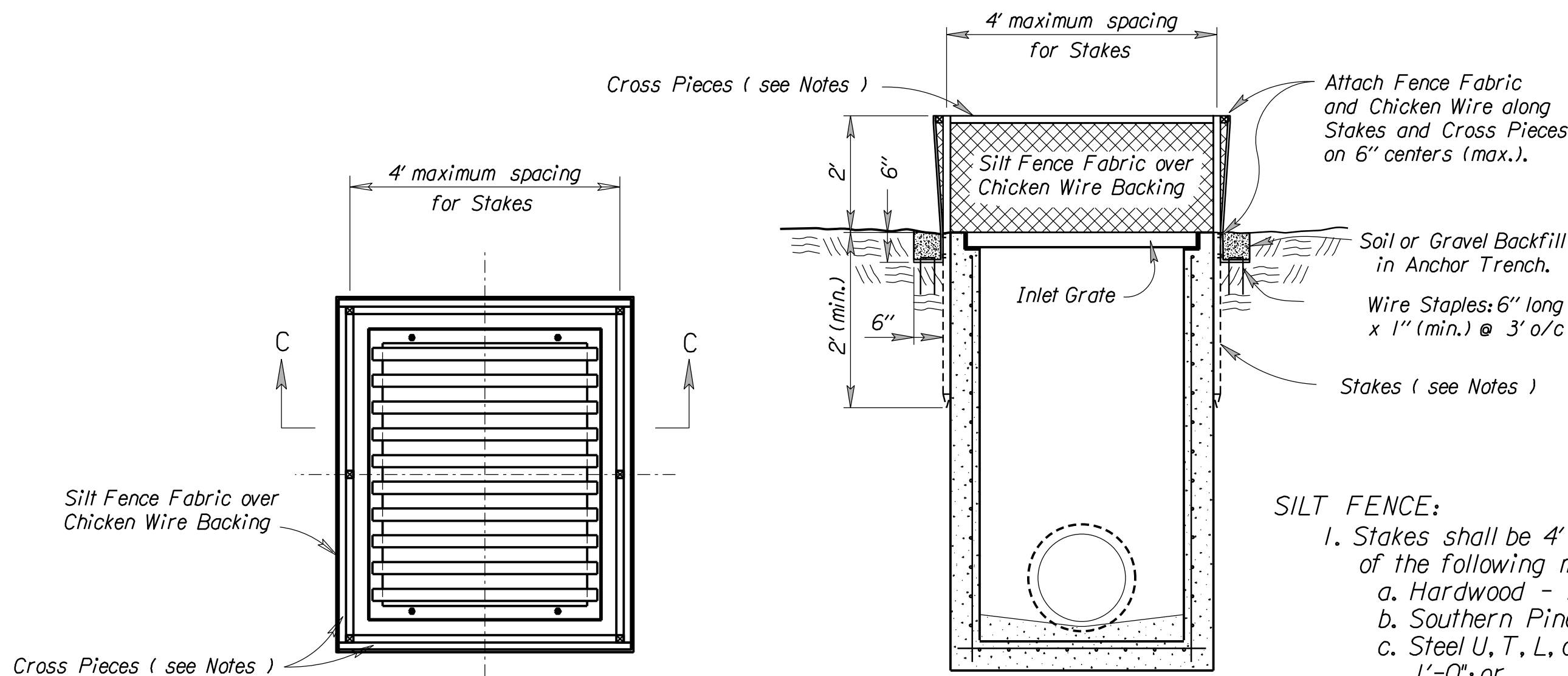
NO SCALE



SECTION A - A



SECTION B - B



PLAN

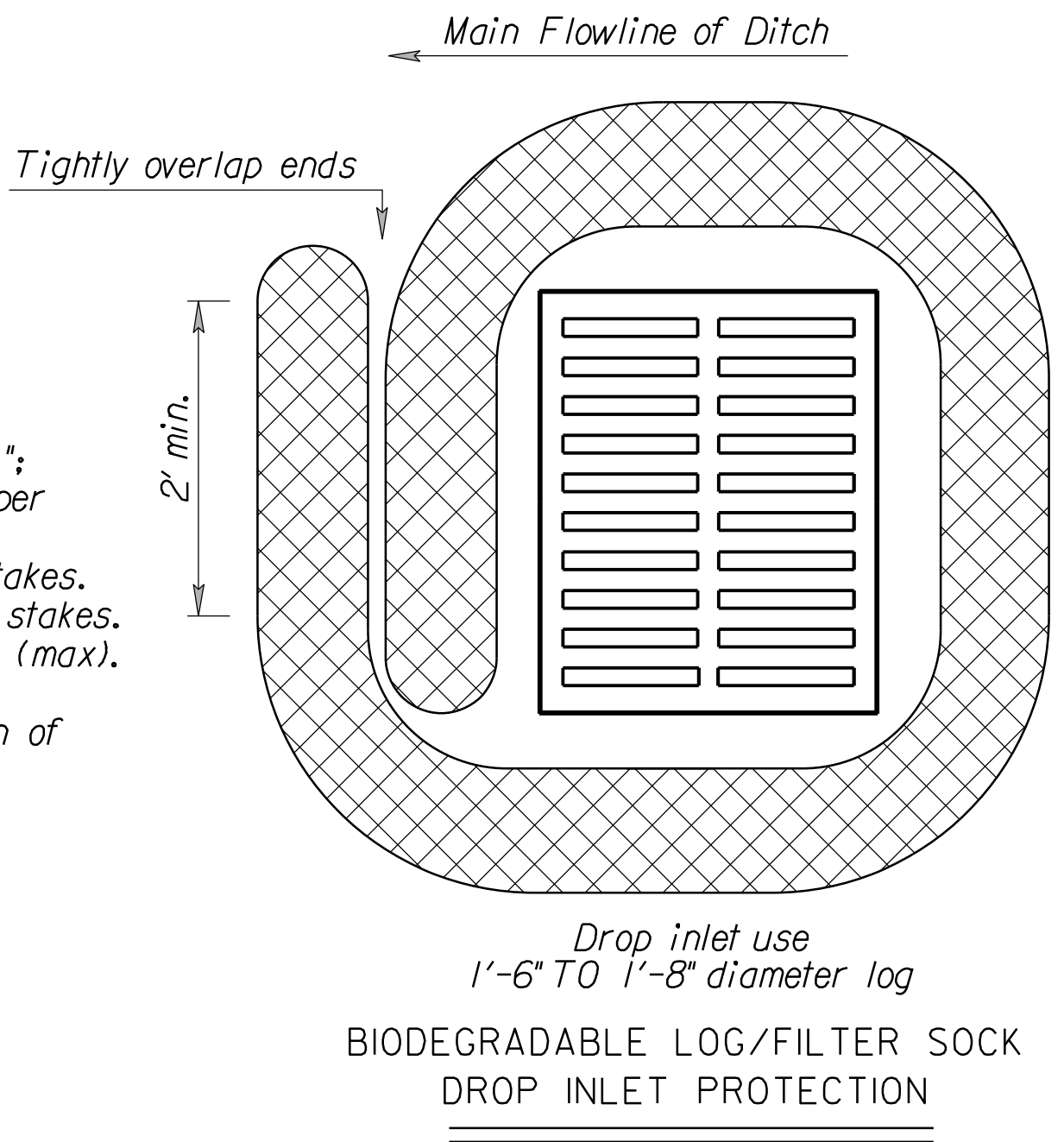
TEMPORARY INLET SEDIMENT BARRIER
(SILT FENCE METHOD)

NO SCALE

- SILT FENCE:**
- Stakes shall be 4' (min.) long and of one of the following materials:
 - Hardwood - 1 3/16" x 1 3/16";
 - Southern Pine (No. 2) - 2 5/8" x 2 5/8";
 - Steel U, T, L, or C Section - .95 lbs. per 1'-0"; or
 - Synthetic - same strength as wood stakes.
 - Cross pieces shall be of same material as stakes.
 - Attach fence fabric securely on 6" centers (max).
 - Use of high flow material is acceptable.
 - Refer to plan sheets to estimate the length of silt fence required.

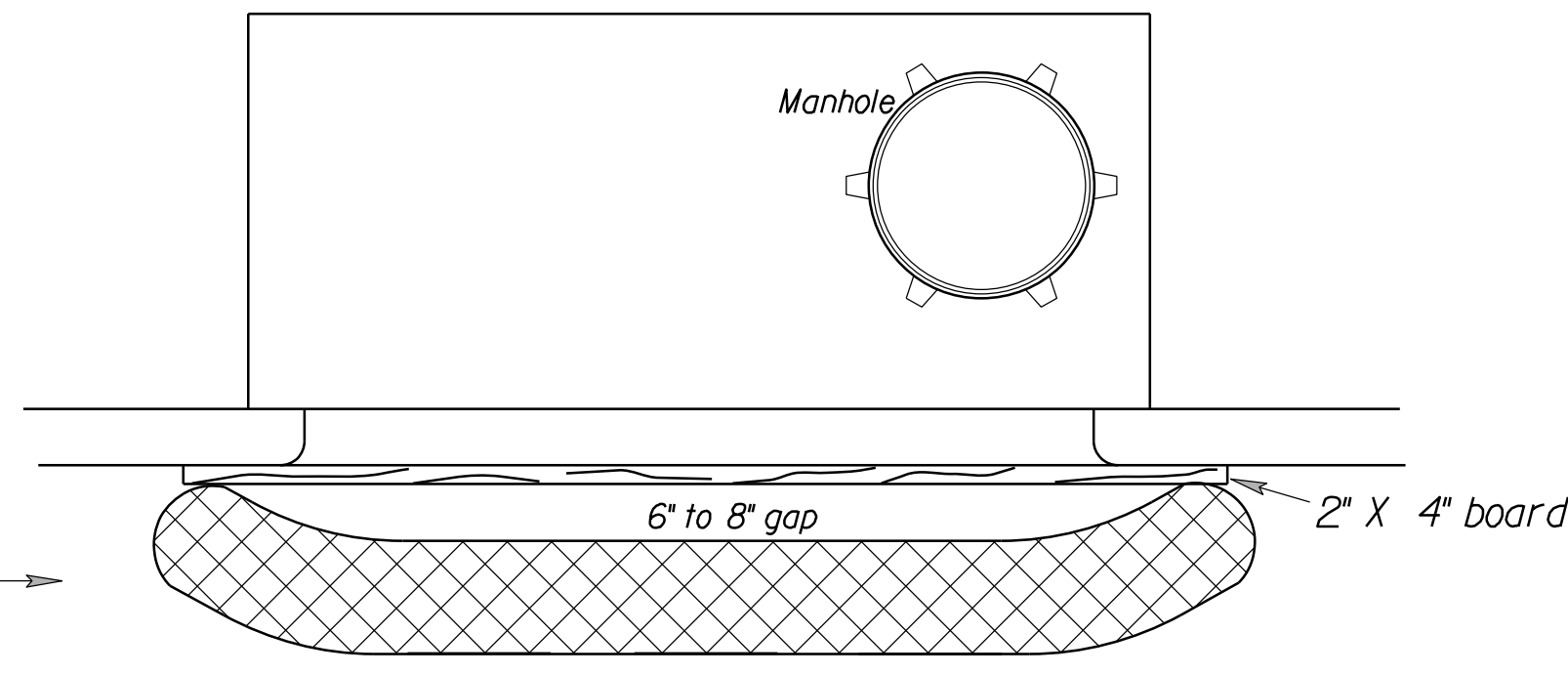
Bags = synthetic net (3mm mesh) or burlap bags

Rock = approximately 1" to 2" diameter



Drop inlet use
1'-6" TO 1'-8" diameter log

BIODEGRADABLE LOG/FILTER SOCK
DROP INLET PROTECTION



CURB INLET PROTECTION

- If multiple gravel bags are required, place them in such a way that no gaps are evident.
- Height of bags (8" minimum diameter) must not be above top of curb.
- Alternative products may be used other than gravel bags such as the "Gutter Buddy". Products must be approved by the Engineer.
- Curb inlet protection will be measured and paid for as Filter Sock.

Note: 25% of log shall be keyed into ground during installation.

Stake every 4'

Material Requirements	
Use 100% shredded mulch or other non-compost biodegradable material as fill for logs.	
No compost or fines.	
No hay or straw.	
Do not use material which prohibits water infiltration.	
Log Mesh: Use mesh with 1/4" openings or larger. Mesh must allow water infiltration but also hold fill material in place.	

3	9/26/19	Changed Direction of Main Flowline of Ditch Arrow	MRD	SHS
2	3/10/15	Revised Standard	RA	SHS
1	6/01/13	Revised Standard	MRM	SHS
NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
TEMPORARY EROSION AND POLLUTION CONTROL				
TEMP. INLET SEDIMENT BARRIER (SILT FENCE)				
TEMP. INLET SEDIMENT BARRIER (T.S.D.)				
CURB INLET PROTECTION				
DROP INLET PROTECTION				
LA852C				
FHWA APPROVAL		3/10/2015	APP'D	Scott H. Shields
DESIGNED	RA	DETAILED	RA	QUANTITIES
DESIGN CK.	SHS	DETAIL CK.	SHS	CADD CK.

Std. Base File:
Plotted By: unfiled
File: M:\M-20\20-1458M\CADD\Drawing Set\26-na852a.dgn
Plot Date: 8/30/2021

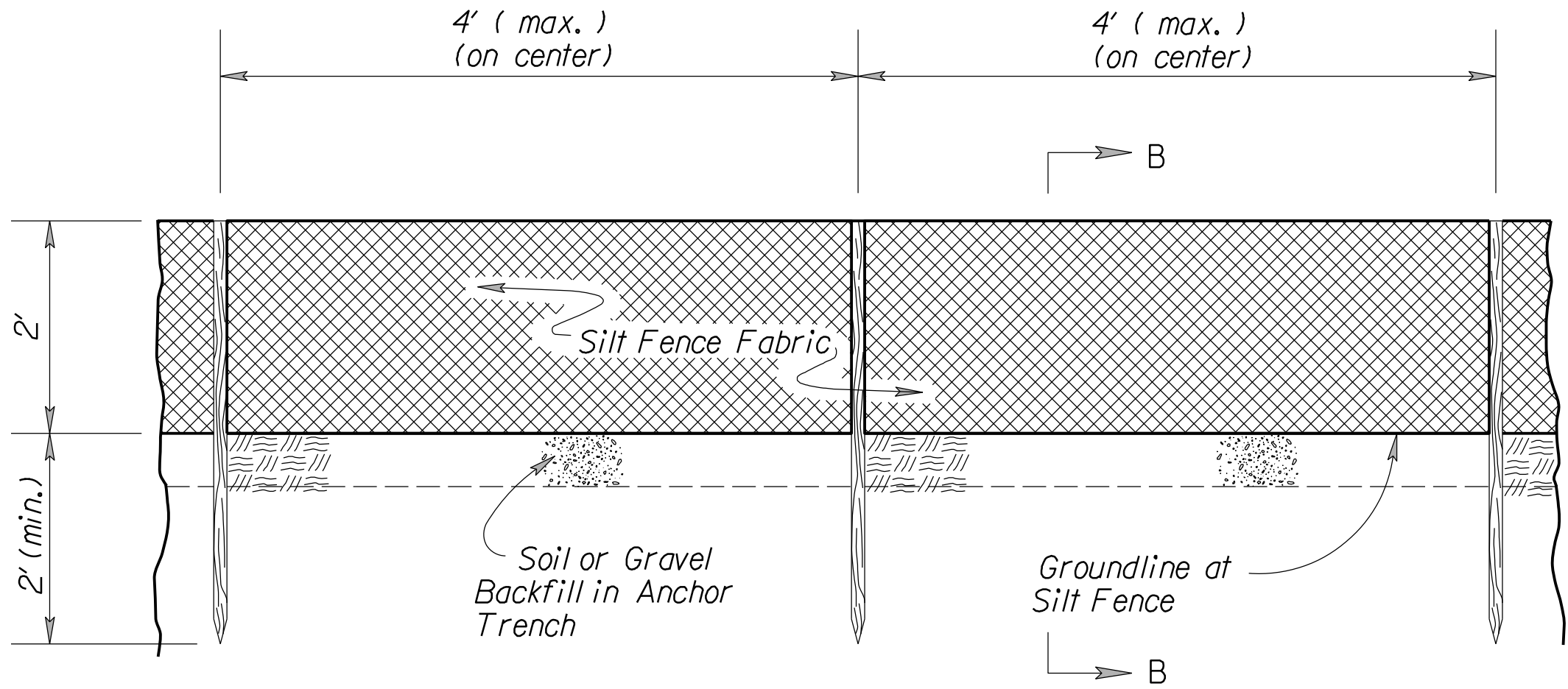
20-1458M	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
	KANSAS	43 C-5078-01	2022	26	44

INSTALLATION NOTES

- SILT FENCE:
- Stakes shall be 4' (min.) long and of one of the following materials:
 - Hardwood - 1 3/16" x 1 3/16";
 - Southern Pine (No. 2) - 2 5/8" x 2 5/8";
 - Steel U, T, L, or C Section - .95 lbs. per 1'-0"; or
 - Synthetic - same strength as wood stakes.
 - Attach fence fabric with 3 zip ties within the top 8" of the fence
Alternate attachment methods may be approved by the Engineer on a performance basis.
 - Use of high flow material is acceptable.
 - Refer to plan sheets to estimate the length of silt fence required.

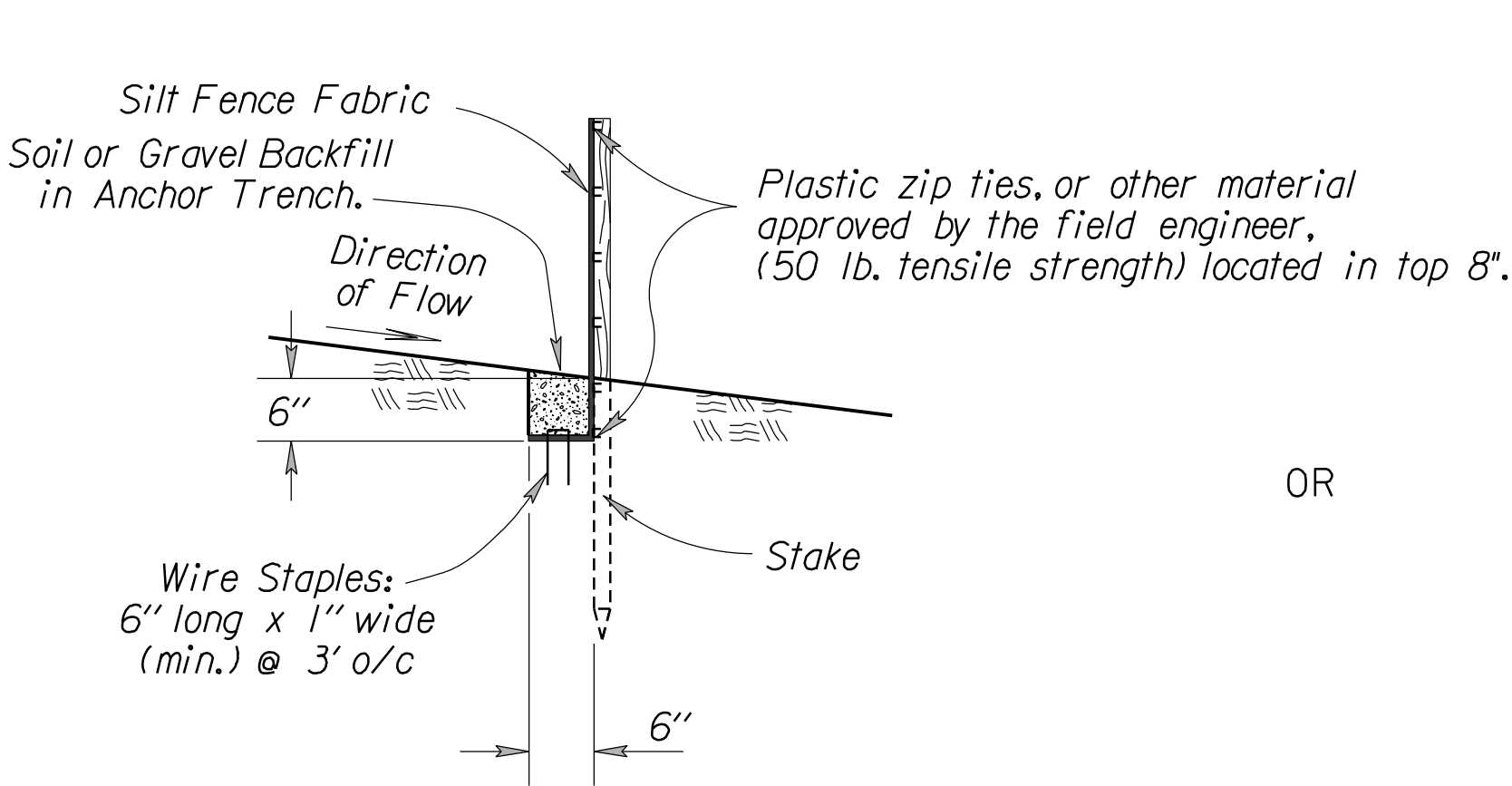
BIODEGRADABLE LOG OR FILTER SOCK

- Place biodegradable logs or filter sock tightly together minimum overlap of 18".
- Wood stakes shall be 2" x 2" (nom.).
- Refer to plan sheets to estimate length of biodegradable log and filter sock required.
- Each log or sock (except compost filter socks) should be keyed into the ground at a minimum of 25% of its height. Compost filter socks should be placed on smooth prepared ground with no gaps between the sock and soil.
- Length of stakes should be 2 times the height of the log at a minimum with minimum ground embedment equal to the height of the log / sock.



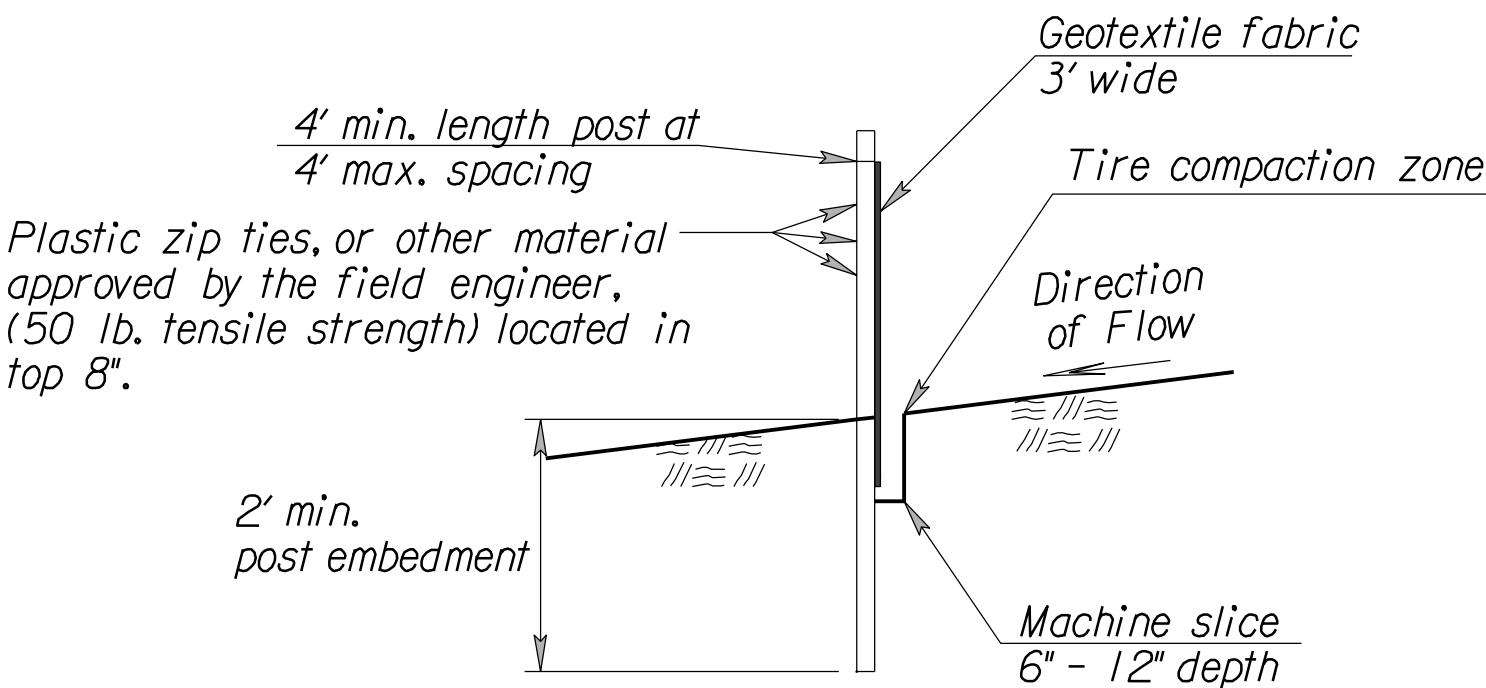
TYPICAL ELEVATION

SILT FENCE BARRIER
NO SCALE



SECTION B-B

OR



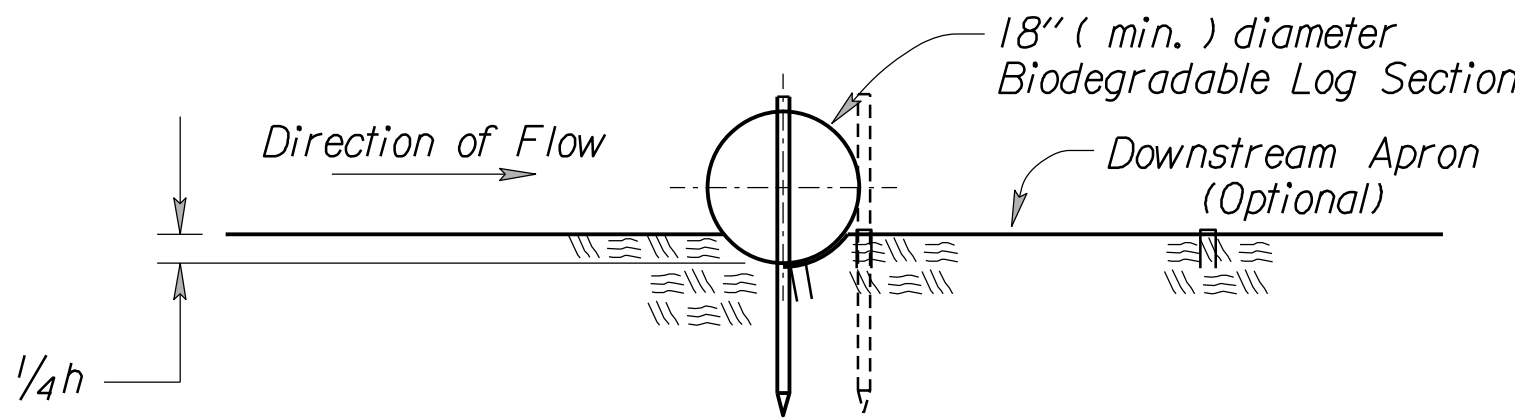
SECTION B-B

Biodegradable Log or Filter Sock Slope Interruptions

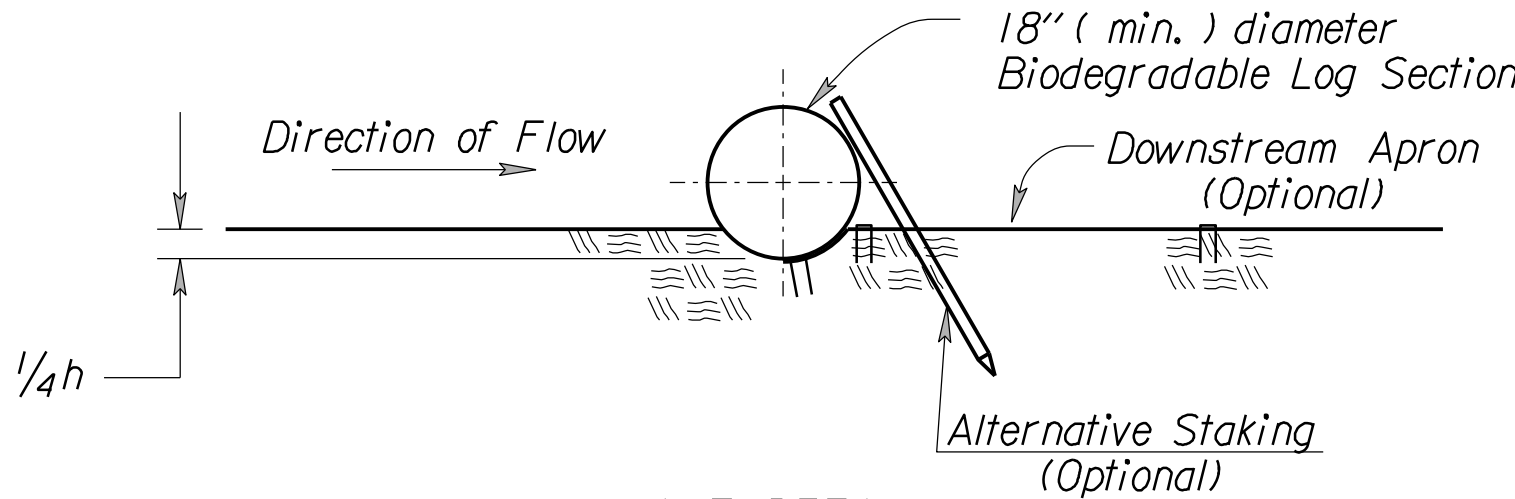
		PRODUCT		
		9" Sediment Log or 8" Filter Sock (ft)	12" Sediment Log or 12" Filter Sock (ft)	20" Sediment Log or 18" Filter Sock (ft)
Slope Gradient	≤4H:1V	40	60	80
	3H:1V	30	45	60

BIODEGRADABLE LOG MATERIAL		
	LOW FLOW	HIGH FLOW
9"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber
12"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber
18"-20"	Straw/Compost	Excelsior / Wood Chips / Coconut Fiber

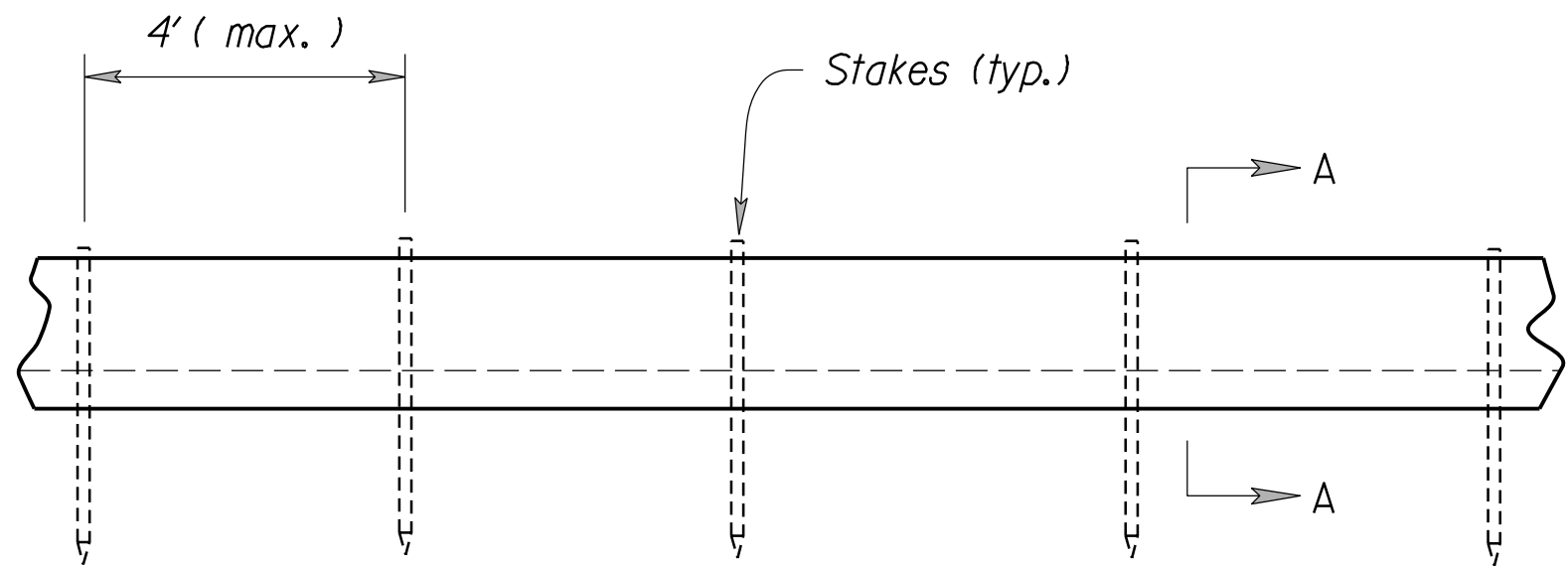
Deviations should be approved by the Field Engineer.



SECTION A - A



ALT. DETAIL
OPTIONAL



TYPICAL ELEVATION

BIODEGRADABLE LOG SLOPE INTERRUPTIONS
OR Filter Sock

GENERAL NOTES

- Slope interruptions shall be placed along contour lines, with a short section turned upgrade at each end of the barrier.
- The maximum length of the slope interruptions shall not exceed 250 feet, and the barrier ends need to be staggered.
- Interruptions damaged by Contractor's negligence, including improper maintenance or lack of maintenance, shall be repaired immediately by Contractor at no additional cost to KDOT.
- Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

3	6/28/16	Revised Standard	RA	SHS
2	3/01/15	Revised Standard	RA	SHS
1	6/01/13	Revised Standard	MRM	SHS
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION				
TEMPORARY EROSION AND POLLUTION CONTROL				
SLOPE INTERRUPTIONS				
BIODEGRADABLE LOG / SILT FENCE				
LA852D				
FHWA APPROVAL	9/14/2016	APP'D	Scott H. Shields	
DESIGNED	SHS	DETAILED	RA	QUANTITIES
DESIGN CK.	SHS	DETAIL CK.	RA	QUAN. CK.

Std. Base File:

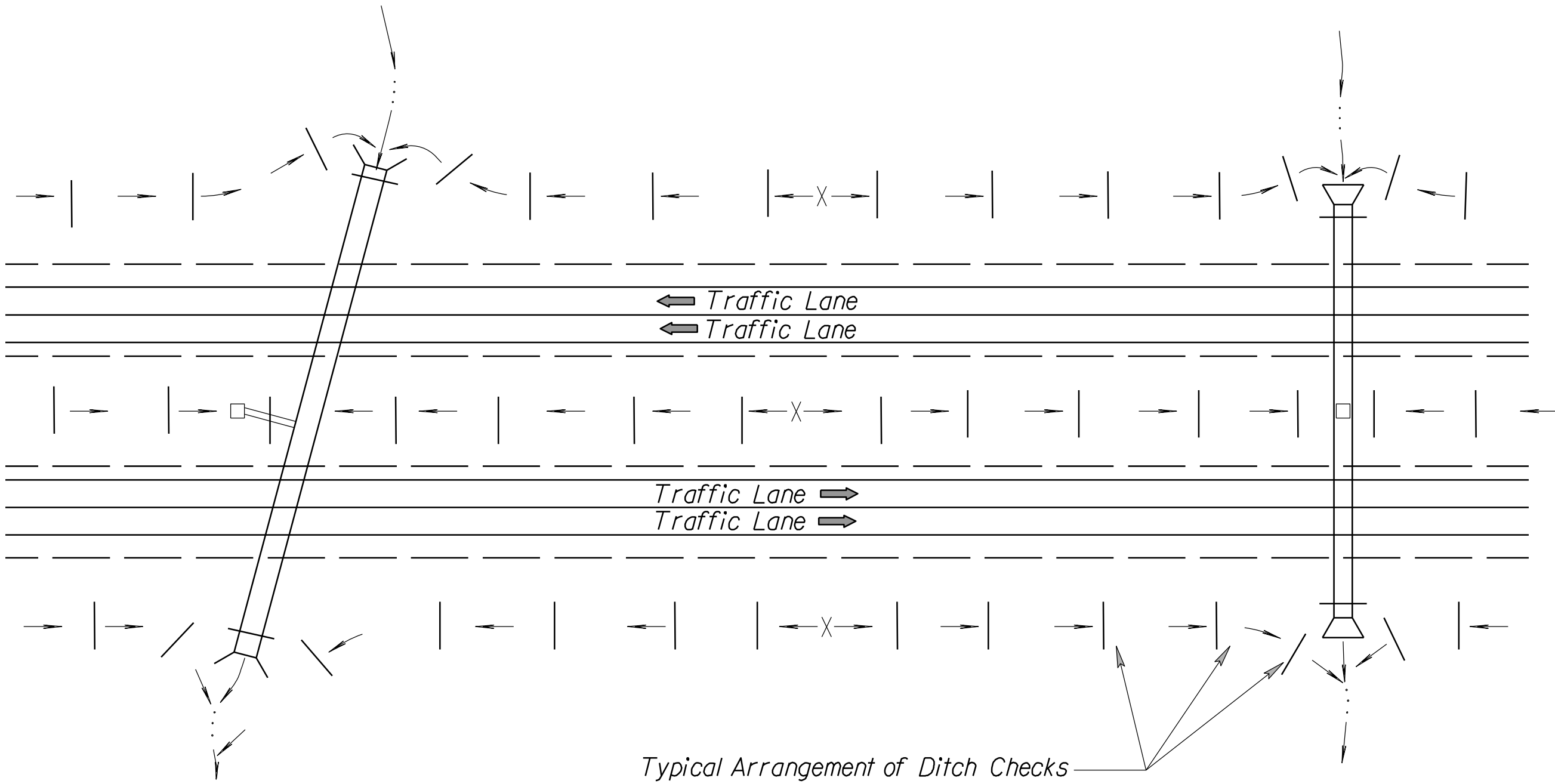
Plotted By: *united*

File: *W-20-20-1458MACADDrawing Set\27-la852e.dgn*

Plot Date: *8/30/2021*

Plot Location:

20-1458M	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
	KANSAS	43 C-5078-01	2022	27	44



TYPICAL DITCH CHECK LAYOUT PLAN
NO SCALE

20" BIOLOG CHECK SPACING	
DITCH Q SLOPE (%)	SPACING INTERVAL (FEET)
1.0	125
2.0	60
3.0	40
4.0	30
5.0	25
NOTE: Use this spacing for all except Rock Ditch Checks.	

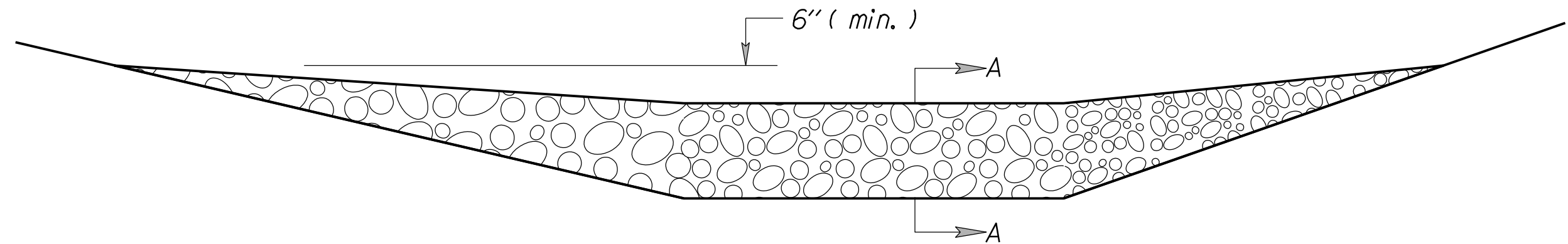
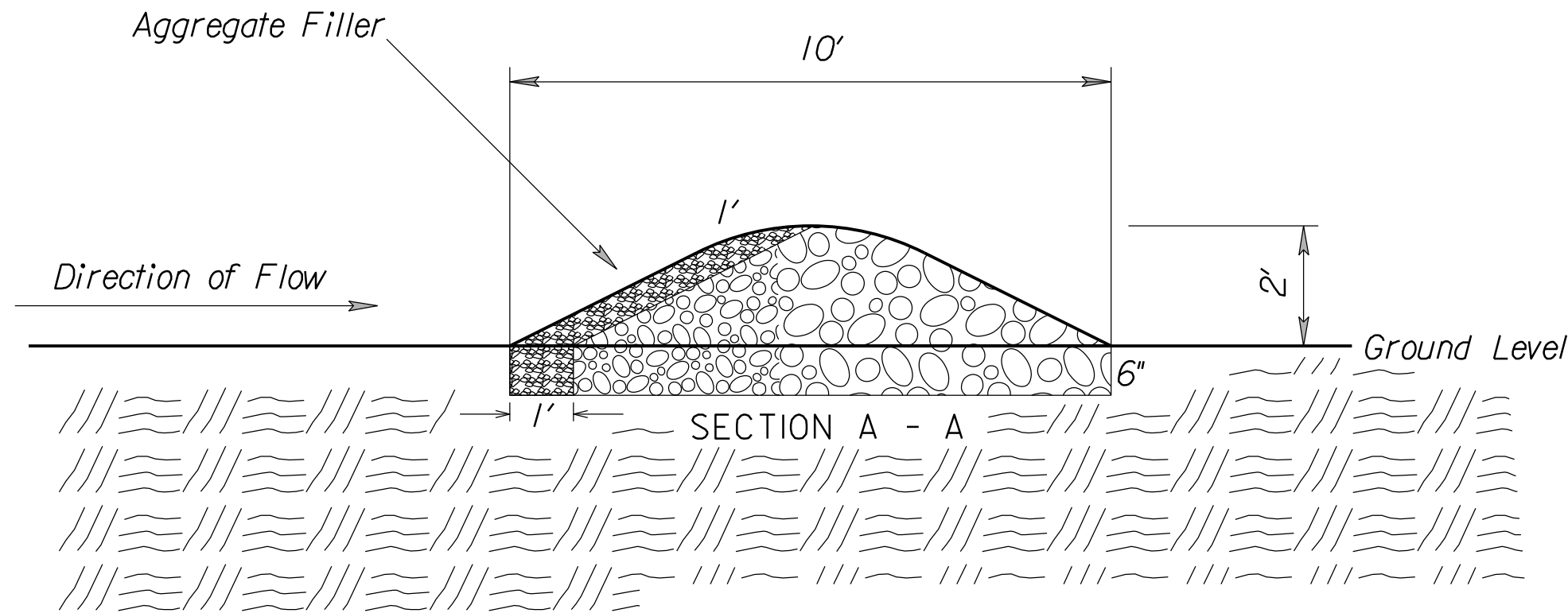
18" FILTER SOCK CHECK SPACING	
DITCH Q SLOPE (%)	SPACING INTERVAL (FEET)
1.0	110
2.0	55
3.0	35
4.0	25
5.0	20
NOTE: Use this spacing for all except Rock Ditch Checks.	

GENERAL NOTES

- The choice of ditch check methods is at the option of the Contractor.
- Use only rock checks in situations where the ditch slope is 6 percent or greater.
- Ditch checks damaged by Contractor's negligence, including improper maintenance or lack of maintenance, shall be repaired by Contractor at no extra cost to KDOT.

3	8/10/16	Revised Standard	RAA	SHS
2	6/28/16	Revised Standard	RAA	SHS
1	6/01/13	Revised Standard	MRM	SHS
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION				
TEMPORARY EROSION AND POLLUTION CONTROL				
DITCH CHECKS				
LA852E				
FHWA APPROVAL		9/14/2016	APP'D	Scott H. Shields
DESIGNED	SHS	DETAILED	RAA	QUANTITIES
DESIGN CK.	SHS	DETAIL CK.	SHS	QUAN. CK.
			CADD	RAA
			CADD CK.	SHS



TYPICAL ELEVATION

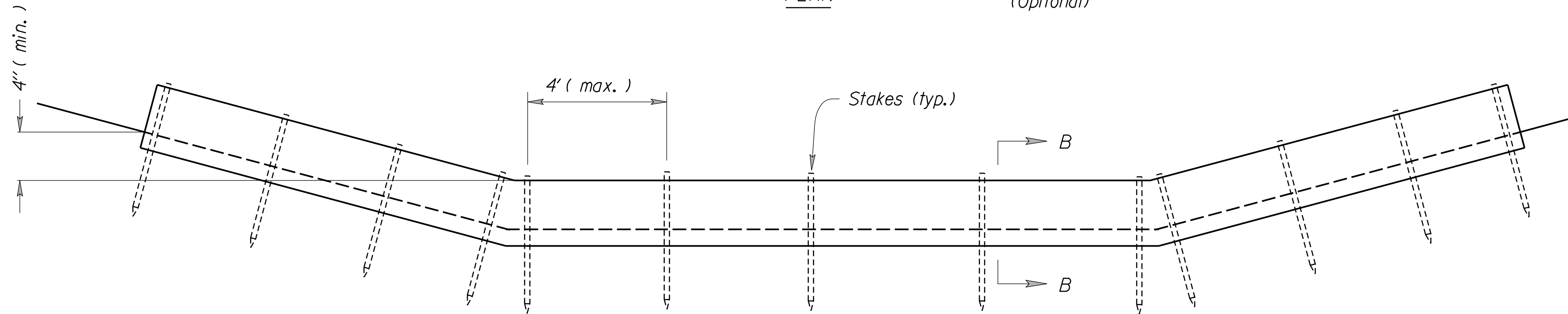
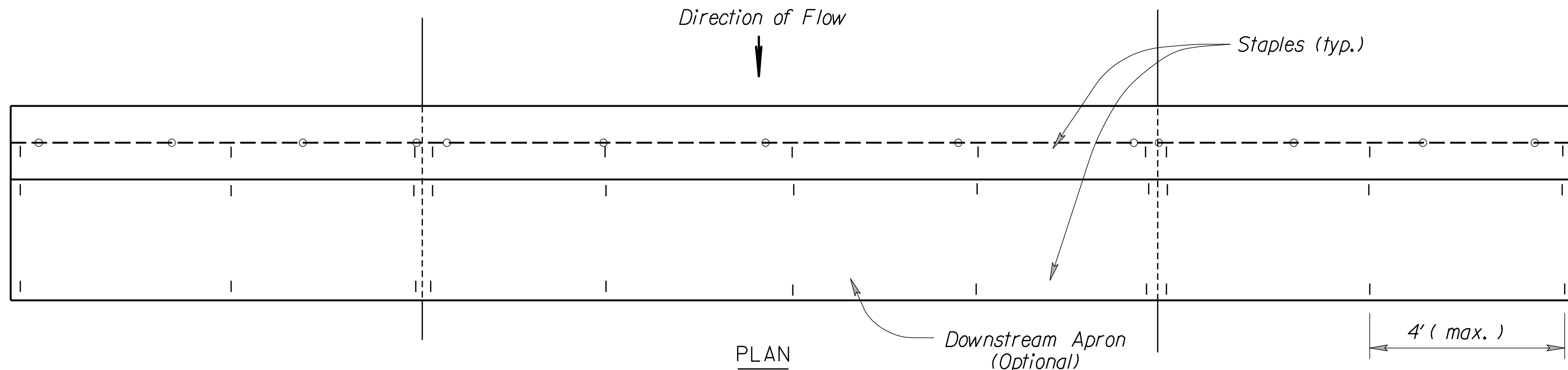
ROCK DITCH CHECK

NO SCALE

TEMPORARY ROCK DITCH CHECK SPACING	
DITCH Q SLOPE (%)	SPACING INTERVAL (FEET)
5.0	60
6.0	50
7.0	43
8.0	36
9.0	33
10.0	29
NOTE: Use this spacing for Rock Ditch Checks only.	

ROCK DITCH CHECK NOTES

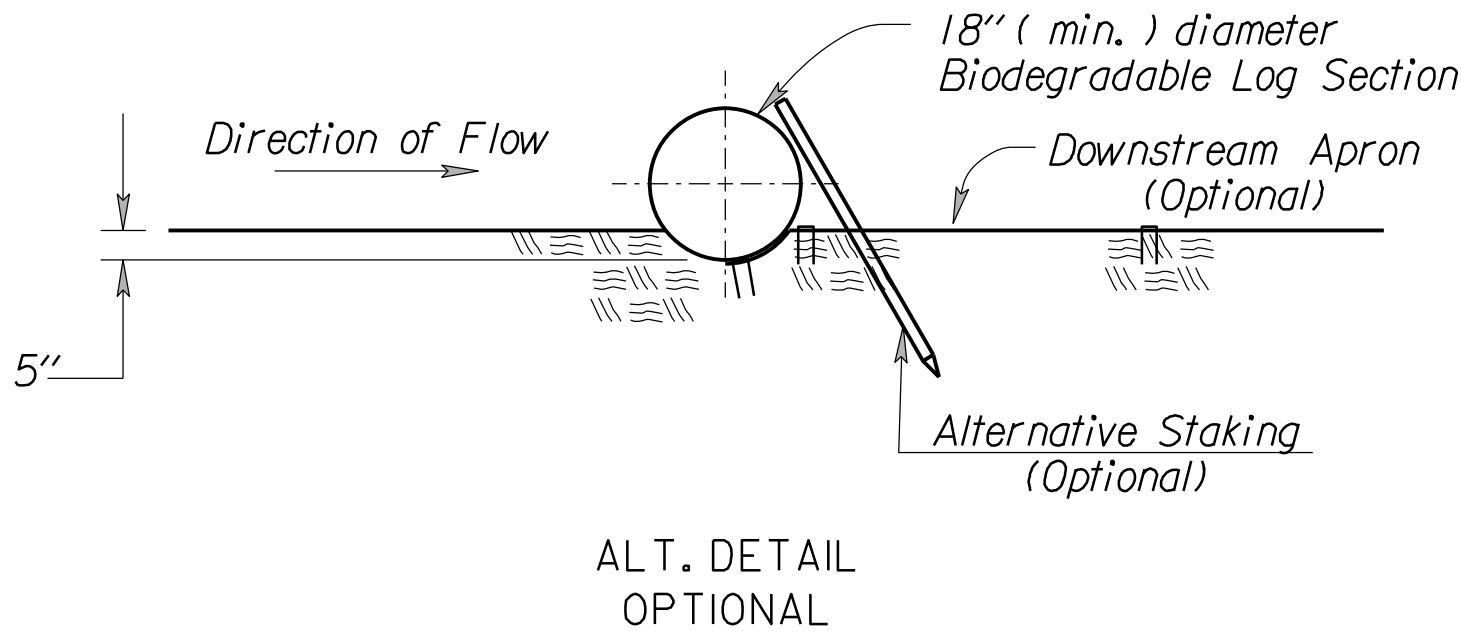
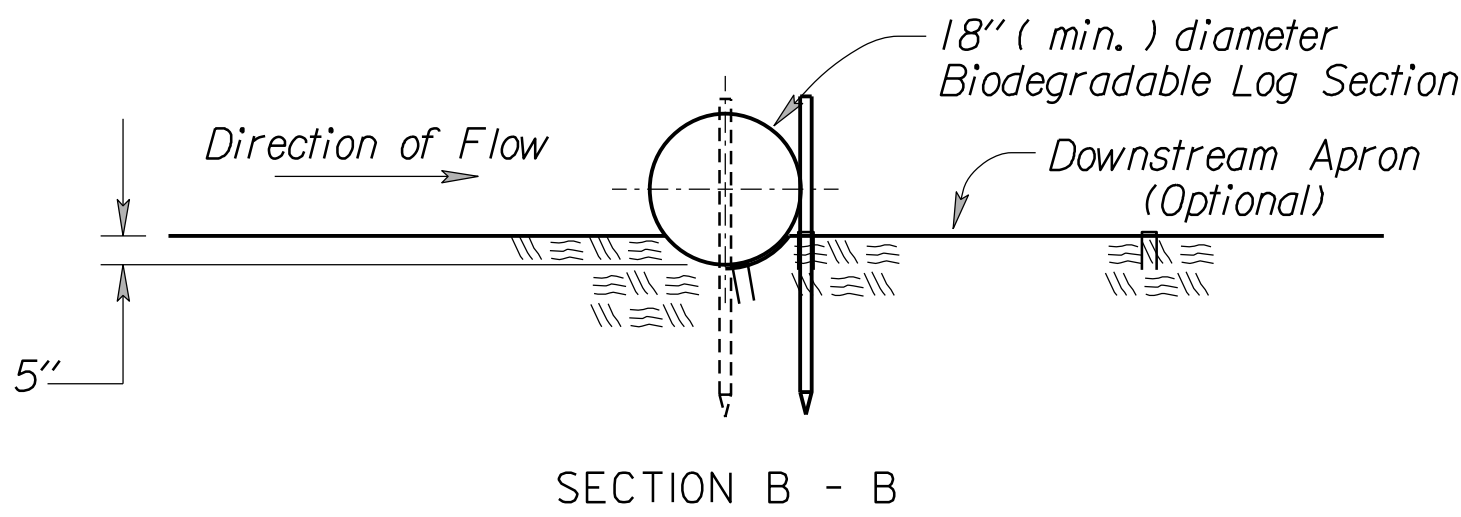
1. Rock shall be clean aggregate, D50-6" and aggregate filler.
2. Place rock in such manner that water will flow over, not around ditch check.
3. Do not use rock ditch checks in clear zone.
4. Excavation: The ditch area shall be reshaped to fill any eroded areas. Prior to placement of the rock, the ditch shall be excavated to the dimensions of the Rock Ditch Check and to a minimum depth of 6" (150mm). After placement of the rock, backfill and compact any over-excavated soil to ditch grade. This work shall be subsidiary to the bid item Temporary Ditch Check (Rock).
5. Aggregate excavated on site may be used as an alternate to the 6" rock, if approved by the Engineer.
6. The Engineer may approve the use of larger aggregates for the downstream portion of the check when conditions warrant their use.
7. When the use of larger rock is approved, D50-6" rock will be placed between the larger aggregate and the aggregate filler.
8. Aggregate filler will be placed on the upstream face of the ditch check. Aggregate filler will comply with Filter Course Type I, Division 1114.



TYPICAL ELEVATION

BIODEGRADABLE LOG DITCH CHECK

OR Filter Sock Ditch Check
NO SCALE



BIODEGRADABLE LOG DITCH CHECK NOTES

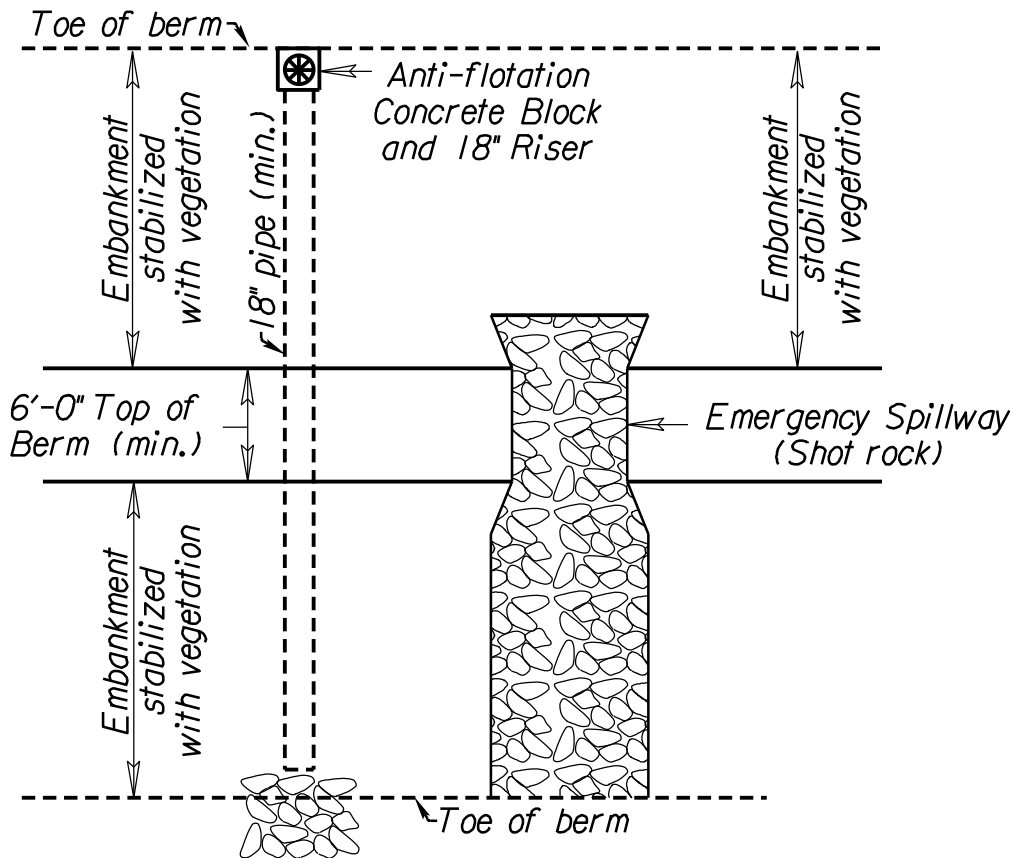
1. Use as many biodegradable log sections as necessary to ensure water does not flow around end of ditch check.
2. Overlap sections a minimum of 18".
3. Stakes shall be wood or steel according to Section 2114 of the Standard Specifications. Length of stakes shall be a minimum of 2 x the diameter of the log.
4. Use Erosion Control (Class I) (Type C) as the downstream apron when required.
5. A downstream apron is required when directed by the Engineer. Apron material will be paid at the contract unit price.
6. Each log or sock (except compost filter socks) should be keyed into the ground at a minimum of 25% of its height. Compost filter socks should be placed on smooth prepared ground with no gaps between the sock and soil.

NO.	DATE	REVISIONS	BY	APP'D
3	11/19/20	Revised Standard	MRD	ML
2	8/10/16	Revised Standard	RAA	SHS
1	10/21/15	Revised Standard	RAA	SHS

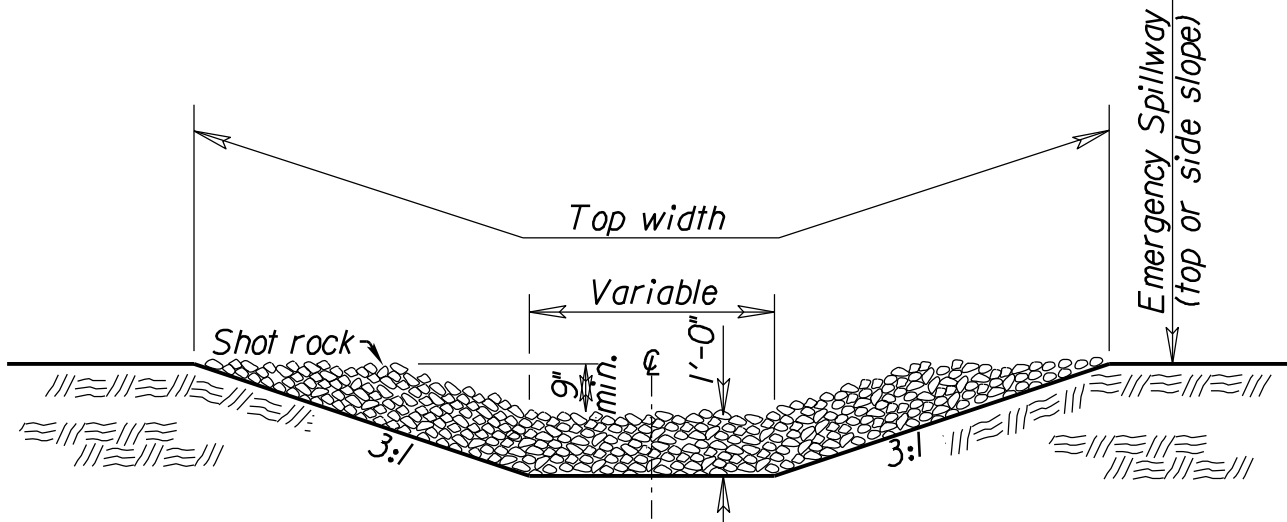
KANSAS DEPARTMENT OF TRANSPORTATION			
TEMPORARY EROSION AND POLLUTION CONTROL			
ROCK DITCH CHECKS			
BIODEGRADABLE LOG DITCH CHECKS			
LA852G			
FHWA APPROVAL		11/19/2020 APP'D	
DESIGNED	ML	DETAILED	DK
DESIGN CK.	ML	DETAIL CK.	ML
QUANTITIES		CADD	
QUAN.CK.		CADD CK.	
BY		APP'D	
Mervin Lore		RAA	

Std. Base File:
 Plotted By: unutilled
 File: M:\M-20\20-1458\IN\CADD\Drawing Set\29-14852h.dgn
 Plot Location:
 Plot Date: 8/30/2021

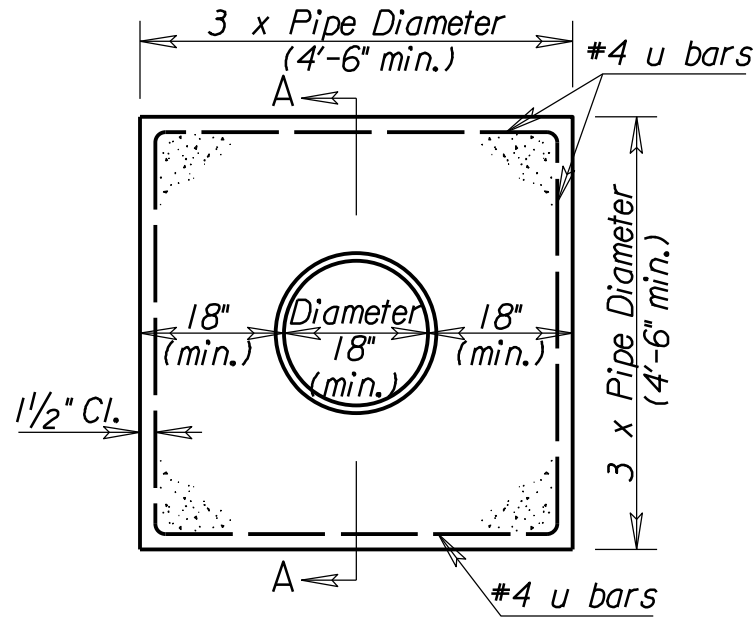
20-1458M	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
	KANSAS	43 C-5078-01	2022	29	44



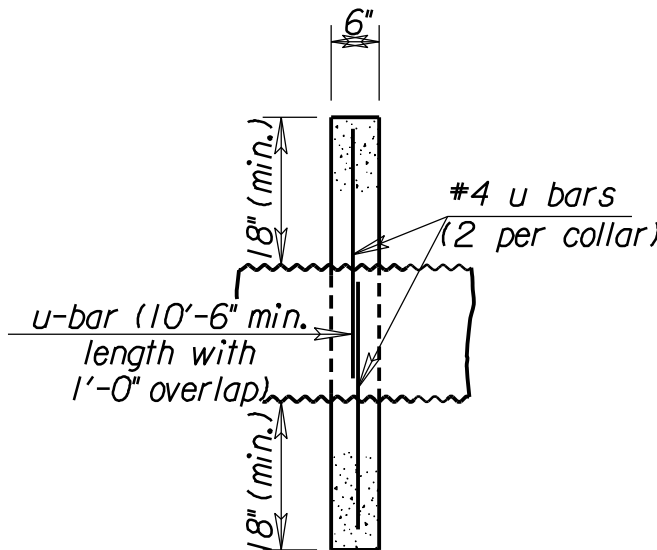
SEDIMENT STORAGE BASIN (PLAN)



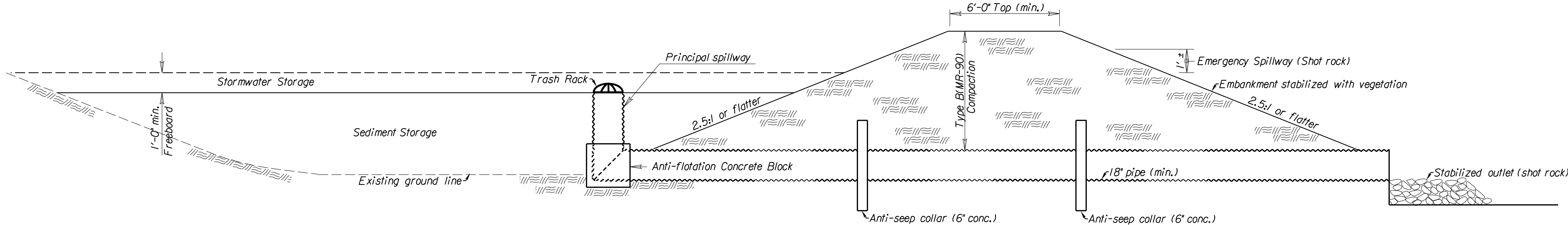
CROSS SECTION (EMERGENCY SPILLWAY)



CONCRETE ANTI-SEEP COLLAR



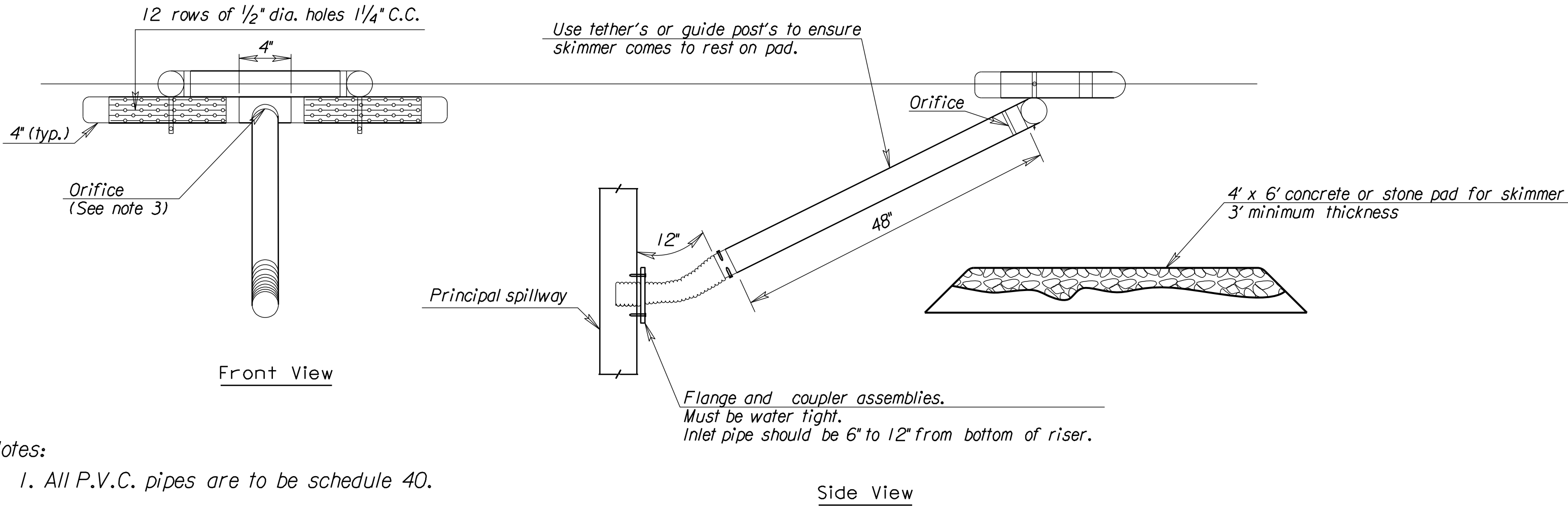
SECTION A-A



SEDIMENT STORAGE BASIN (ELEVATION)

NOTES:

- 1) Temporary Sediment Basins shall be constructed at locations as directed by the Engineer or as approved in the SWPPP Schedule. All work and materials necessary, including but not limited to, the fill material, compaction, drainage pipes, aggregates and all other incidentals necessary to construct the basin, shall be paid as "Temporary Sediment Basin".
- 2) Lengths and top dimensions shall be determined in the field by the Engineer.
- 3) Skimmer dewatering device required and must be used regardless the size of the drainage area.



- Notes:
1. All P.V.C. pipes are to be schedule 40.
 2. HDPE flexible drain pipes is to be attached to the pond outlet structure with water-tight connections.
 3. The orifice shall be sized of to provide drawdown time to 2 to 5 days and approved by the engineer.
 4. Other skimmer designs maybe used that dewater from the surface at a controlled rate. The design must be approved by the engineer.

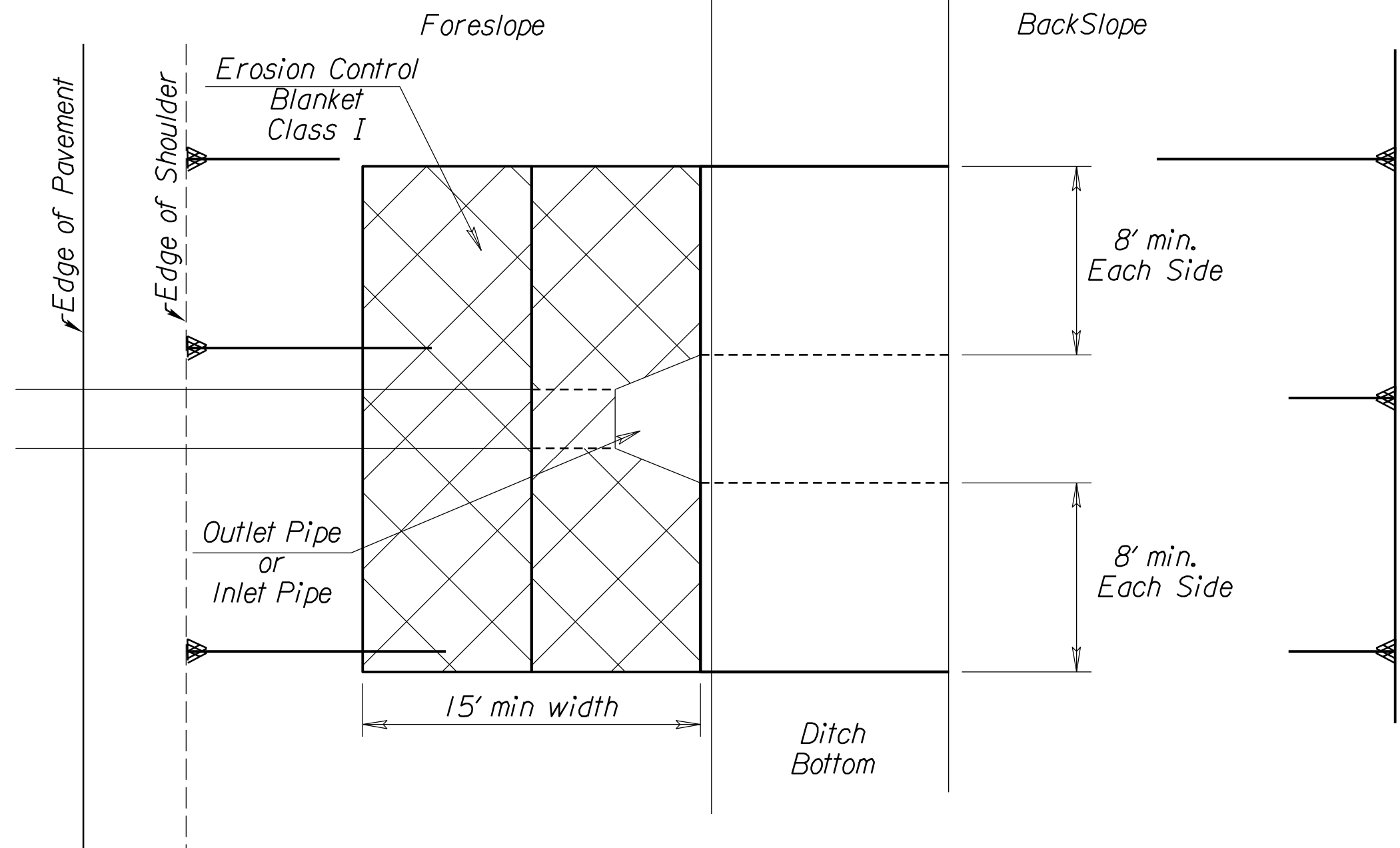
SKIMMER DEWATERING DEVICE

SEDIMENT STORAGE BASIN LOCATIONS		
STATION TO STATION	SIDE	REQUIRED STORAGE CAPACITY

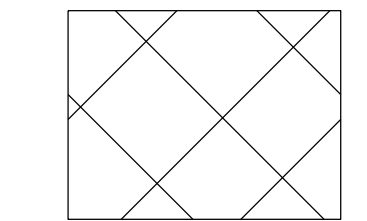
3					
2	9/3/13	Added Skimmer Dewatering Device	MRW	SHS	
1	7/17/13	Revised Standard	MRW	SHS	
NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
TEMPORARY EROSION AND POLLUTION CONTROL					
SEDIMENT STORAGE BASIN					
LA852H					
FHWA APPROVAL	09/24/2013	APP'D	Scott H. Shields		
DESIGNED	BB	DETAILED	BB	QUANTITIES	CADD
DESIGN CK.	SHS	DETAIL CK.	SHS	QUAN. CK.	CADD CK.

Std. Base File: la855.dgn
Plotted By: unfiled
File: M:\M-20\20-1458M\CADD\Drawing Set\30-la855.dgn
Plot Date: 8/30/2021
Plot Location:

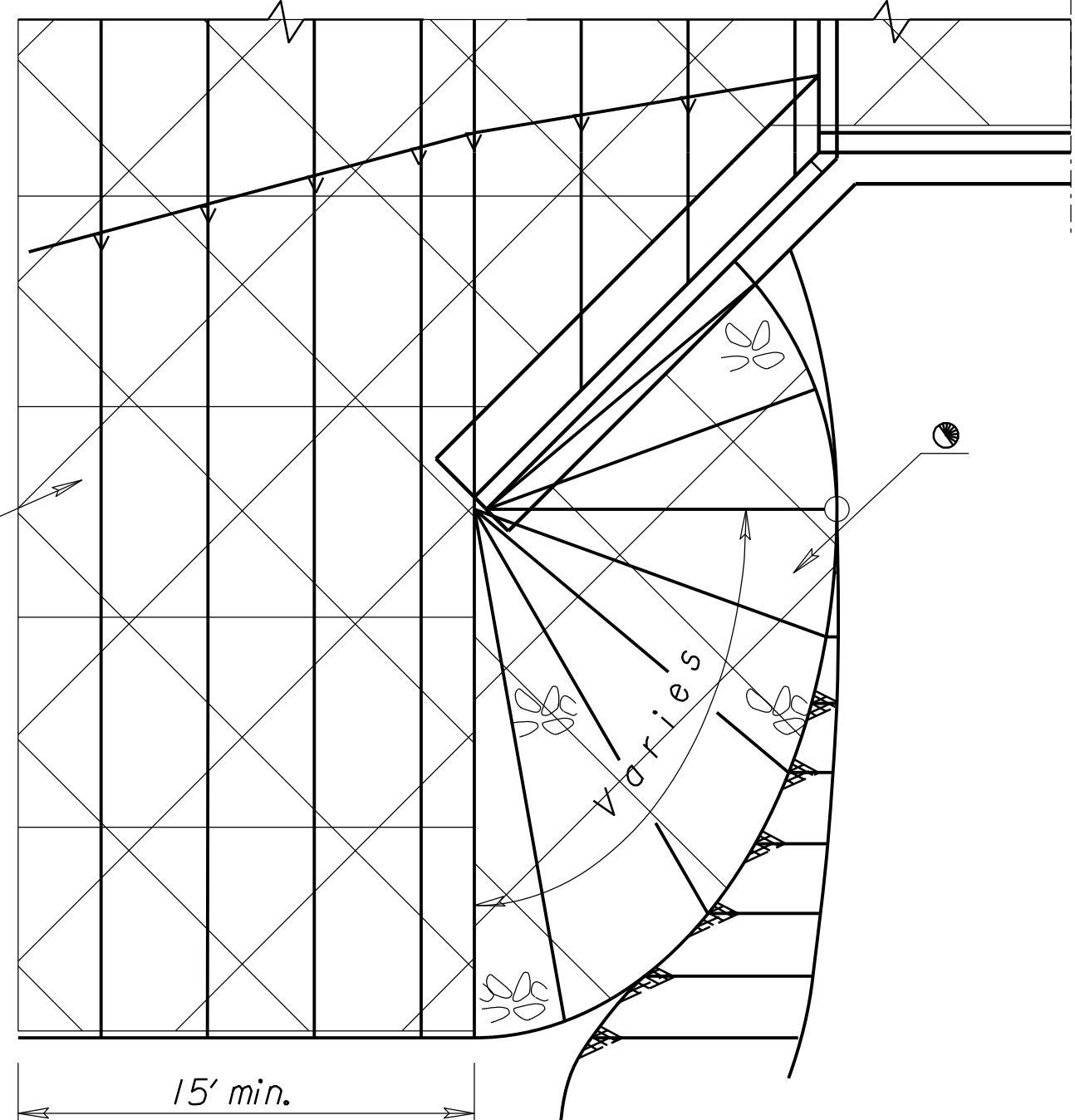
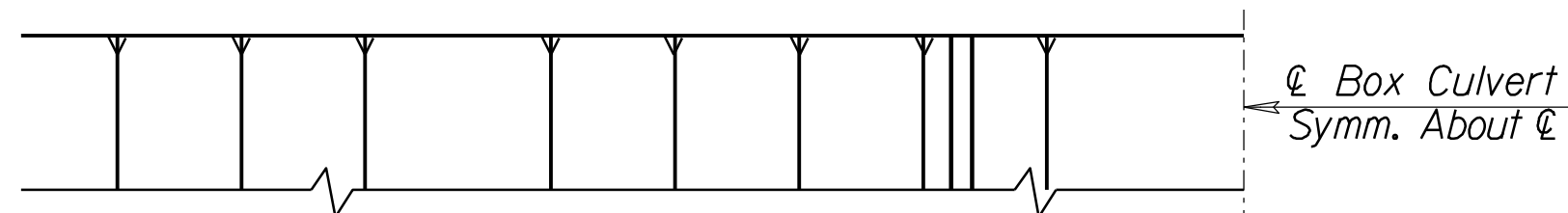
20-1458M	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
	KANSAS	43 C-5078-01	2022	30	44



PARTIAL PLAN PIPE



Limits of Erosion Control Blanket



PARTIAL PLAN BOX CULVERT

INSTALLATION DETAILS FOR EROSION CONTROL CLASS I

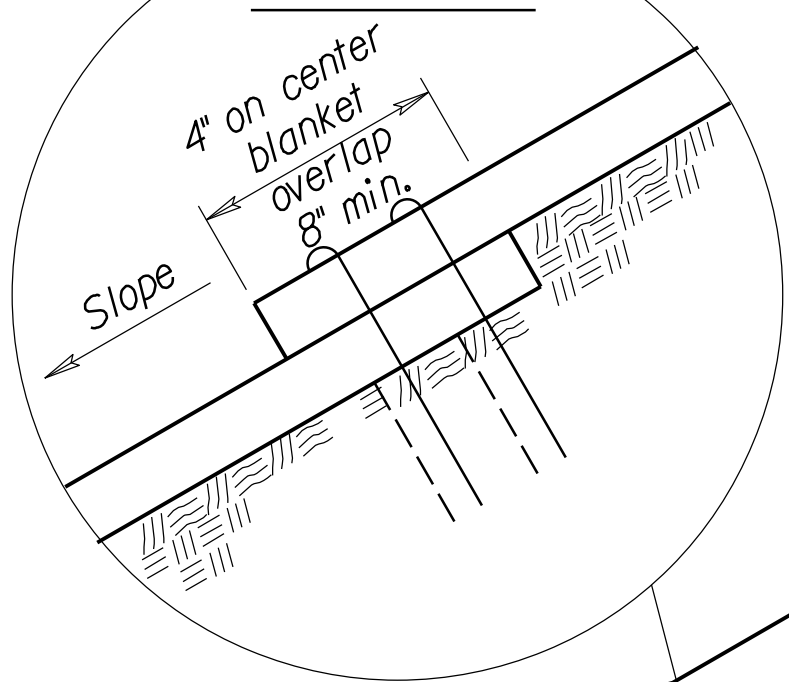
Erosion Control Blankets shall be laid loosely in the direction of the slope, beginning at the bottom of the slope. In order for blanket to be in contact with the soil, lay blanket loosely, avoiding stretching.

- ANCHOR SLOTS:** The top of the blanket should be "slotted in" at the top of the slope and anchored in place with anchors 6 inches apart. The slots should be 6 inches wide x 6 inches deep with the blanket anchored in the bottom of the slot, then backfilled, tamped and seeded.
- LONGITUDINAL SEAMS:** The edges of the blanket should overlap each other a minimum of 6 inches, with anchors catching the edges of both blankets.
- SPLICE SEAM:** When splices are necessary, overlap a minimum of 8 inches in direction of water flow. Stagger splice seams.
- TERMINAL FOLD:** The bottom edge of the blanket shall be turned under a minimum of 4 inches, then anchored in place with anchors 9 inches apart.
- TYPICAL ANCHORS:** Anchor design shall be as recommended by the manufacturer.
- STAPLE CHECK:** Establish Staples in 2 rows 4" on center apart. Staple Checks - shall be 30' apart.

● Erosion Control Class I may be omitted if the area is immediately covered by permanent slope protection (where directed by the plans).

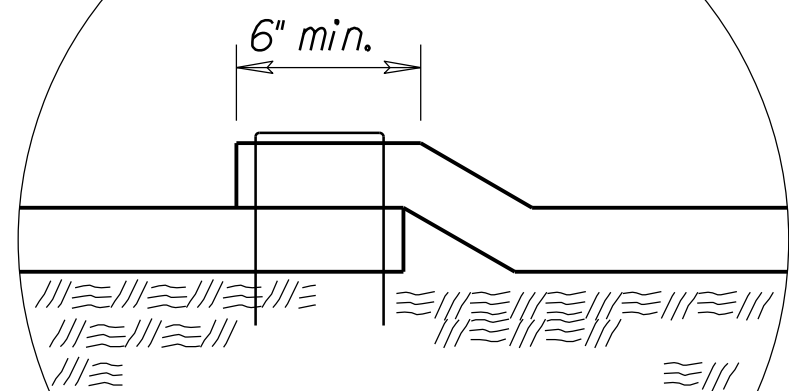
NOTE:
Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.
Single post ring and shank staple is acceptable.

SPLICE SEAM



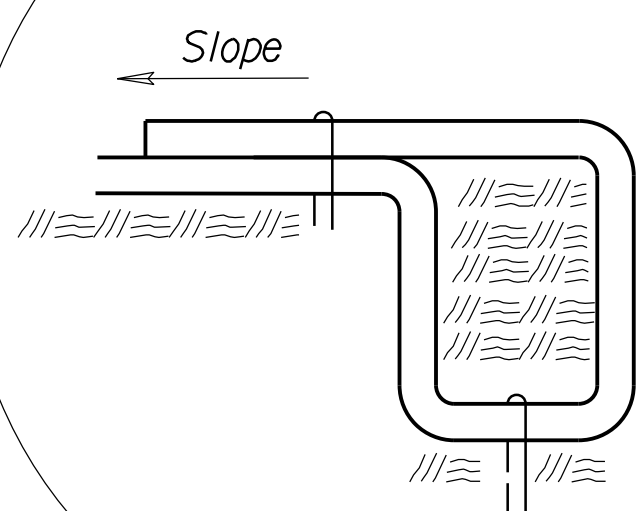
6" min.

LONGITUDINAL SEAM



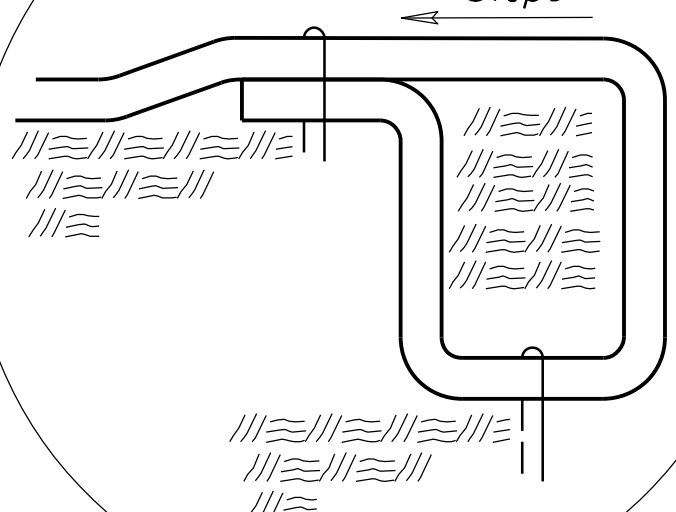
ANCHOR SLOT

Alt. A

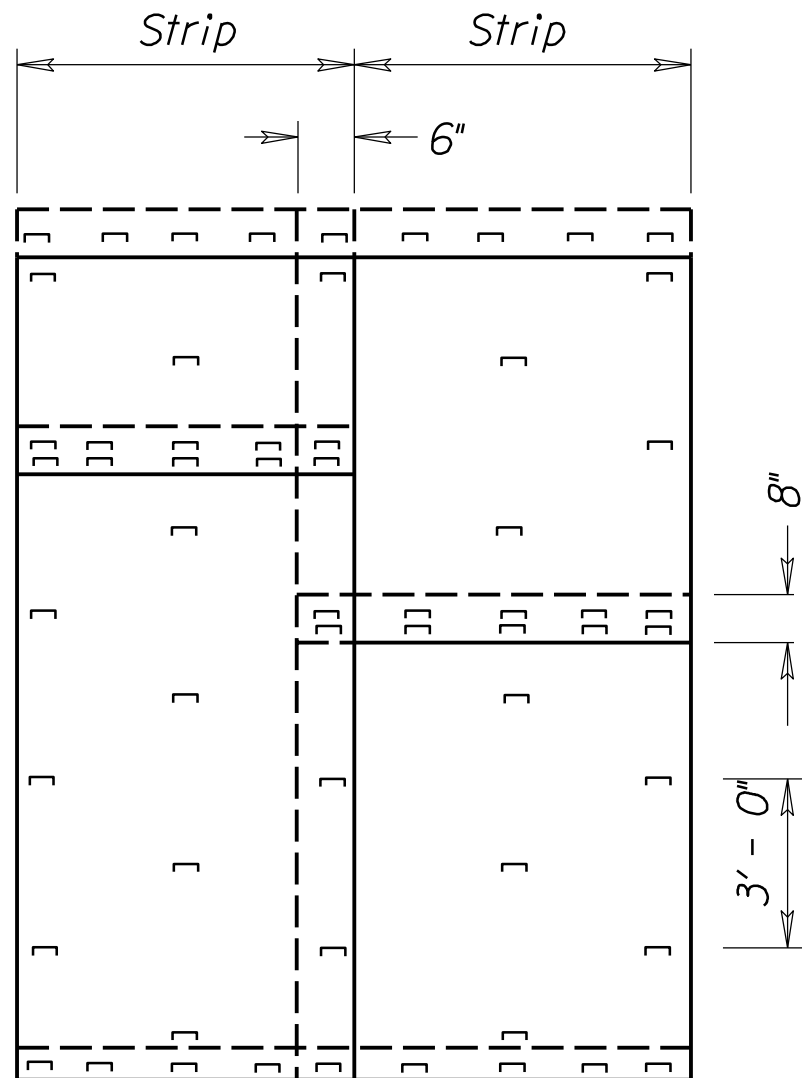
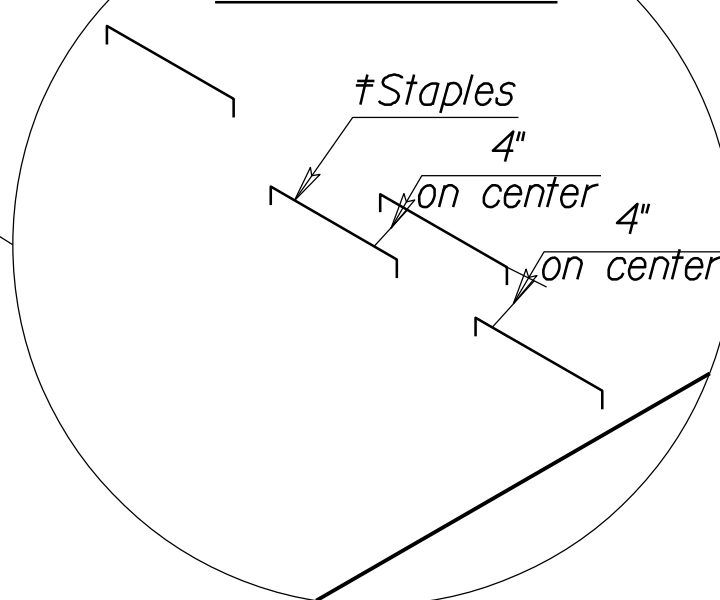


ANCHOR SLOT

Alt. B



STAPLE CHECK



PLAN VIEW - ANCHORING DIAGRAM

ISOMETRIC VIEW

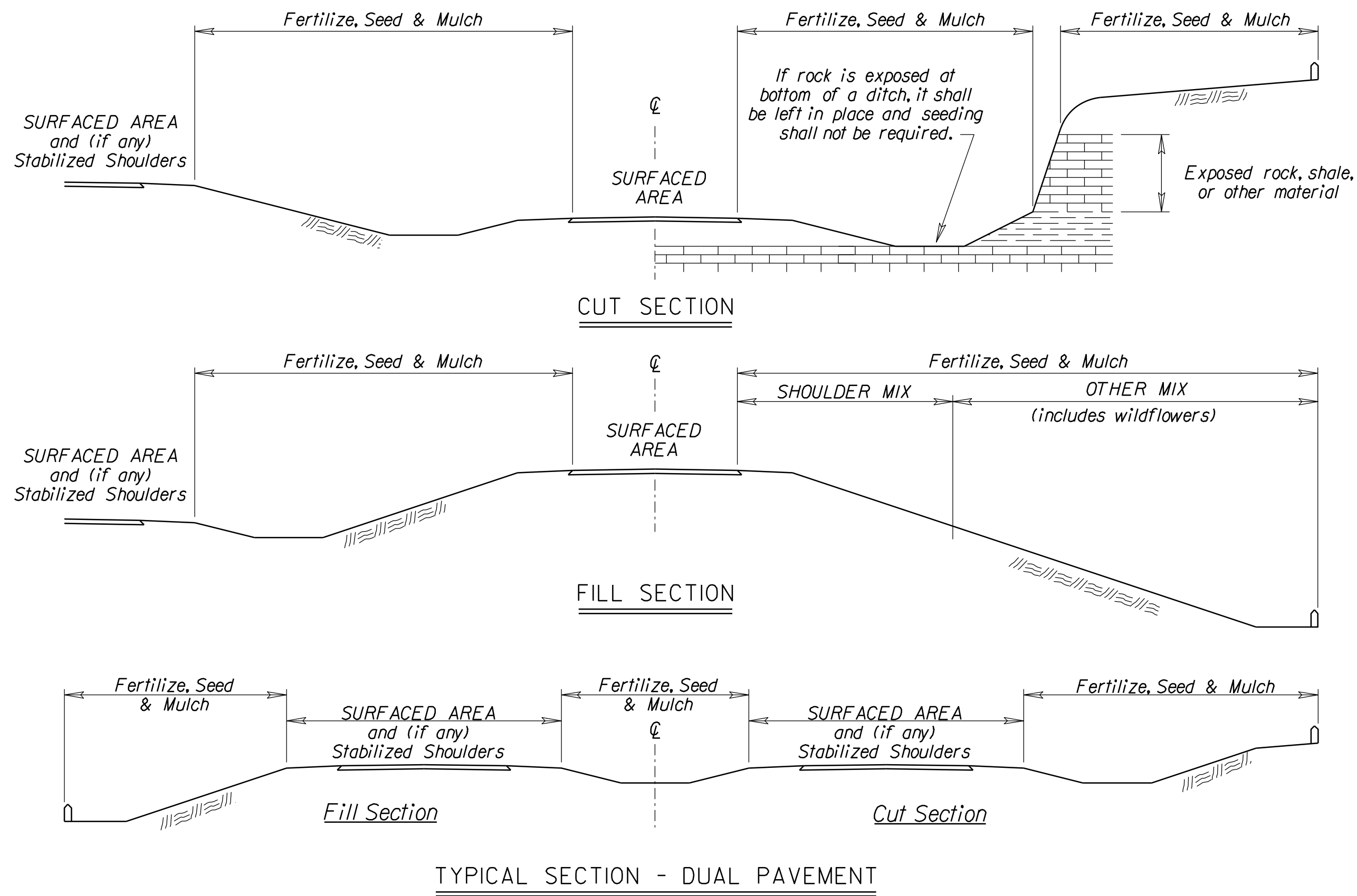
4	3/01/15	Revised Standard	RAA	SHS
3	2/23/15	Revised Standard	RAA	SHS
2	9/15/14	Revised Standard	MRM	SHS
1	9/10/07	Revised Standard	MRM	SHS
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION

INSTALLATION DETAIL
EROSION CONTROL CLASS I
SLOPE PROTECTION

L855	3/10/2015	APP'D	Scott H. Shields
DESIGNED	RAA	DETAILED	RAA
QUANTITIES	CADD	QUAN.CK.	RAA
DESIGN CK.	DETAIL CK.	CADD CK.	RAA

GENERAL NOTES



GRASS & WILDFLOWER SEEDING SEASONS	
COOL SEASON GRASSES	WARM SEASON GRASSES & WILDFLOWERS
February 15 thru April 20 August 15 thru September 30	November 15 thru June 1
SPECIES	SPECIES
Bluegrasses	Bermuda Grass
Brome Grasses	Big Bluestem
Canada Wildrye	Blue Grama
Fescues	Buffalo Grass
Prairie Junegrass	Indiangrass
Ryegrasses	Little Bluestem
Sterile Wheatgrass	Sand Bluestem
Tall Dropseed	Sand Dropseed
Western Wheatgrass	Sand Lovegrass
	Side Oats Grama
	Switchgrass
	Wildflower Mixes
When the area to be seeded is 1 acre or more, if Cool Season grasses are mixed with Warm Season grasses, seed the area during the Warm Season.	
When the area to be seeded is less than 1 acre, seed the area any time of the year.	

SODDING SEASONS	
COOL SEASON GRASSES	WARM SEASON GRASSES
March 1 thru April 15 September 1 thru November 15	May 15 thru September 1
SPECIES	SPECIES
Bluegrass Sod	Buffalo Grass Sod
Fescue Sod	

If the soils workable, the Engineer may allow placement of sod between November 15 and March 1. If sod is placed during this time, maintain the sod until 20 days after the beginning of the spring sodding season.

The entire disturbed area, excepting the paved or surfaced areas, steep rocky slopes and areas of undisturbed native sod or other desirable vegetation shall be fertilized (limed when required), seeded and mulched. Soil preparation shall conform to the Standard Specifications except as noted below.

All borrow areas shown on the plans are to be fertilized, seeded, and mulched. However, operation in borrow areas where crops are growing may be omitted when requested by the owner.

If temporary cover has provided stable slopes with no erosion, seed the permanent grasses into the existing cover. If there has been erosion that requires repair prior to seeding, then it may be necessary to regrade the area, resulting in bare ground.

FERTILIZER: A ratio and application rate that equals or exceeds the required minimum rate per acre of N, P₂O₅, K₂O listed in Summary of Seeding Quantities will be acceptable.

MULCHING: Mulch shall be spread uniformly over all disturbed areas and punched in the soil, unless otherwise noted on the plans. The rate of application per acre, thickness in place, for the mulching material is generally as follows:

$1\frac{3}{4}$ - $2\frac{1}{4}$ Tons per Acre = $1\frac{1}{2}$ " loose depth spread uniformly over acre.

Agricultural products, such as native prairie hay, used for mulching and erosion control practices, excluding wood based mulch, shall meet the North American Weed Free Forage Standards.

Other vegetative mulches are acceptable only with the Engineer's concurrence.

The above rate is a guide. It will be at the discretion of the Engineer to determine what rate is sufficient for adequate protection of newly seeded areas.

NATIVE WILDFLOWER MIX I			
PLS	RATE	NAME	QTY (lb)
	0.3	Butterfly Milkweed	
	0.3	Common Milkweed	
	0.3	Black Eyed Susan	
	0.5	Blanket Flower	
	0.5	False Sunflower	
	0.5	Lance-Leaf Coreopsis	
	0.2	Maximilian Sunflower	
	0.1	New England Aster	
	0.2	Pinnate Prairie Coneflower	
	0.2	Plains Coreopsis	
	0.3	Purple Coneflower	
	0.3	Upright Prairie Coneflower	
	0.3	Dames Rocket	
	0.3	Lemon Mint	
	0.2	Pitcher Sage	
	0.2	Wild Bergamot	
	1.0	Illinois Bundleflower	
	0.2	Common Evening Primrose	
	0.1	Hoary Verbena	
	0.8	Purple Prairie Clover	
	0.3	Roundhead Lespedeza	
	3.0	Showy Partridge Pea	
	0.2	White Prairie Clover	
	10.3	Total (lb)	

NATIVE WILDFLOWER MIX 2		
PLS RATE	NAME	QTY (lb)
0.3	Butterfly Milkweed	
0.3	Black Eyed Susan	
0.5	Black Sampson Coneflower	
1.0	Blanket Flower	
0.2	Maximilian Sunflower	
0.2	Plains Coreopsis	
0.2	Upright Prairie Coneflower	
0.2	Western Yarrow	
0.3	Lemon Mint	
0.4	Pitcher Sage	
1.5	Illinois Bundleflower	
0.2	Common Evening Primrose	
1.0	Blue Wild Indigo	
0.4	Leadplant	
0.4	Purple Prairie Clover	
0.3	White Prairie Clover	
7.4	Total (lb)	

Package and deliver the wildflower seed separately from the grass seed mix. Package and deliver the Tall Drop Seed separately from the grass seed and the wildflower mix. Place the grass seed (except Tall Drop Seed) in the large seed box and drill (cover) seed $\frac{1}{8}$ " - $\frac{1}{4}$ ". Place the wildflower seed in a separate seed box and drill (cover) seed $\frac{1}{16}$ " maximum. Place the Tall Drop Seed in a separate (third) seed box and place the seed (using the seed drill) on the soil surface.

OPTION: Broadcast Tall Drop Seed on the soil surface.

[illegible]

NOTE: When seeding less than 1 acre, temporary and permanent seeding shall be combined and seeded at the same time. There is no seasonal restriction for seeding projects less than 1 acre.

SHLDR = Seeded with the Shoulder Mix. Typically 15 feet for 2-lane roads and 30 feet for 4-lane roads. Includes outside roadsides, turfed portions of shoulders, and turfed portion of the median.

OTHER - Seeded with the "Other" Mix. Designated as all other turf areas, except the Shoulder. Usually includes a Native Wildflower Mix.

NOTE: Projects less than 1 acre shall be bid as "Seeding" by the lump sum. All disturbed areas shall be seeded, fertilized and mulched at the listed rate per acre. The acres are estimated.

Refer to the Standard Specifications, Division 900, Section 904 'Seeding', and Section 907 'Sodding', for the seeding and sodding seasons.

* See LA852A for mulching quantity. The quantity of mulch is estimated (Acres of Seeding X 1.5 X 2 Tons/Acre). The total mulch required shall be determined in the field. The bid item for mulching shall be paid for according to the Standard Specifications.

2	11/25/20	Updated Seeding / Sodding Periods Charts	MRD	ML
1	08/03/20	Revised Standard	MRD	SHS
NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION

PERMANENT SEEDING SUMMARY OF SEEDING QUANTITIES

LA850

FHWA APPROVAL		05/06/2019		APP'D		Mervin Lare	
DESIGNED MRD		DETAILED MRD		QUANTITIES		CADD	
DESIGN CK.		DETAIL CK.		QUAN,CK.		CADD CK.	

Std. Base File:	-----
Plotted By: <i>untitled</i>	Plot Location:
File: <i>M:\20-20-458\NCAD\Drawing Set\31-10650.dgn</i>	
Plot Date: <i>12/3/2021</i>	

Plotted : \$SYTIME\$\$
File : \$\$DGN\$PEC\$\$

Drawn By : untitled
File : \$\$DGN\$PEC\$\$

1) Design Speed: Those items delegated to temporary traffic control should be designed and installed using the posted/legal speed of the roadway prior to work starting.

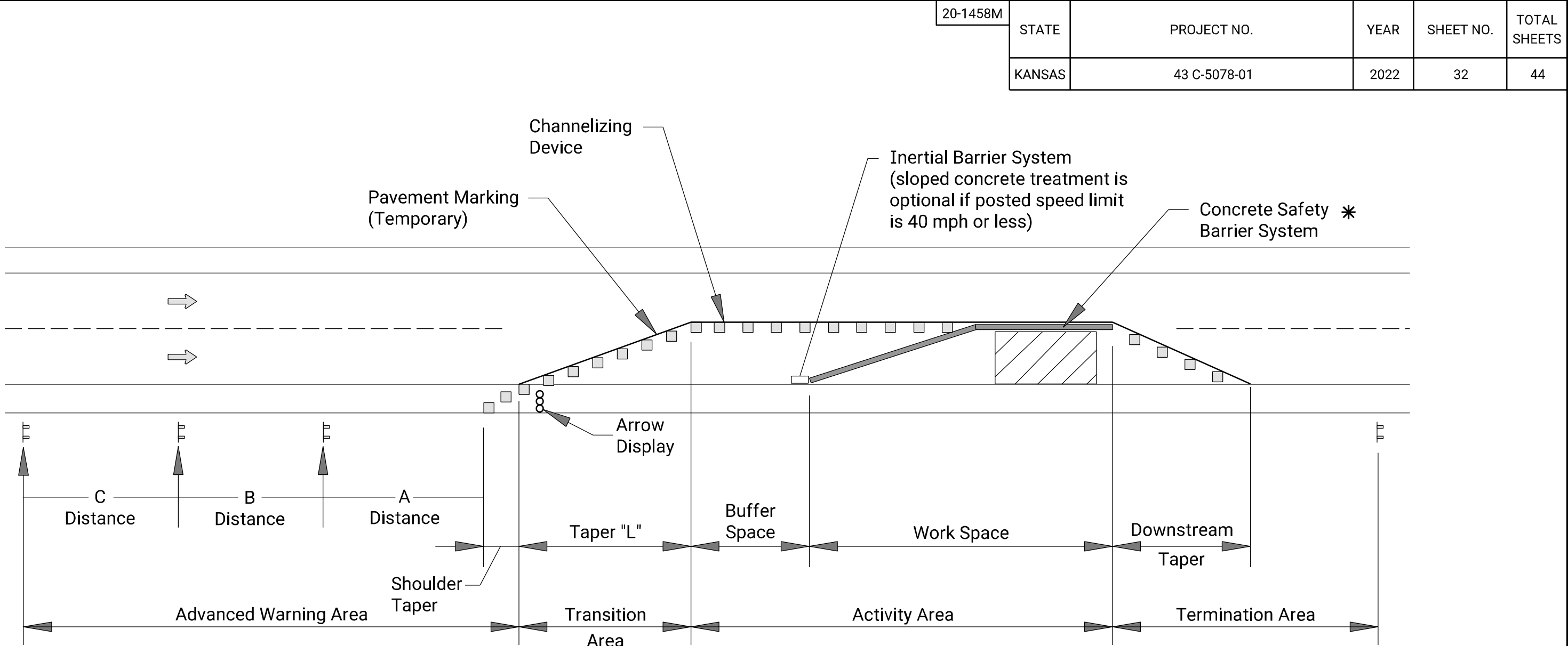
2) Minimum Lane Width: Lane widths shall be a minimum of 11' (measured between centerlines of pavement markings) or as shown on the plans, or as directed by the engineer. A lane width less than 11' may require restricted roadway width signing.

3) Consideration should be made to separate pedestrian and, if needed, bicycle movements from both work site activity and vehicular traffic. Unless a reasonable safe route that does not involve crossing the roadway can be provided, pedestrians should be appropriately directed with advance signing that encourages them to cross to the opposite side of the roadway. In urban and suburban areas with high vehicular traffic volumes, these signs should be placed at intersections (rather than midblock locations) so that pedestrians are not confronted with midblock work sites that will induce them to attempt skirting the work site or making a midblock crossing.

4) When existing pedestrian facilities are disrupted, closed, or relocated, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility.

5) When the driving surface open to traffic is milled or is a temporary surface made of loose material, or when directed by the engineer a W8-15 (Grooved Pavement) or W8-7 (Loose Gravel) sign shall be used on mainline approaches. This sign should be placed a "C" distance after the W20-1 (Road Work Ahead) sign. A W8-15p motorcycle plaque shall be used to supplement the W8-15 or W8-7 signs. All signs shall be displayed as long as the condition is present.

6) Alternative temporary rumble strip options may be available. Please contact the Temporary Traffic Control Unit for more information at 785-296-1179 or 785-296-1183.



TYPICAL WORK ZONE COMPONENTS

* When concrete barrier system is used, portable channelizing devices are not needed along the tangent barrier section.

Minimum advance warning sign spacing (in feet):

SPEED (MPH) *	A	B	C
URBAN (40 MPH OR LOWER)	100	100	100
URBAN (45 MPH OR HIGHER)	350	350	350
RURAL (55 MPH OR LOWER)	500	500	500
RURAL (60 MPH OR HIGHER)	750	750	750
EXPRESSWAY/FREEWAY	1000	1500	2640

* Posted speed prior to work starting

The minimum spacing between signs shall be no less than 100', unless directed by the engineer.

The spacing between any signs may be increased beyond the minimum values in the table above as approved by the engineer in order to maximize visibility.

Taper Formulas:

$L = WS$ for speeds of 45 MPH or more

$L = WS^2/60$ for speeds of 40 MPH or less

Where: L = Minimum length of taper in feet
 S = Numerical value of posted speed prior to work starting in MPH
 W = Width in offset feet

Shifting Taper= $1/2 L$
Shoulder Taper= $1/3 L$

Channelizer Placement:

- The spacing between devices in transition area (taper) should not exceed a distance in feet equal to 1/2 the posted speed limit in mph prior to work starting.
- The spacing between devices in the advanced warning area and the activity area should not exceed a distance in feet equal to two times the posted speed limit in mph prior to work starting.
- Channelizing devices shall be placed for optimum visibility, normally at right angles to the traffic flow.
- Place directional indicator barricades in series to direct traffic onto the new path. The arrow sign should not be visible to opposing traffic.
- Alternating diagonal orange and white striping must slope downward in the direction traffic is expected to pass.

Buffer Space

SPEED (MPH) *	20	25	30	35	40	45	50	55	60	65	70	75
LENGTH (ft)	115	155	200	250	305	360	425	495	570	645	730	820

* Posted speed prior to work starting

Neither work activity nor storage of equipment, vehicles, or material should occur in the buffer space. When a protection vehicle is placed in advance of the work space, only the space upstream of the vehicle constitutes the buffer space.

If temporary concrete safety barrier system is used to separate approaching traffic from the work space, the barrier system shall be considered part of the activity area. A full lane width should be available throughout the length of the buffer space. See typical work zone components above.

3					
2	03/13/18	W8-15p usage changed to Shall	R.W.B.	E.G.K.	
1	08/18/15	Channelizer spacing info	R.W.B.	K.E.	
NO.	DATE	REVISIONS	BY	APPD	

KANSAS DEPARTMENT OF TRANSPORTATION

TRAFFIC CONTROL
GENERAL NOTES

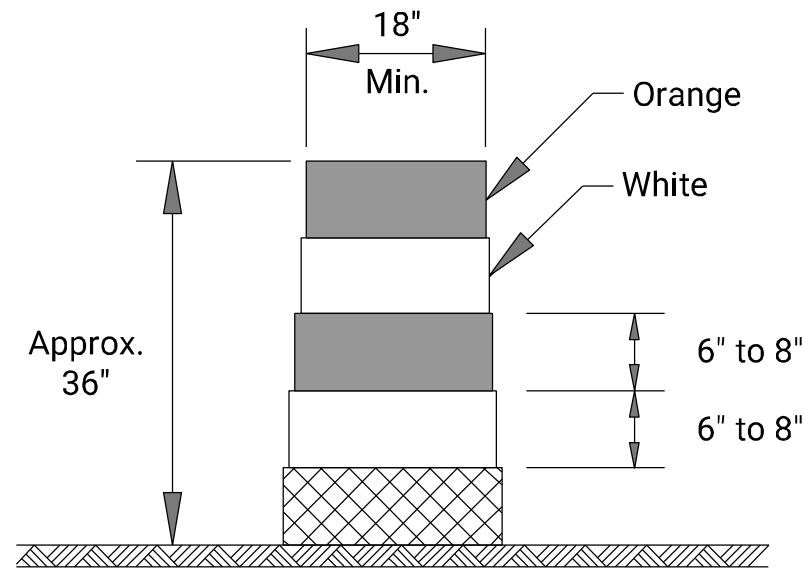
TE700

FHWA APPROVAL		03/13/18	APPD	Eric Kocher	
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.		DETAIL CK.		QUAN. CK.	TRACE CK.

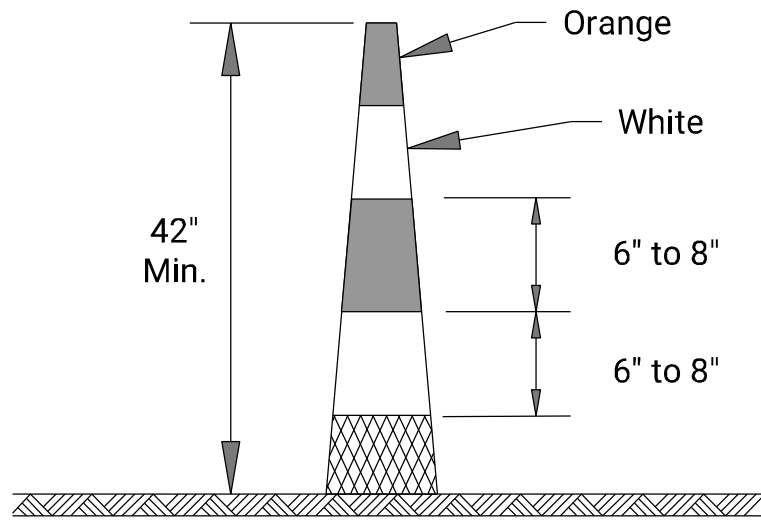
Drawn By : untitled
File : \$DGN\$SPEC\$

Plotted : \$SYTIME\$
\$KDOTGRP\$

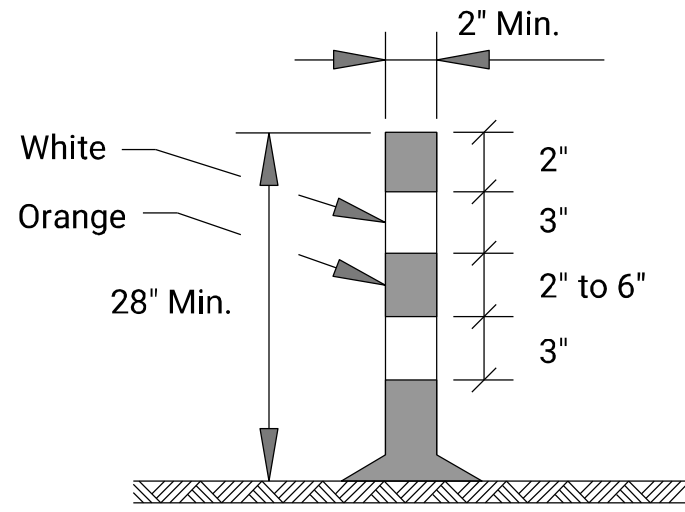
20-1458M	STATE	PROJECT NO.	YEAR	SHEET NO.	TOTAL SHEETS
	KANSAS	43 C-5078-01	2022	33	44



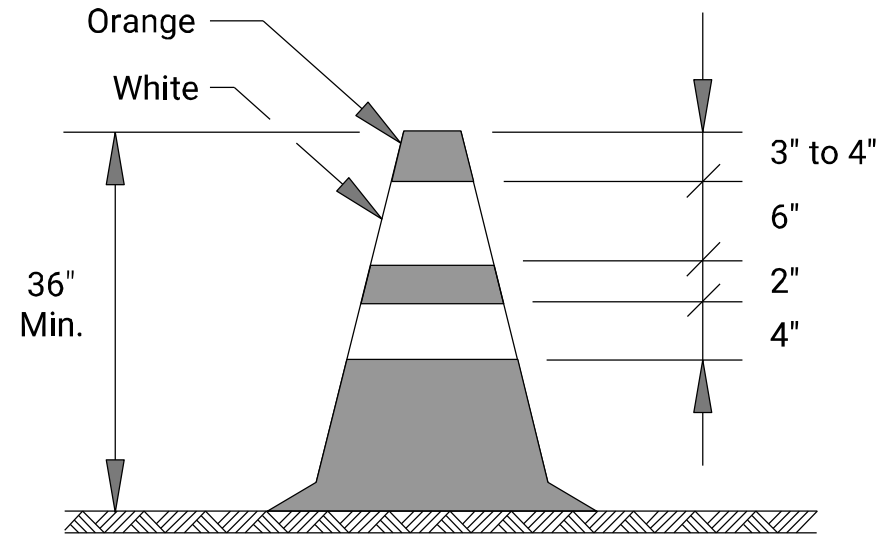
DRUM



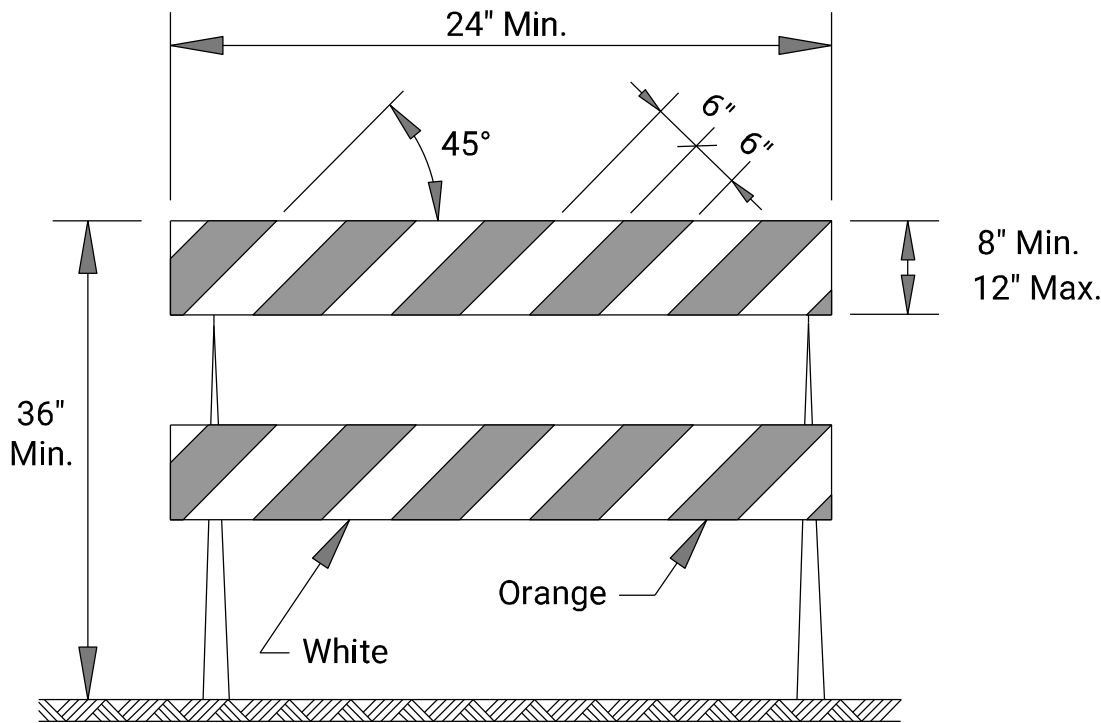
CONICAL
DELINEATOR



TUBULAR MARKER
Striping as shown for up to 42".

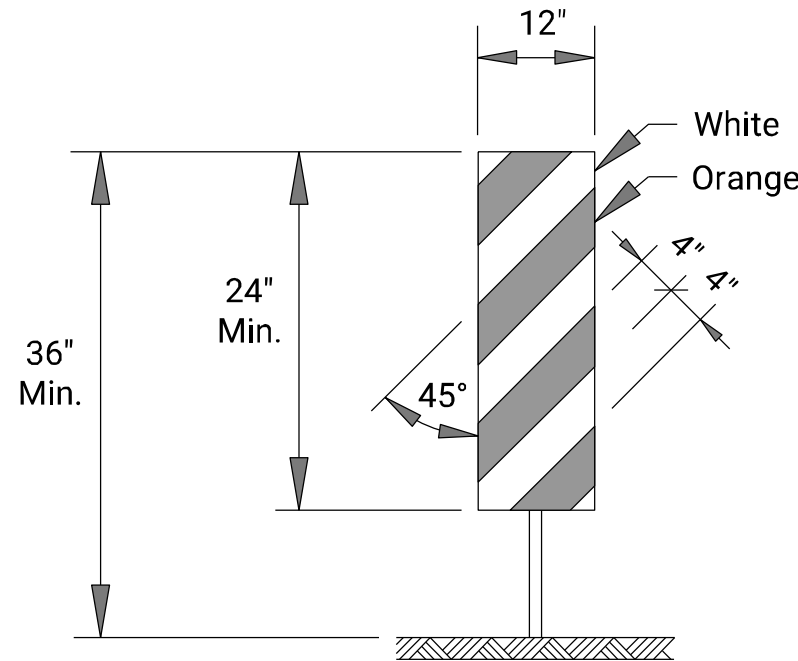


TRAFFIC CONE



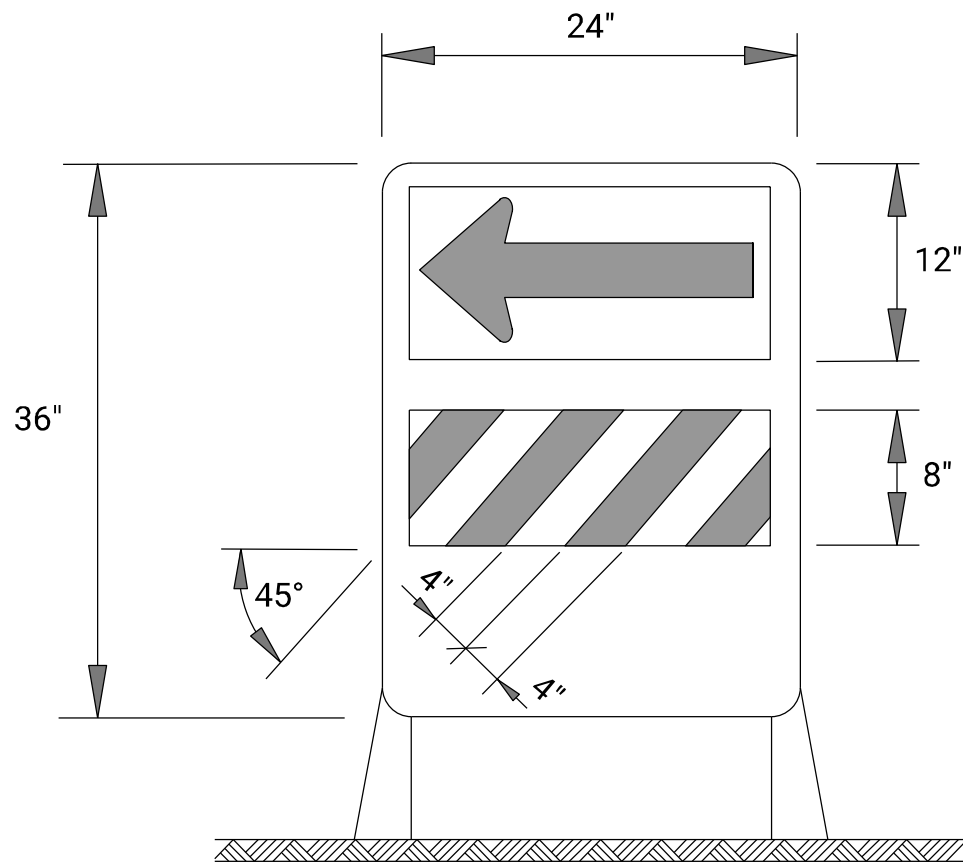
TYPE 2 BARRICADE

For rails less than 36" long, 4" wide stripes may be used.
All stripes shall slope downward to the traffic side for channelization.



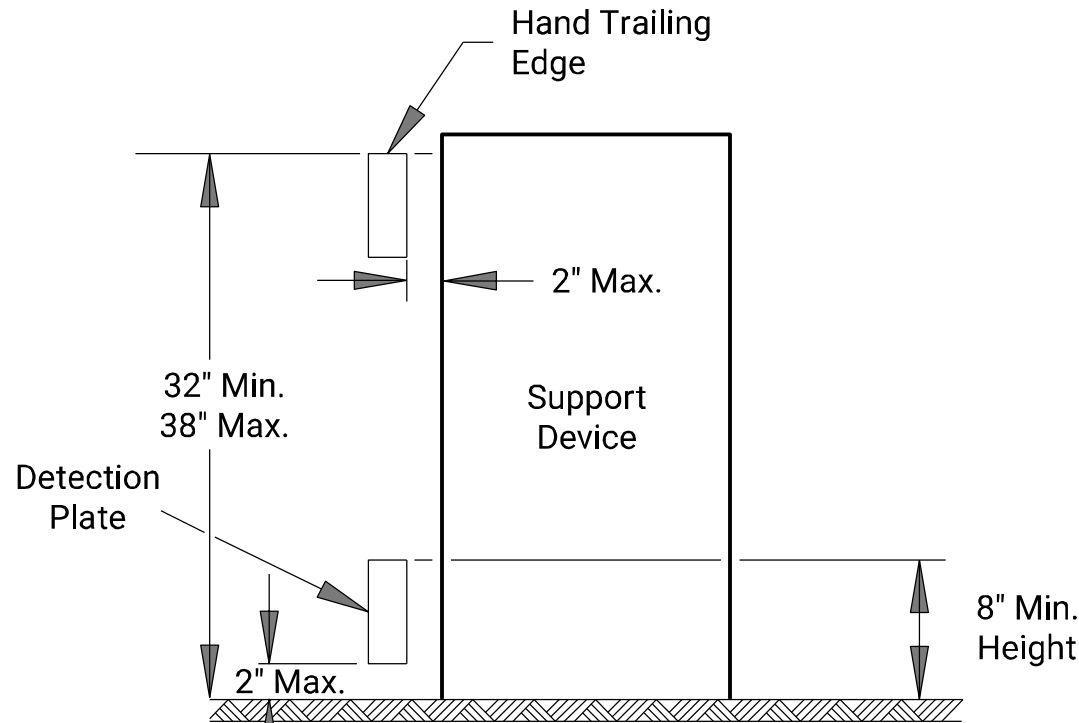
VERTICAL PANEL

The stripes shall slope downward to the traffic side for channelization.



DIRECTION INDICATOR BARRICADE

The stripes shall slope downward in the direction traffic is to pass.
The direction indicator barricade shall be used in series to direct the motorist into the intended lane of travel.



PEDESTRIAN CHANNELIZER

1. Support device shall not project beyond the detection plate into the pathway.
2. Hand trailing edges and detection plates are optional for continuous walls.
3. Interconnect pedestrian channelizers to prevent displacement and to provide continuous guidance through or around work.
4. Alternate pathways shall be firm, stable, and slip resistant.
5. Treat height differentials > 1/2" in the surfaces of alternate paths with a firm, stable, and slip resistant temporary ramp having a slope of 12:1 or flatter and having a width equal to the alternate path.
6. Use alternating orange/white on interconnected devices.

Location		Cross-overs	Shoofly Divisions	Tangents	Tapers	Ramps	Head to Head	Object Identifier	Lead-in Devices	Gores
Portable	Drums	Yes	Yes	Yes	Yes	Yes	(1)	Yes	Yes	Yes
	Conical Delineators	Yes	Yes	Yes	Yes	Yes	(1)	Yes	Yes	Yes
	Vertical Panels	(2)	(2)	(2)	(2)	(2)	(1,2)	Yes	(2)	(2)
	Direction Indicator Barricade	No	No	No	Yes	No	No	No	No	No
	Type 2 Barricade	(2)	(2)	(2)	(2)	No	No	Yes	No	No
	Traffic Cones	No	No	(4)	(4)	(4)	No	(4)	(4)	(4)
Fixed	Tubular Markers	(3)	(3)	(3)	No	(3)	Yes	No	Yes	Yes
	Vertical Panels	(3)	(3)	(3)	(3)	(3)	(3)	Yes	(2,3)	(2)

- (1) Not allowed on centerline delineation along freeways or expressways.
- (2) The stripes shall slope downward to the traffic side for channelization.
- (3) May be used upon the approval of the engineer.
- (4) Daytime operations only.

3					
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NO.	DATE	REVISIONS			BY APP'D
KANSAS DEPARTMENT OF TRANSPORTATION					
TRAFFIC CONTROL CHANNELIZING DEVICES					
TE702					
FHWA APPROVAL		06/01/15	APP'D	Kristina Erickson	
DESIGNED	L.E.R.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.		DETAIL CK.		QUAN. CK.	TRACE CK.

Drawn By : untitled
File : \$\$DGN\$PEC\$\$
Plotted : \$\$YTIME\$\$
File : \$\$KDOTGRP\$\$

Note: Signs shown for one approach to work zone.

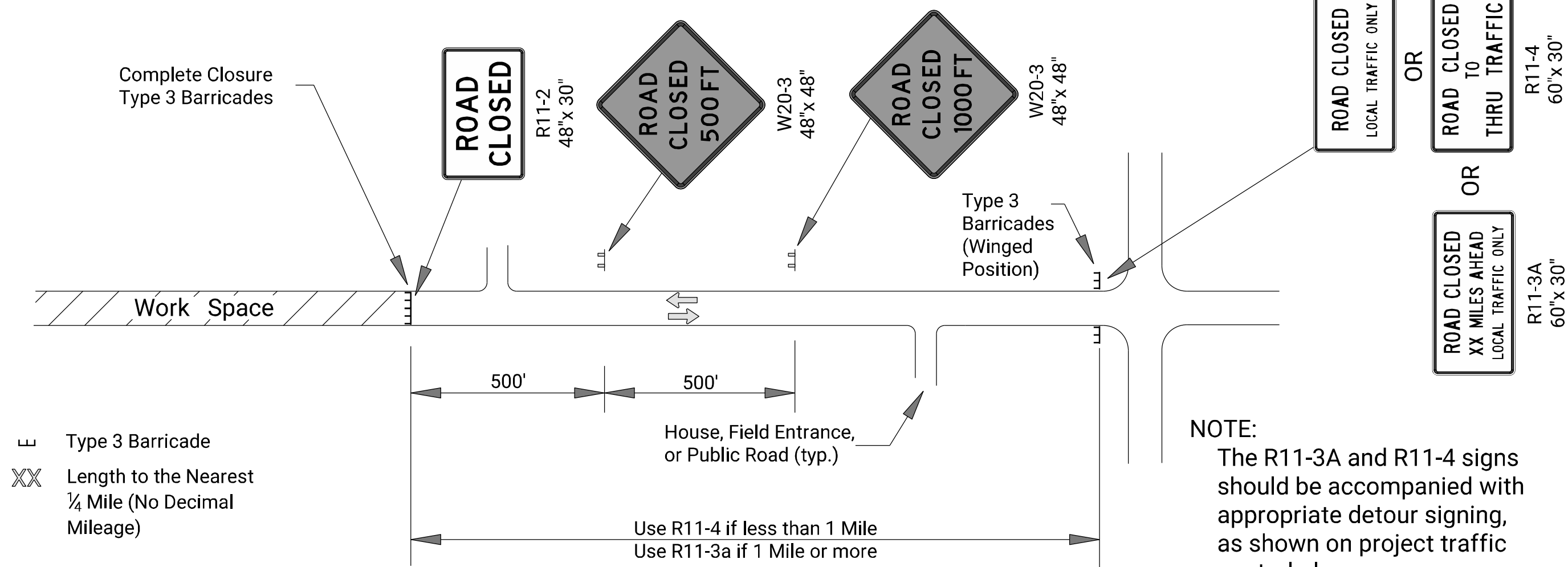


FIGURE 1: TYPICAL SIGNING FOR ROAD CLOSURE (MAINLINE OR SIDE ROAD)

Note: Sign shown for one approach to intersection (work zone).

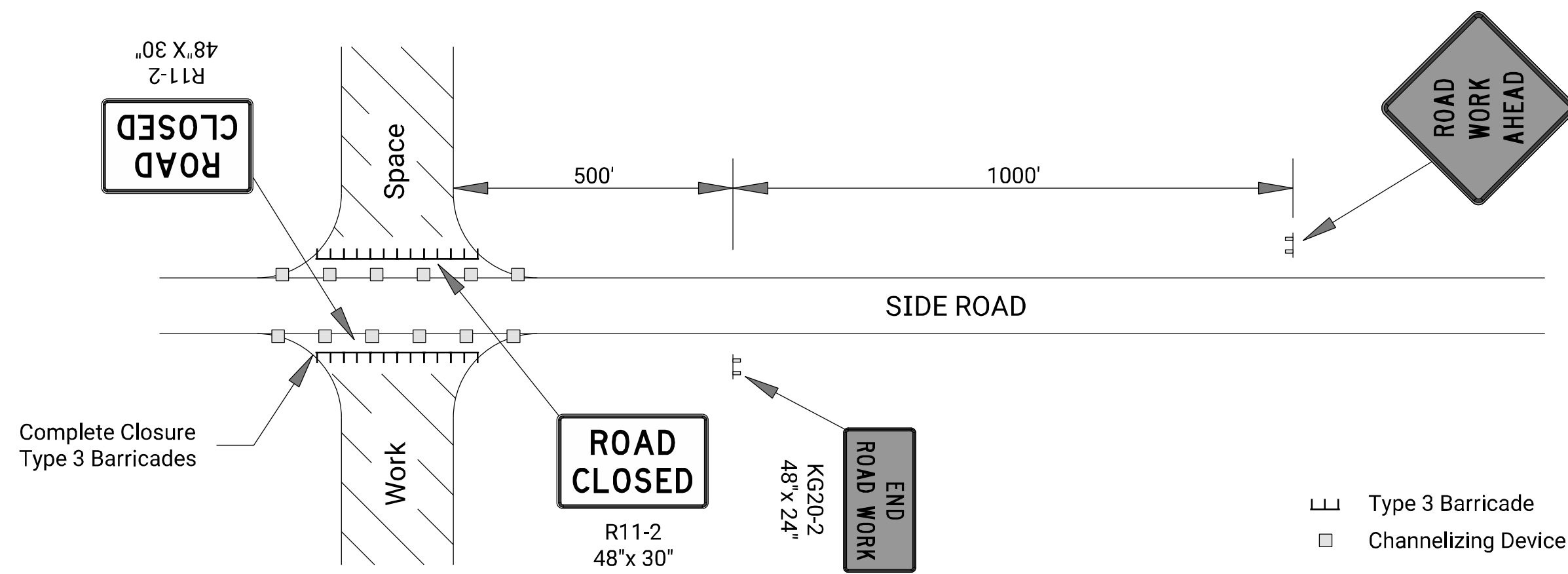


FIGURE 2: TYPICAL SIGNING FOR SIDE ROAD OPEN

Note: Signs shown for one approach to work zone.

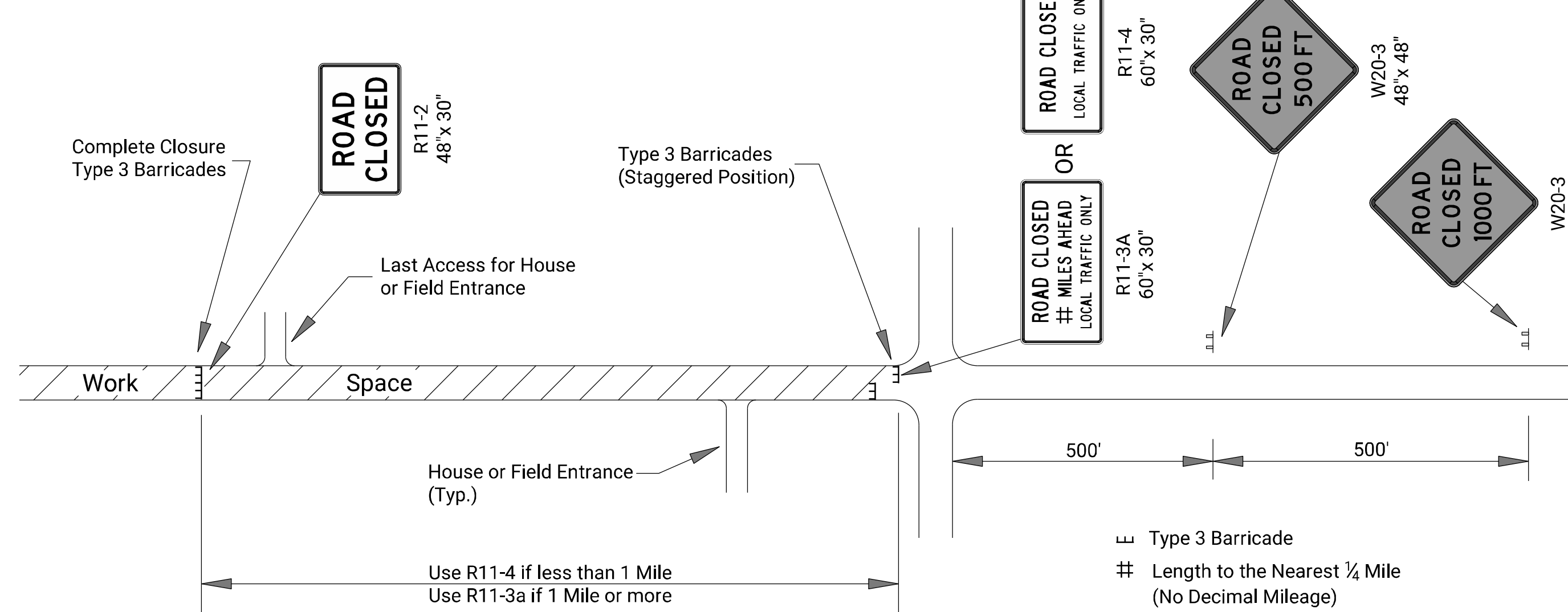


FIGURE 3: TYPICAL SIGNING FOR ROAD CLOSURE - LOCAL TRAFFIC ACCESS

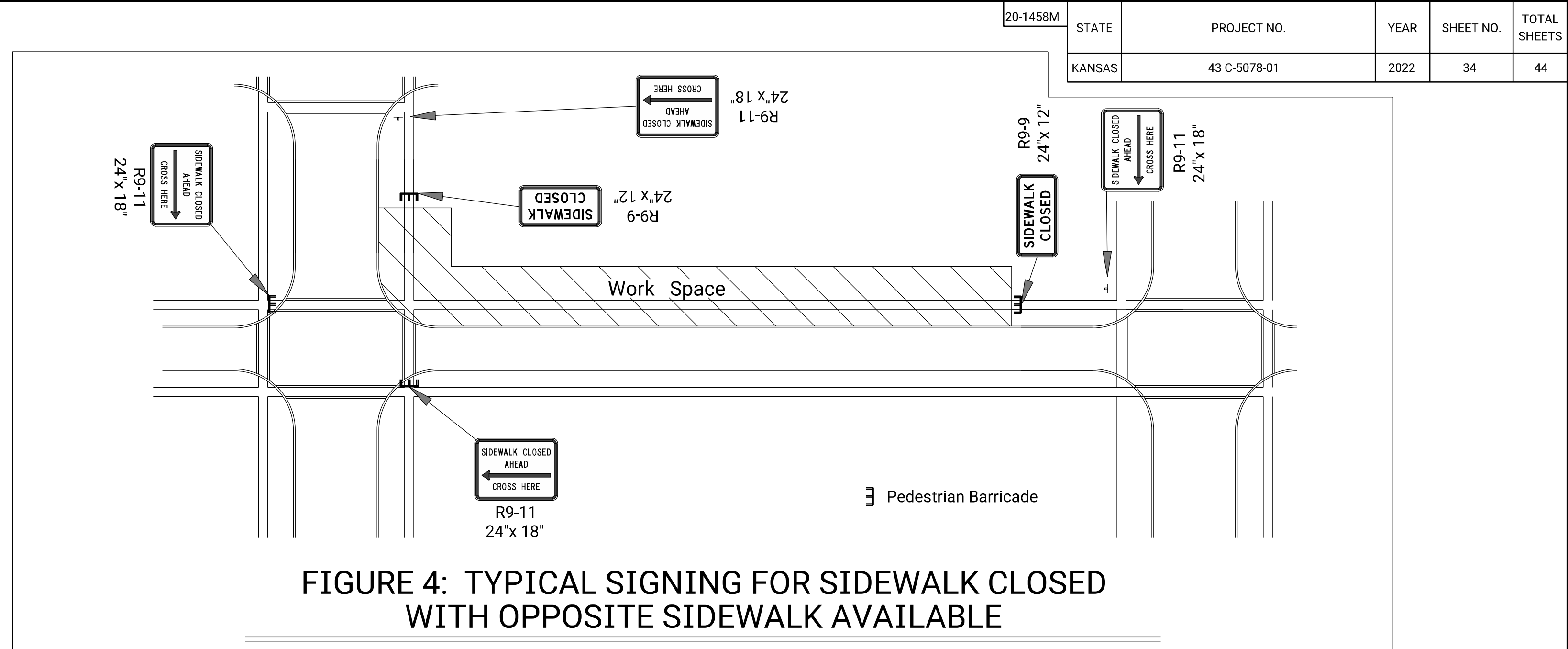
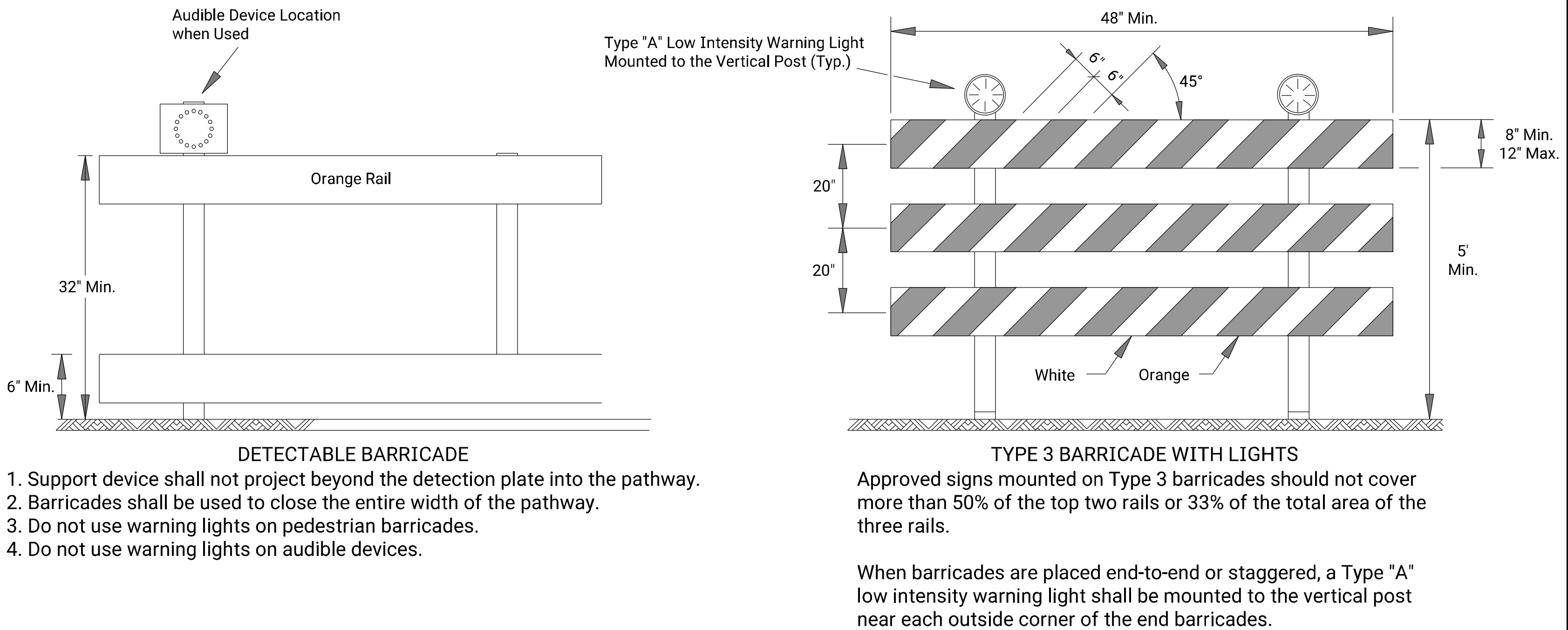


FIGURE 4: TYPICAL SIGNING FOR SIDEWALK CLOSED WITH OPPOSITE SIDEWALK AVAILABLE



ROAD CLOSED GENERAL NOTES

As shown in Figure 1, at the point where thru traffic must detour and local traffic can proceed to the location where the roadway is completely closed, the R11-3a (ROAD CLOSED # MILES AHEAD LOCAL TRAFFIC ONLY) or R11-4 (ROAD CLOSED LOCAL TRAFFIC ONLY or ROAD CLOSED TO THRU TRAFFIC) sign shall be used with Type 3 barricades (winged position), placed on the shoulders of roadway.

As shown in Figure 3, when local traffic must be allowed access into the work zone, Type 3 barricades shall be longitudinally staggered to maintain the appearance of a closed roadway. A second line of end-to-end Type 3 barricades shall be placed just beyond the last access point in the work zone, to completely close the roadway.

The R11-4 (ROAD CLOSED TO THRU TRAFFIC or ROAD CLOSED LOCAL TRAFFIC ONLY) sign shall be used when the distance to the point of complete closure of the roadway is less than 1 mile.

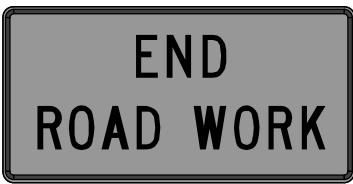
The R11-3a (ROAD CLOSED # MILES AHEAD LOCAL TRAFFIC ONLY) sign shall be used when the distance to the point of complete closure of the roadway is 1 mile or greater.

The words "BRIDGE OUT" (or BRIDGE CLOSED) may be substituted for the words "ROAD CLOSED" on the R11-3a or R11-4 sign where applicable.

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NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
TRAFFIC CONTROL CLOSURES					
TE704					
FHWA APPROVAL 06/01/15 APP'D Kristina Erickson					
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES	TRACED
DESIGN CK.		DETAIL CK.		QUAN. CK.	TRACE CK.

Drawn By : untitled
File : \$SDGNSPEC\$
Plotted : \$SYTIME\$
File : \$KDOTGRP\$

SIGN LAYOUT INFORMATION



Std. Size
Expwy/Freeway
6" C
48"x 24"



Std. Size
Expwy/Freeway
6" C
48"x 24"



Std. Size
3" C
24"x 6"

Expwy/Freeway
6" C
48"x 12"



Mileage to be Determined
by the Engineer.



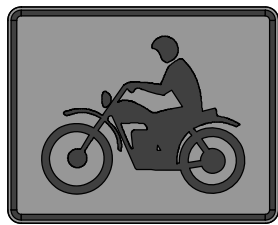
Std. Size
Expwy/Freeway
48"x 48"



Std. Size
Expwy/Freeway
8" D
48"x 48"



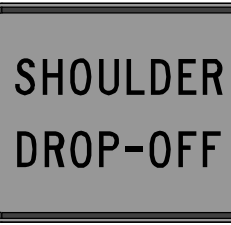
Std. Size
Expwy/Freeway
8" D
48"x 48"



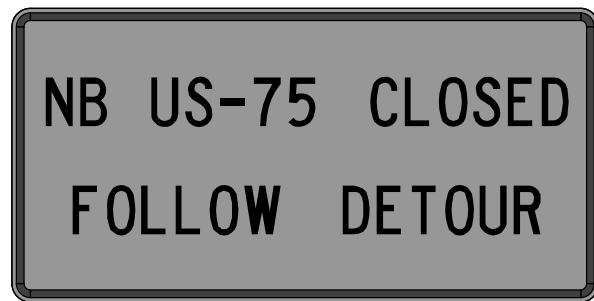
Std. Size
Expwy/Freeway
30"x 24"



Std. Size
Expwy/Freeway
8" D
48"x 48"



Std. Size
Expwy/Freeway
30"x 24"



Std. Size
6" C

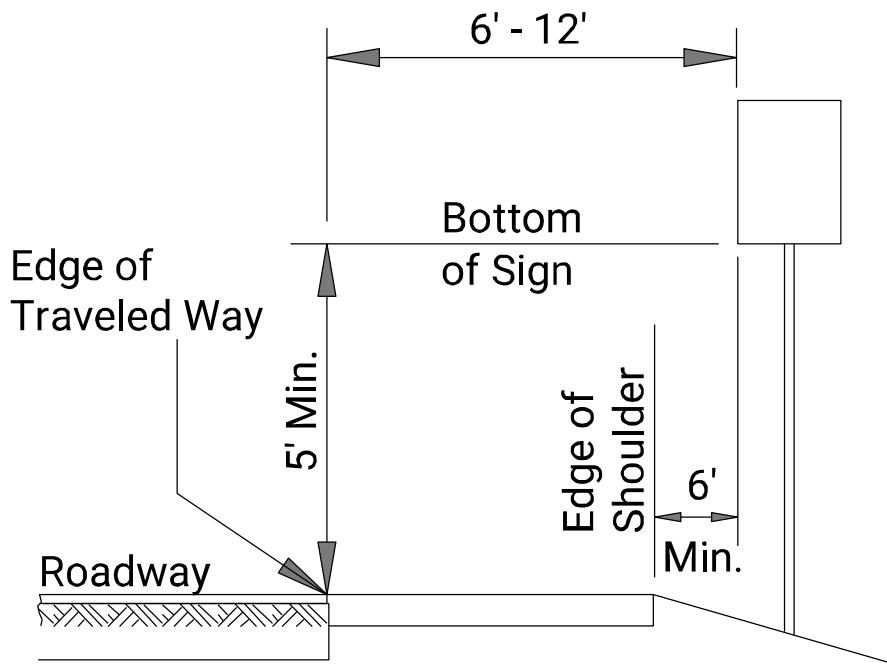
Expwy/Freeway
10" D



Std. Size
Uppercase: 6" C
Lowercase: 4.5" C

Expwy/Freeway
Uppercase: 10" D
Lowercase: 8" D

All city names and street names on special signs and destination signs
must have upper and lower case letters.

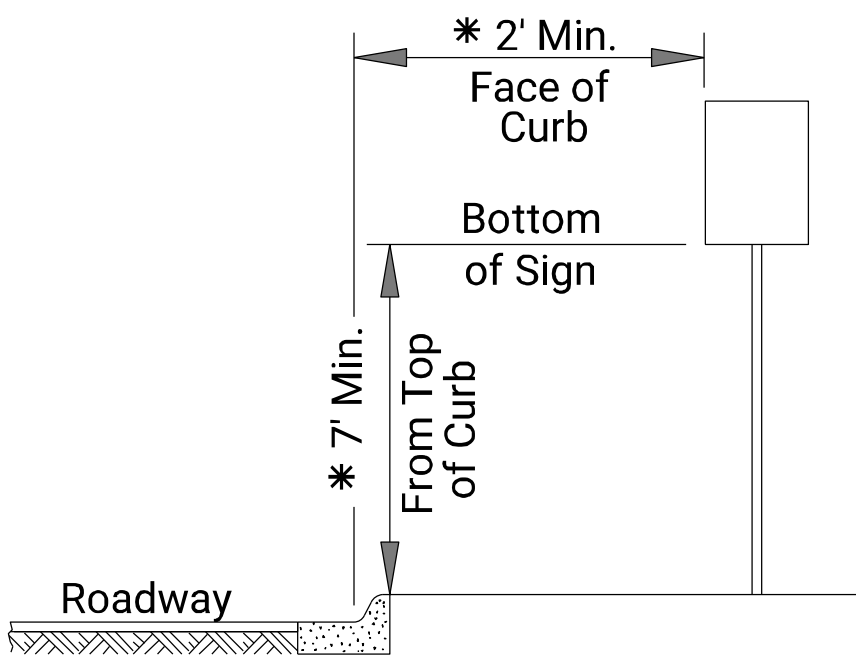


RURAL

1) Ground-mounted signs shall be mounted at a minimum height of 5' measured from the bottom of sign to the near edge of the pavement.

2) Large signs having an area exceeding 50 square feet installed on multiple breakaway posts shall be mounted a minimum of 7' above the ground.

3) The height of the secondary sign mounted below another sign may be 4' measured from the bottom of the sign to the near edge of the pavement. Signs shall not overlap each other.



URBAN

1) Signs shall be mounted at a minimum height of 7' measured from the bottom of sign to the near edge of the pavement.

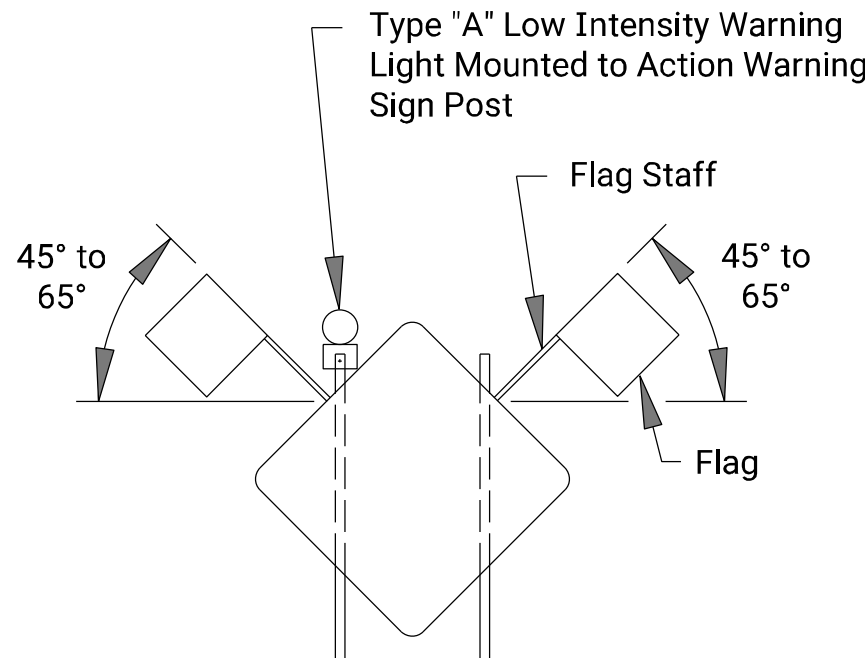
2) Neither portable nor permanent sign supports should be located on sidewalks or areas designated for pedestrian or bicycle traffic.

3) Signs mounted lower than 7' should not project more than 4" into pedestrian facilities.

4) The height from of the secondary sign mounted below another sign may be 6' measured from the bottom of sign to the near edge of the pavement. Signs shall not overlap each other.

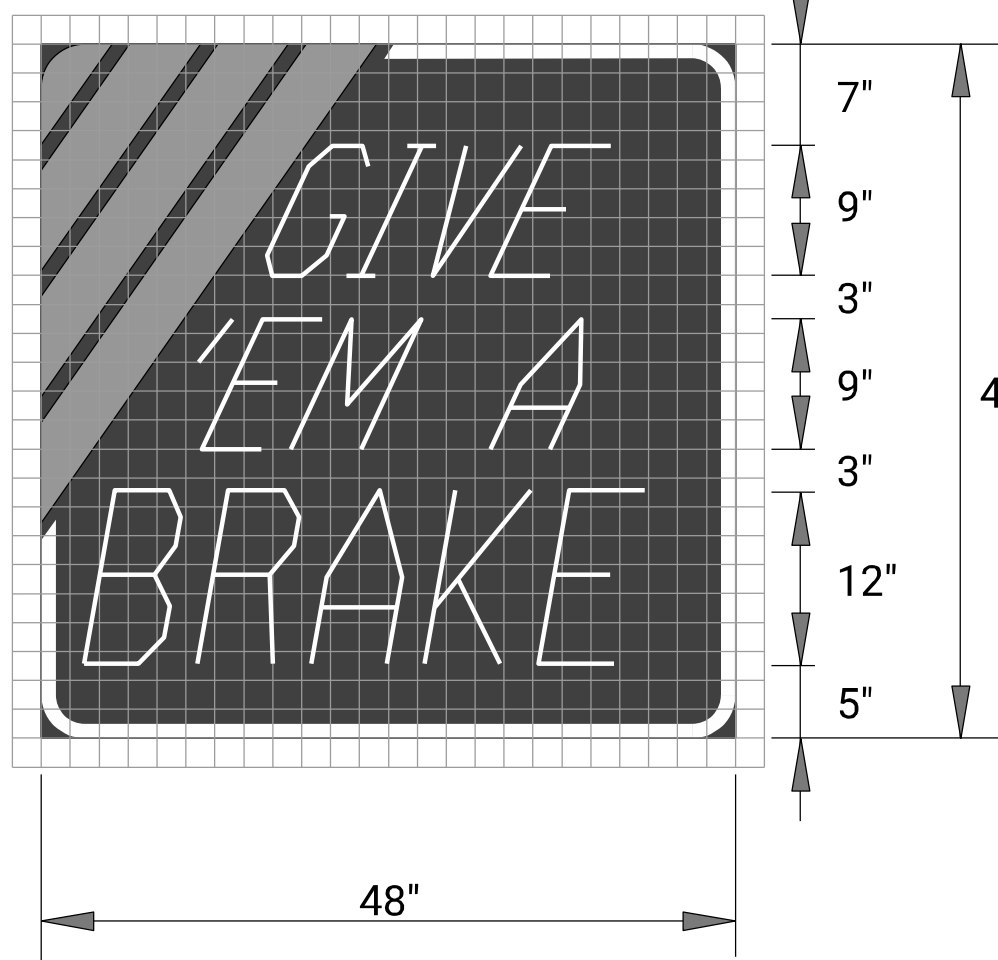
5) Large signs having an area exceeding 50 square feet installed on multiple breakaway posts shall be mounted a minimum of 7' above the ground.

* 6) Pedestrian detour signing shall be a minimum of 2' measured from the top of the pedestrian pathway to the bottom of the sign and shall not protrude into the walkway nor shall it project beyond the back of curb.

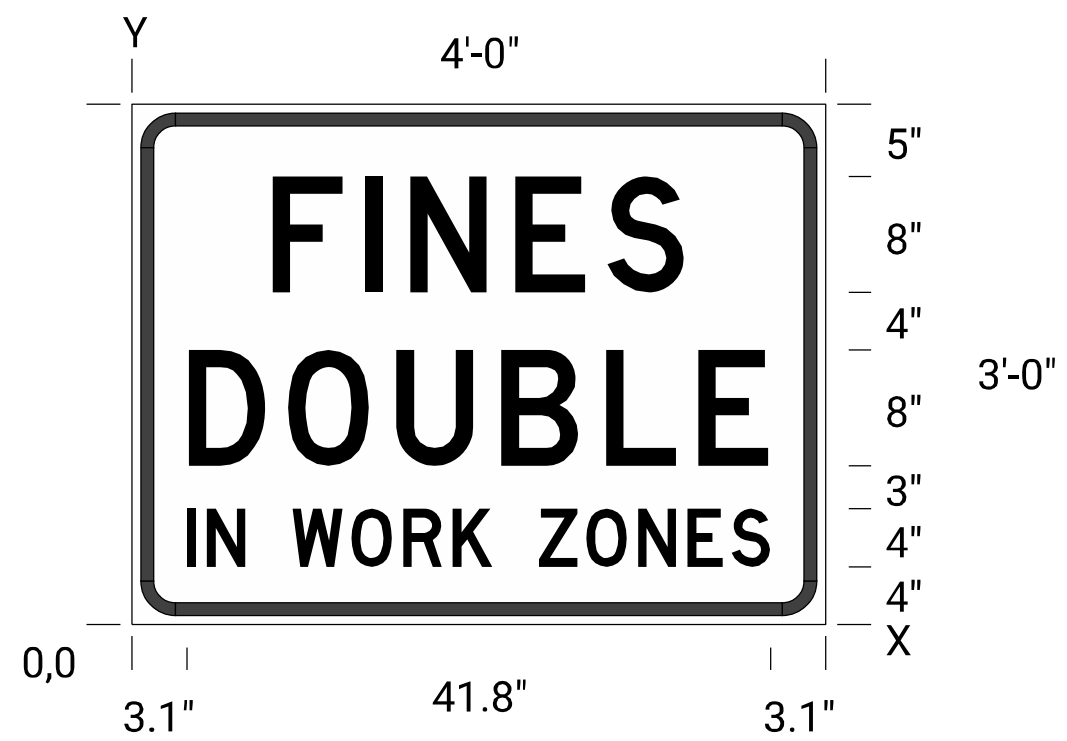


When the sign width is equal to or greater than 9', three or more wood posts may be used with a minimum of 4' between the centerline of each post. All signs less than 9' in width shall use a maximum of two wood posts.

In the case of hitting rock when driving posts
1. Shift the sign location. Do not violate minimum sign spacing.
2. With the engineer's approval, use acceptable alternative sign stands.



KI-104a



KI-105a

Sign Number	GIVE EM A BRAKE
Width x Height	4'-0" x 4'-0"
Border Width	1.0"
Corner Radius	4.0"
Stripe Width	3.0"
Mounting	Ground
Background	Type: Non-Reflective Color: Black
Legend/Border	Type: Reflective Color: White
Legend Font	Dutch 801 Roman SWC 25 Degree Slant
Stripes	Type: Reflective Color: Orange

Sign Number	FINES DOUBLE
Width x Height	4'-0" x 3'-0"
Border Width	0.9"
Corner Radius	3.0"
Mounting	Ground
Background	Type: Reflective Color: White
Legend/Border	Type: Non-Reflective Color: Black

Dimensions in inches

Spacings are to start of next letter

Y FONT	LETTER SPACINGS														HT LEN
23.0 D	9.7	6.4	3.2	7.3	6.4	5.4	9.7								8.0 28.6
11.0 D	3.9	6.9	7.5	7.3	6.4	4.9	3.9								8.0 40.3
4.0 D	3.1	1.6	2.7	3.2	4.3	3.8	3.6	2.8	3.2	3.4	3.8	3.6	3.2	2.7	3.1 41.8

Notes:

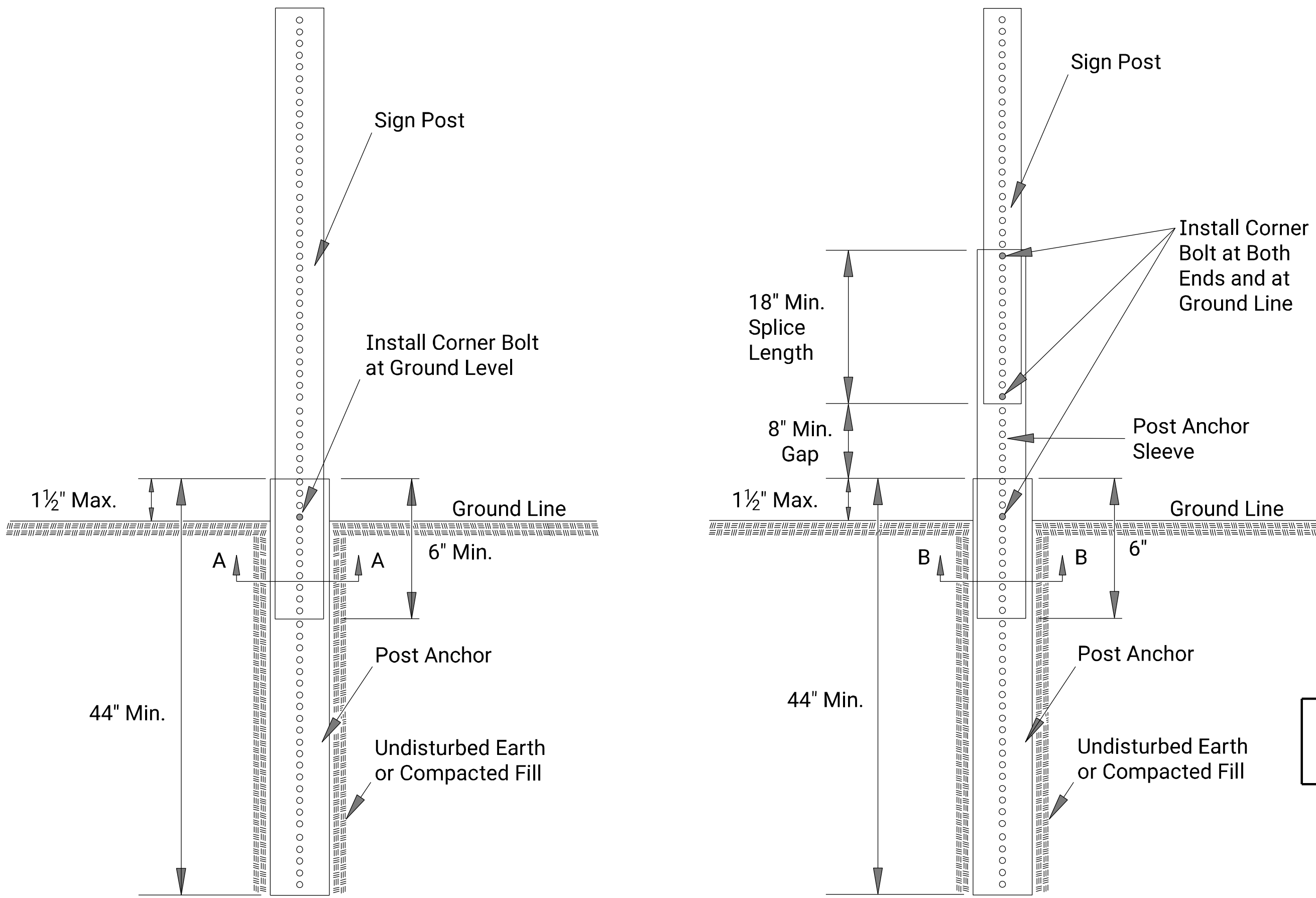
Typically, there are two sets of informational signs installed per project: one for each direction of traffic.

Install signs a minimum of 500' in advance of the road work ahead sign. The engineer may designate a more appropriate location if conditions dictate.

The informational signs are not to interfere with the traffic control signs for the project.

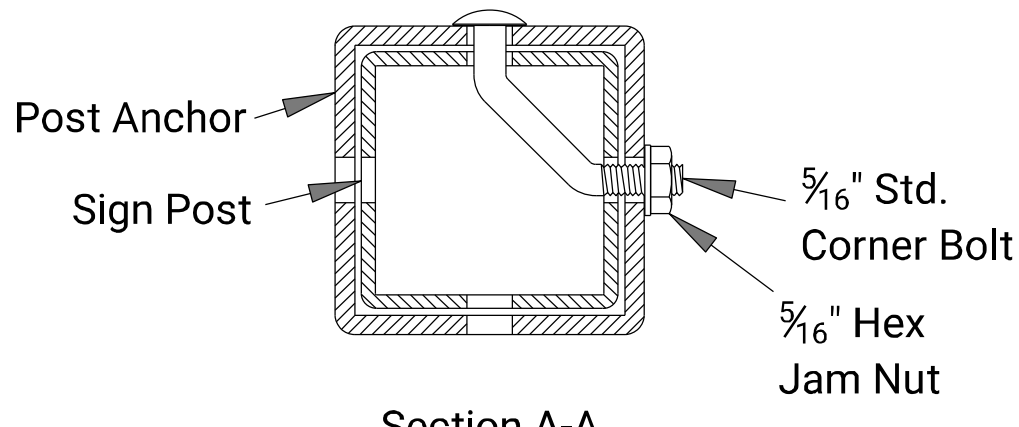
3				
2				
1				
NO.	DATE	REVISIONS	BY	APP'D
KANSAS DEPARTMENT OF TRANSPORTATION				
TRAFFIC CONTROL SIGN INFORMATION				
TE710				
FHWA APPROVAL 06/01/15 APP'D Kristina Pyle				
DESIGNED	R.W.B.	DETAILED	R.W.B.	QUANTITIES
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE	CK.

PERFORATED SQUARE STEEL TUBE (P.S.S.T.) POST SETUP

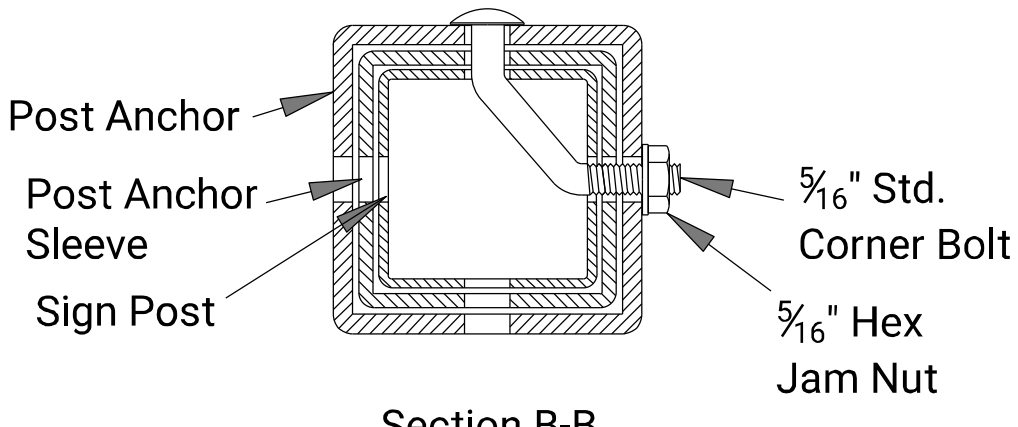


P.S.S.T. Detail

Telescoping P.S.S.T. Detail



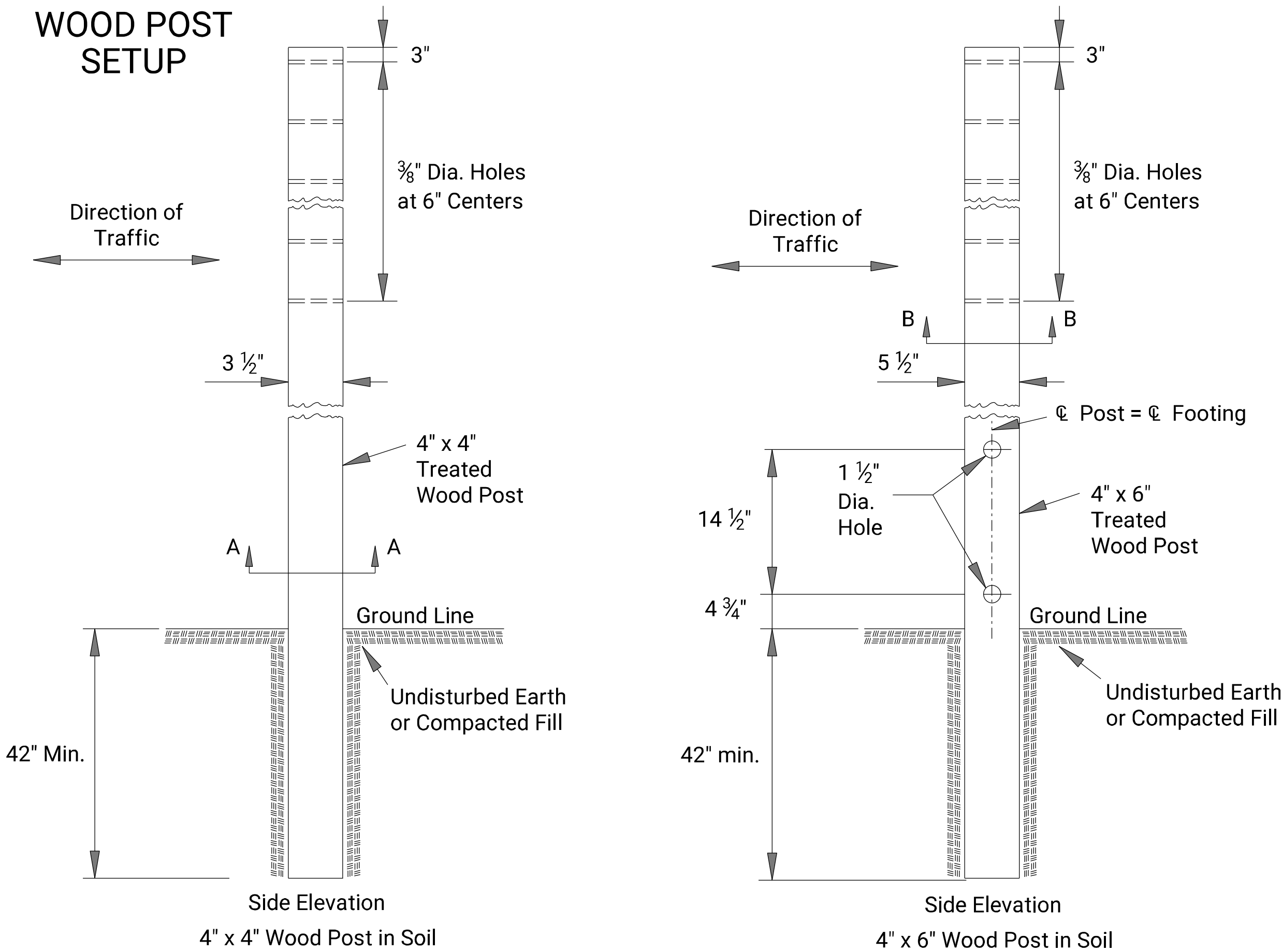
Section A-A



Section B-B

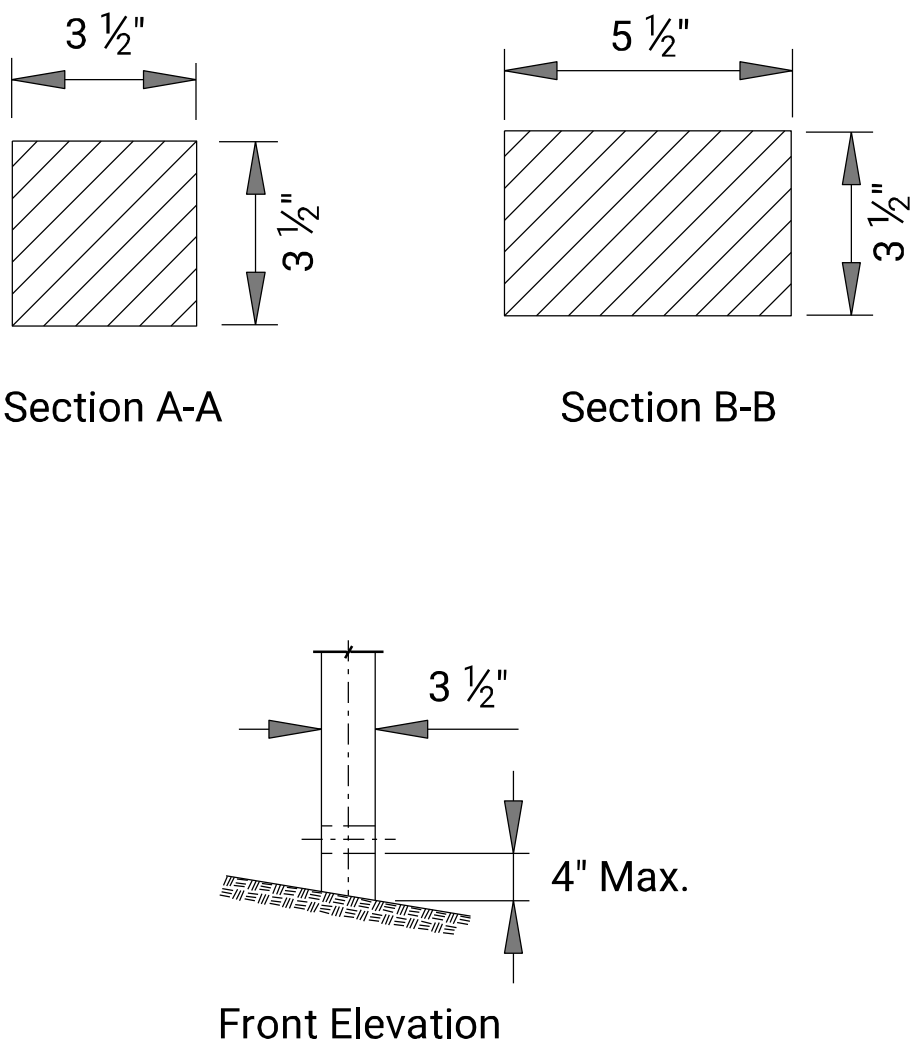
Details for 2", 2 1/4", or 2 1/2" sign posts
Place bolts in the same corner along each sign post.

WOOD POST SETUP



Side Elevation
4" x 4" Wood Post in Soil

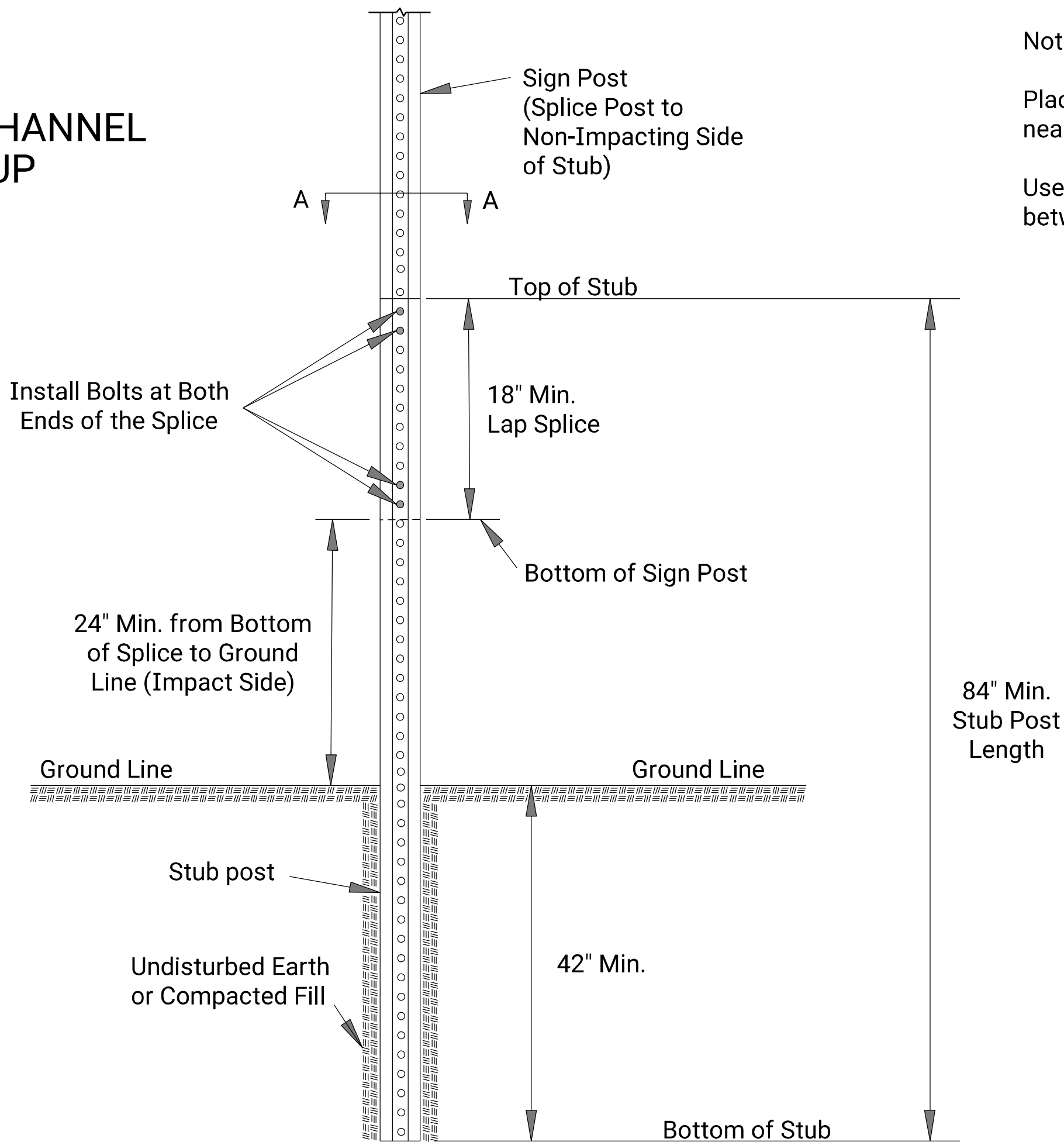
Side Elevation
4" x 6" Wood Post in Soil



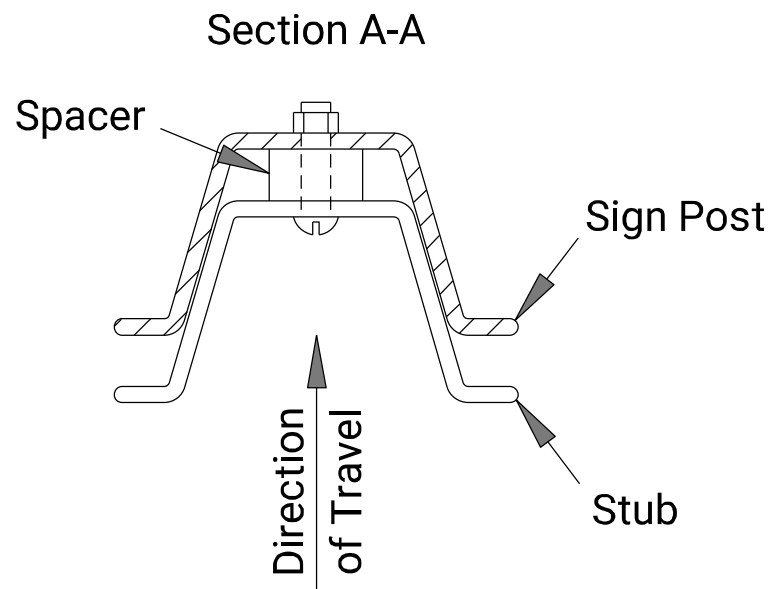
Front Elevation

See TE710 for Additional
Details and Requirements

3 LB/F U-CHANNEL SETUP



Notes:
Place two bolts at both ends of the splice through the holes nearest the ends of the splice.
Use manufacturer recommended spacers over the bolts between the spliced pieces of U-Channel.



3					
2					
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NO.	DATE	REVISIONS	BY	APP'D	
KANSAS DEPARTMENT OF TRANSPORTATION					
TRAFFIC CONTROL SIGN POSTS					
TE712					
DESIGNED		06/01/15		APP'D Kristina Pyle	
DESIGN CK.		B.A.H. DETAILED		R.W.B. QUANTITIES	
		DETAIL CK.		QUAN. CK.	
				TRACED	
				TRACE CK.	

Summary Of Traffic Control Devices (Each Per Day)

[illegible]

Lighted Devices *	
Work Zone Warning Light (Type "A" Low Intensity)	8
Work Zone Warning Light (Red Type "B" High Intensity)	
Arrow Display	
Portable Changeable Message Sign	

3				
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NO.	DATE	REVISIONS	BY	APP'D

KANSAS DEPARTMENT OF TRANSPORTATION

TRAFFIC CONTROL

SUMMARY OF DEVICES

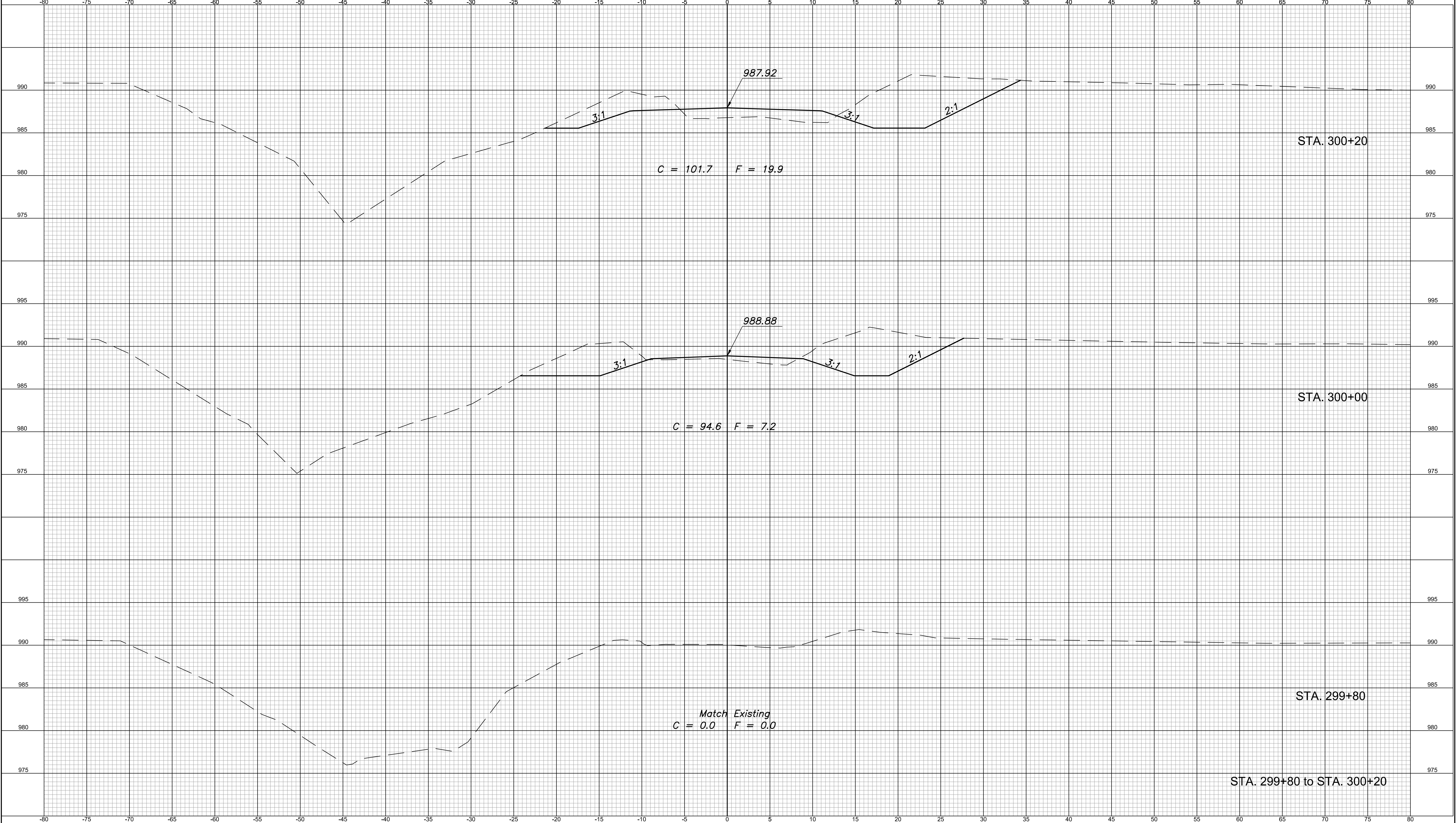
RECAPITULATION OF QUANTITIES

~~EE795~~

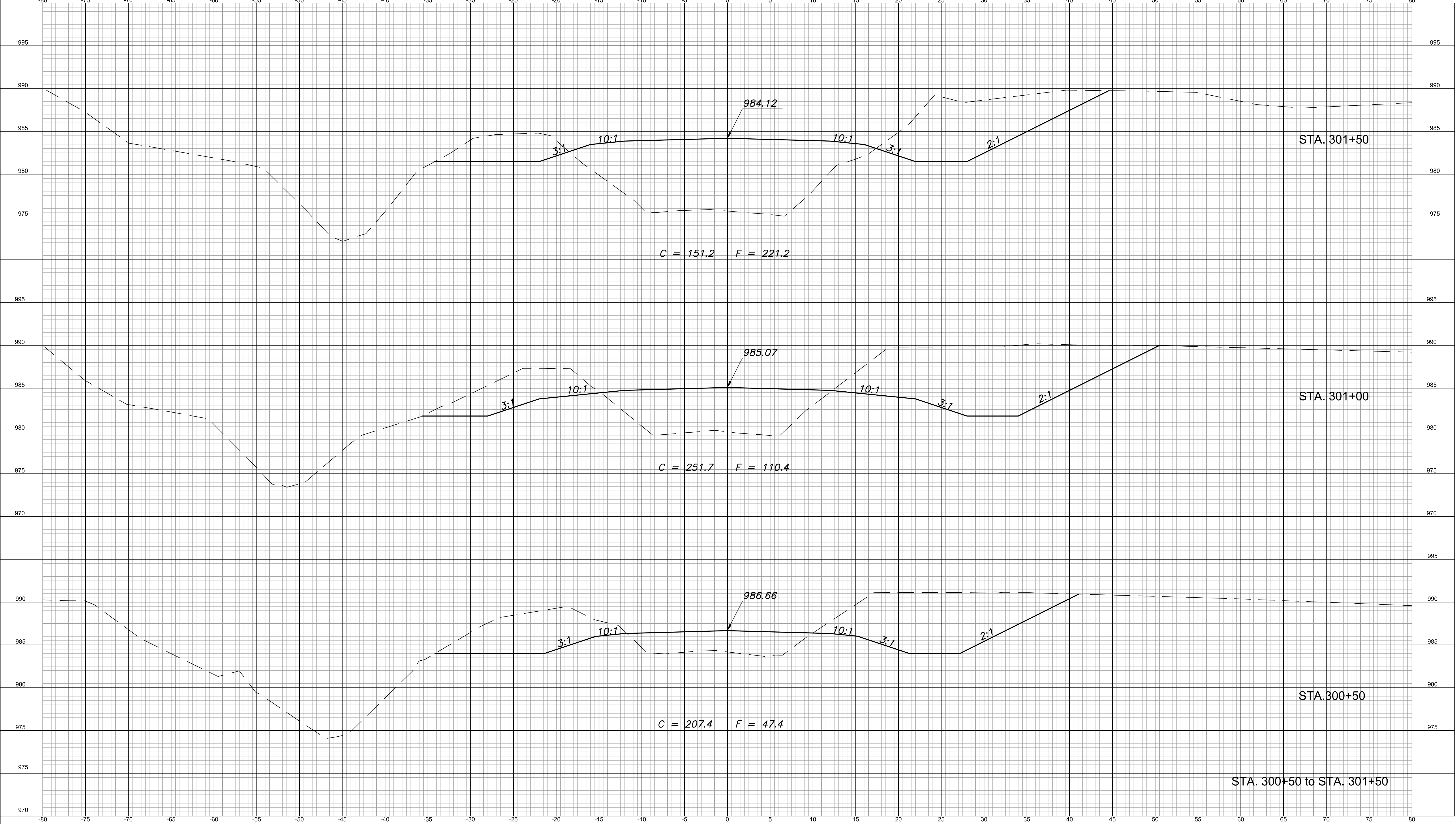
FHWA APPROVAL		06/01/15	APP'D	Kristina Erickson
DESIGNED	B.A.H.	DETAILED	R.W.B.	QUANTITIES
TRACED		TRACED		
DESIGN CK.	DETAIL CK.	QUAN. CK.	TRACE CK.	

NO SCALE

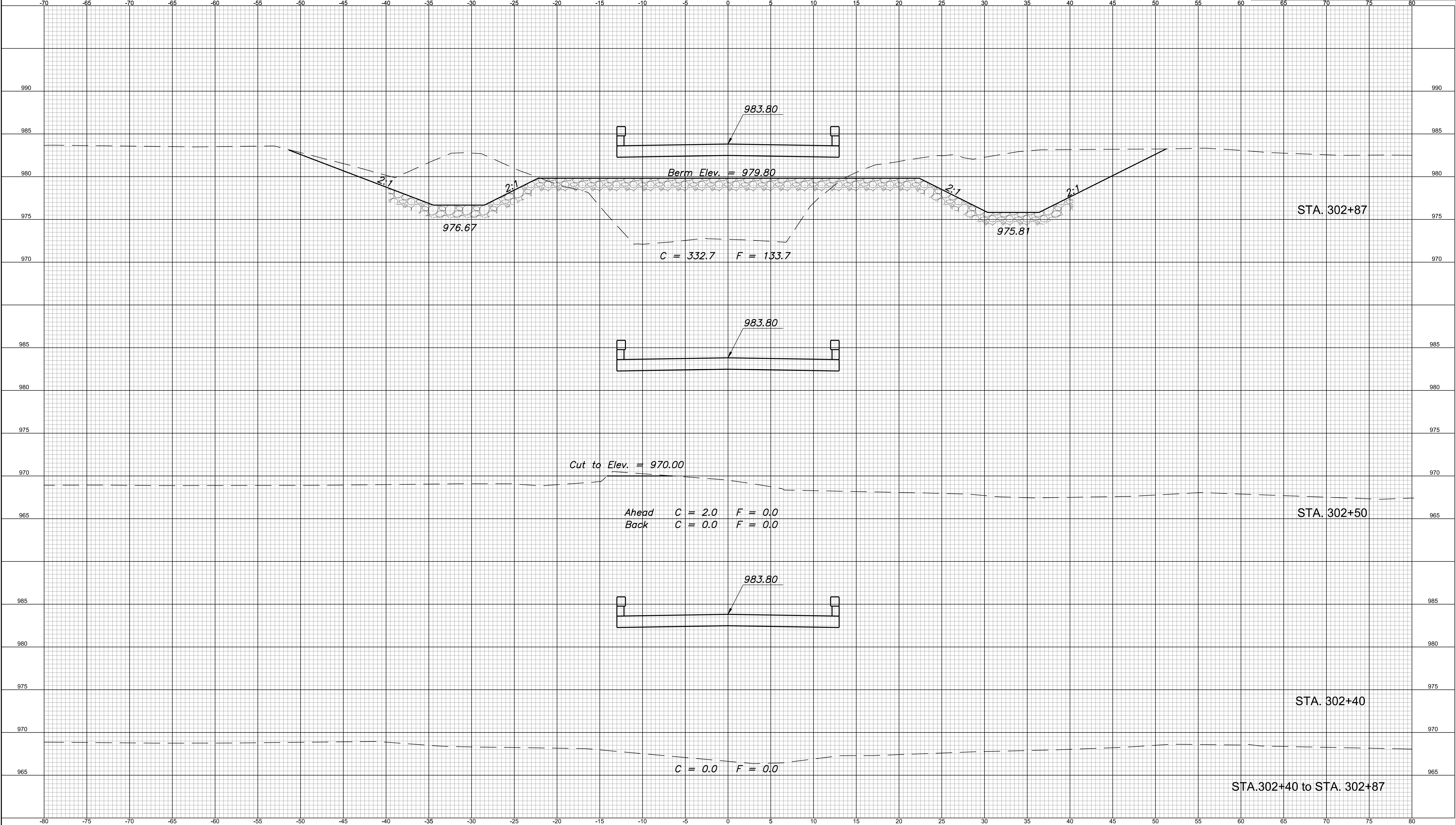
State	Project No.	Fiscal Year	Sheet No.	Total Sheets
Kansas	43 C-5078-01	2022	38	44



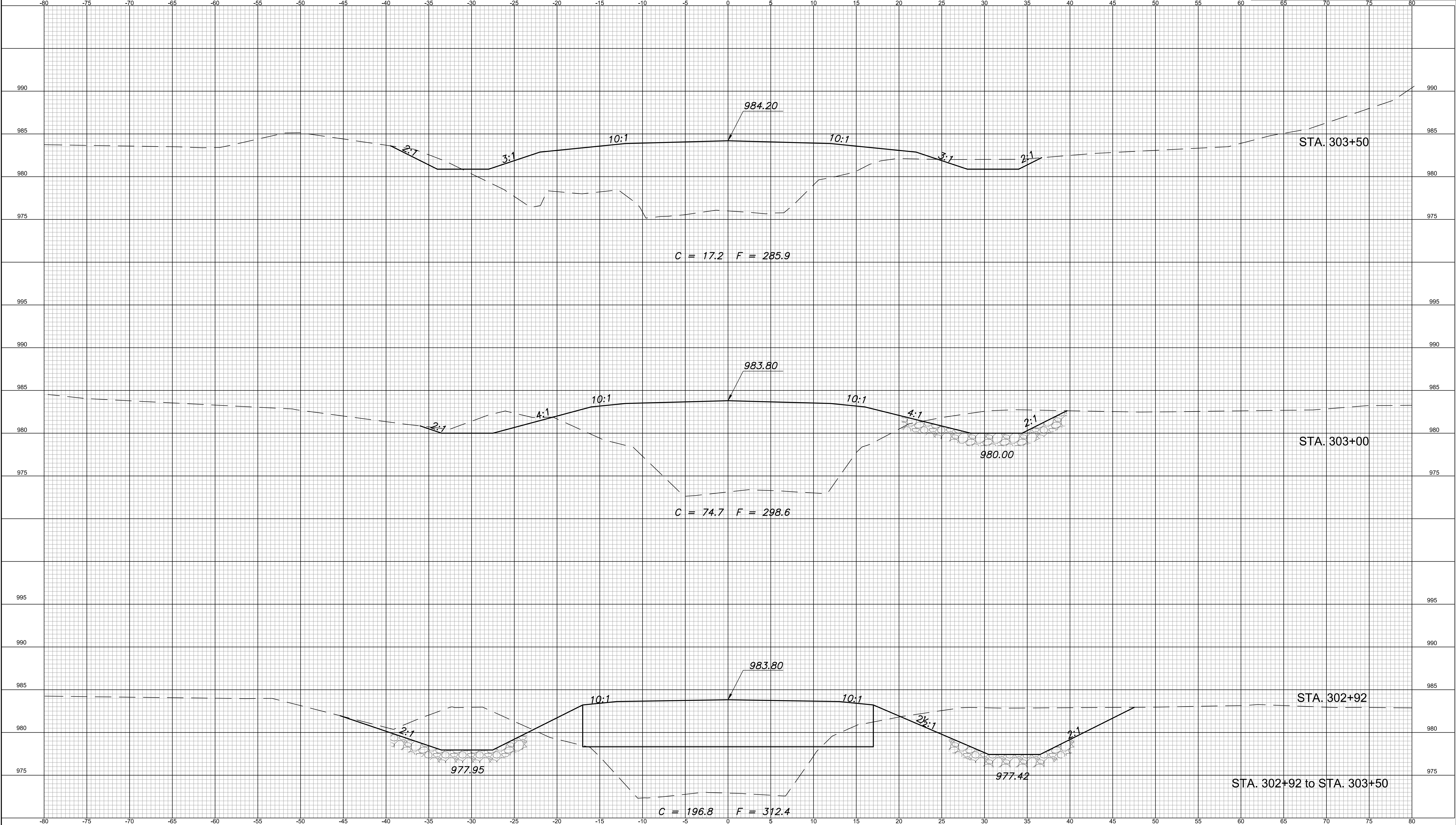
State	Project No.	Fiscal Year	Sheet No.	Total Sheets
Kansas	43 C-5078-01	2022	39	44



State	Project No.	Fiscal Year	Sheet No.	Total Sheets
Kansas	43 C-5078-01	2022	41	44



State	Project No.	Fiscal Year	Sheet No.	Total Sheets
Kansas	43 C-5078-01	2022	42	44



State	Project No.	Fiscal Year	Sheet No.	Total Sheets
Kansas	43 C-5078-01	2022	43	44

